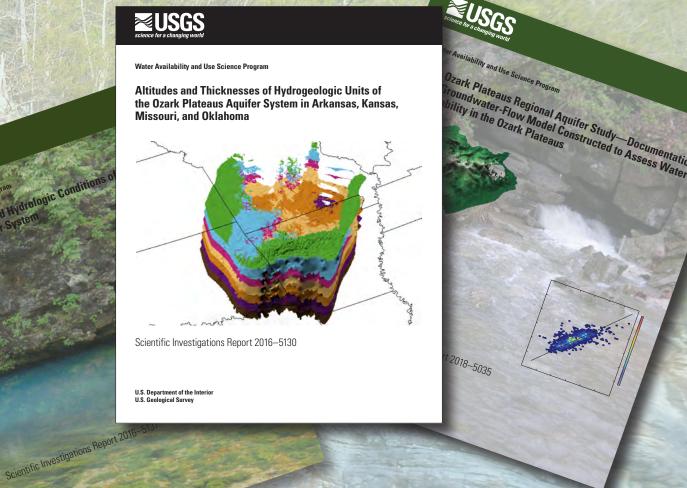


Groundwater Availability in the Ozark Plateaus

Aquifer System



bility and Use Science Program

Professional Paper 1854

U.S. Department of the Interior

U.S. Geological Survey

Groundwater Availability in the Ozark Plateaus Aquifer System By Brian R. Clark, Leslie L. Duncan, and Katherine J. Knierim Water Availability and Use Science Program Professional Paper 1854 **U.S. Department of the Interior U.S. Geological Survey**

U.S. Department of the Interior DAVID BERNHARDT, Secretary

U.S. Geological SurveyJames F. Reilly II, Director

U.S. Geological Survey, Reston, Virginia: 2019

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Foreword

Although often overlooked, groundwater is increasingly important to all our lives. Groundwater is the Nation's principal reserve of freshwater. It provides drinking water for half of the country, is essential to food production in the United States, and facilitates business and industrial activities. Groundwater also is an important source of water for sustaining the ecosystem health of rivers, wetlands, and estuaries throughout the country.

Groundwater level declines resulting from large-scale development of groundwater resources, together with other effects of pumping, have led to concerns about the future availability of groundwater to meet our Nation's needs. The compounding effects of recent droughts underscore the need for an updated status of the Nation's groundwater resources. Assessments of groundwater resources provide the science and information needed by decision makers and the public to manage and use water resources responsibly. The potential future effects on groundwater resources due to climate variability further exacerbate an already challenging situation, and the analysis of these potential effects add to an already complex task.

The U.S. Geological Survey's Water Availability and Use Science Program is conducting large-scale multidisciplinary regional studies of groundwater availability, including the study of the Ozark Plateaus aquifer system described herein. The regional studies are intended to inform citizens, communities, and natural resource managers of the condition of the Nation's groundwater resources and how changes in land use, water use, and climate have affected those resources. The studies also are aimed at developing tools to enable scientists and managers to forecast how these resources may change in the future. The findings from these individual groundwater assessments of principal aquifer systems will be combined to form a national assessment of groundwater availability. Results derived from these studies will help answer questions about the Nation's ability to meet current and future demands for groundwater.

Donald Cline Associate Director, USGS Water Resources Mission Area



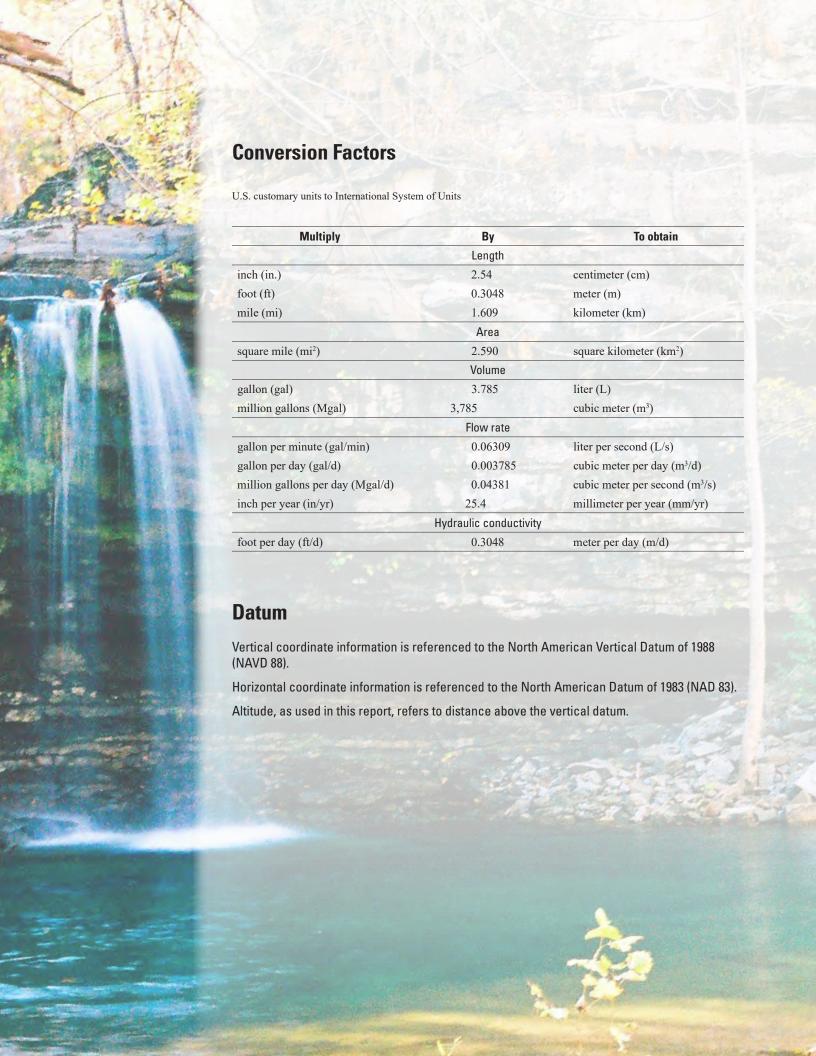
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otograph by Brian Clark

Groundwater Availability in the Ozark Plateaus Aquifer System

By Brian R. Clark, Leslie L. Duncan, and Katherine J. Knierim

Executive Summary

The study described in this report, initiated by the U.S. Geological Survey in 2014, was designed to evaluate fresh groundwater resources within the Ozark Plateaus, central United States, as an area within a broader national assessment of groundwater availability. The goals of the Ozark study were to evaluate historical effects of human activities on water levels and groundwater availability, quantify groundwater resources now and under probable future pumping and climate conditions, and evaluate existing monitoring networks for their value in making better predictions of future groundwater resources. Previous studies include simulation of local-scale groundwater flow under varying temporal scales, or simulation of the regional system under steady-state conditions. While these studies are useful, particularly for the problem for which they were designed, there is a need to look at the larger regional system under transient conditions to fully evaluate the water resource over time. This study focused on multiple spatial and temporal scales to examine changes in groundwater pumping, storage, and water-level declines. The regional scale provides a broad view of the sources and demands on the system with time.

The study area covers approximately 68,000 square miles in the central United States in parts of Missouri, Arkansas, Kansas, and Oklahoma and encompasses the Ozark Plateaus Physiographic Province (Ozark Plateaus), including the Salem Plateau, Springfield Plateau, and Boston Mountains. Groundwater is withdrawn from the Ozark Plateaus aquifer system (Ozark system) for public supply and for domestic, agriculture (including irrigation and aquaculture), livestock, and non-agricultural use (including industrial, thermoelectric power generation, mining, and commercial). The Ozark system provides an important drinking-water supply for people living in the Ozark Plateaus because public supply and domestic use combined constitute the largest groundwater use. Precipitation is the ultimate source of freshwater to the Ozark system; most rainfall occurs during April, May, and June, and precipitation increases generally from north to south across the

Groundwater use currently accounts for only 10 percent of the total water use in the areas overlying the Ozark system, but provides a critical drinking-water resource because

public supply and domestic groundwater withdrawals are largely from groundwater resources. The 380 million gallons per day of groundwater withdrawn from the Ozark system in 2010 accounts for approximately 2 percent of recharge. Although groundwater use represents a small component of the hydrologic budget, because of low storage in aquifer units, cones of depression with steep water-level gradients can develop quickly around pumping centers.

The amount of water entering and leaving the aquifer system from 1900 to about 1965 was relatively constant at a rate of about 13 billion gallons per day (Bgal/d). Much of this inflow of water is discharged through streams in the system to balance the hydrologic budget. Changes in storage over time (from outflows to inflows) reflect the large variability in recharge: if recharge decreases, water levels will decrease, resulting in less groundwater discharge to streams and more water released from aquifer storage. Conversely, when recharge increases, water levels increase, more groundwater discharges to streams, and aquifer storage is replenished. Although pumping generally increased from 1900 to 2016, it does not appear to correlate with the change in storage over the same time period. Regionally, simulated change in groundwater storage corresponds with changes in recharge, more so than with increases in pumping.

Average recharge was 11.6 Bgal/d for the period 1900 to 2016. Recharge was generally above average from predevelopment to 1965, followed by a period of belowaverage recharge from 1965 to about 1980. Recharge remained consistently above average from 1980 to about 1988, after which there was a period of average or below-average recharge, reflected by a decline through the mid-2000s.

The implications and potential effects of increased pumping and long-term climate change on the Ozark Plateaus hydrologic system and groundwater availability are a concern for communities and resource managers in the area. Pumping varies from year to year, but is generally expected to moderately increase with population, industrial, and agricultural needs. Most climate models predict warmer minimum and maximum air temperatures by midcentury in the Ozark Plateaus area, especially from midspring through early fall. Three scenarios were developed to simulate possible future conditions from 2016 to 2060 and assess the potential effects on the hydrologic system and availability of

water resources. For each scenario, changes in water levels and hydrologic budget components were evaluated from predevelopment (1900) to present (2016) and 45 years into the future (2060). The baseline scenario represents an extension of the average (1996 to 2016) seasonal pumping and recharge values. The pumping scenario is an extension of the average (1996 to 2016) seasonal recharge values with increases in pumping following the historical trend for the period 2016–2060 of up to 120 percent of the 1996 to 2016 average seasonal pumping values. The general circulation model (GCM) scenario is an extension of the average (1996 to 2016) seasonal pumping values and variable recharge based on seasonal averages of soil water storage from a water-balance model using temperature and precipitation from multiple GCMs.

The general patterns of water-level decline are similar for each scenario. The areas of water-level decline in southwest Missouri and northeast Oklahoma are only marginally different by 2060 from those of 2009. In one area south of Springfield, Mo., water-level declines are less in the baseline and GCM scenarios than in 2009. This may be the result of a transition from groundwater use to surface-water supplies for a larger percentage of the demand in the area.

For all three scenarios, forecasted pumping, recharge, and aquifer properties play an important role in determining the uncertainty of water-level forecasts at 94 real-time observation wells. Simulated aquifer properties in the productive middle and lower Ozark aquifers and the St. Francois confining unit of the Ozark system contribute most to predictive uncertainty in water levels at approximately 35 percent of the real-time observation wells. Out of the 94 real-time observation wells, 82 are developed in the lower Ozark aquifer.

Introduction

Fresh groundwater in the Ozark Plateaus aquifer system (hereafter referred to as the Ozark system) is the source of drinking water for more than 2 million people through municipal and rural water districts and private domestic wells and is also withdrawn for industrial and agricultural uses. The Ozark system is composed predominantly of fractured and dissolved carbonate rock (karst). Groundwater storage values are relatively low, but conductance through fractures and dissolution-enlarged conduits can be high enough that the aquifer system responds rapidly to hydrologic stresses from climatic and human-induced events. For example, seasonal water-level declines have been observed in Ozark system aquifers throughout southeastern Kansas, northeastern Oklahoma, and southwestern Missouri. In this area, groundwater is under confined conditions, such that relatively small amounts of pumping often result in deep cones of depression. Municipal water suppliers have expressed concerns over groundwater availability, particularly during low-recharge, high-use times. These concerns often relate to short-term, seasonal conditions where a few months of drought can result in large groundwater-level declines because of the low storage values of the aquifer system.

Previous studies included simulation of local-scale groundwater flow under varying temporal scales, or simulation of the regional system under steady-state conditions. While these studies are useful, particularly for the problem for which they were designed, there is a need to consider the larger regional system under transient conditions, including short-term seasonal changes, to fully evaluate the water resource over time. In 2014, the U.S. Geological Survey (USGS) Water Availability and Use Science Program initiated an assessment of groundwater availability of the Ozark system as one of several ongoing regional assessments of the principal aquifers of the Nation (Reilly and others, 2008).

Purpose and Scope

The purpose of this report is to describe the historical, current, and possible future availability of groundwater in the Ozark system. This study synthesizes results from companion reports that interpreted the hydrogeologic framework (Westerman and others, 2016a, b), refined the conceptual model of the flow system (Hays and others, 2016), analyzed recent groundwater-level measurements (Nottmeier, 2015), estimated historical groundwater-use rates (Knierim and others, 2017), and described the construction and calibration of the groundwater-flow model version 1.0 (Clark and others, 2018). Model version 1.0 was modified to version 1.1 and used herein to understand how a range of future conditions may affect future water resources. This report also documents the changes from version 1.0 of the groundwater-flow model to version 1.1 and builds on previous work at a variety of scales that has taken place across the area for many years by various Federal, State, and local agencies. The analysis includes discussion of associated flow through all aquifers and confining units in the Ozark system during 1900-2016.

Study Area Description

The study area covers approximately 68,000 square miles (mi2) in the central United States in parts of Missouri, Arkansas, Kansas, and Oklahoma (fig. 1) and encompasses the Ozark Plateaus Physiographic Province (hereafter, the Ozark Plateaus), including the Salem Plateau, Springfield Plateau, and Boston Mountains (Hays and others, 2016, fig. 1). The Ozark system is generally bounded by the Missouri River on the northern boundary, the Mississippi River and more broadly the Mississippi embayment to the east and southeast, and the Arkansas River to the south (fig. 1). The western boundary is defined by a regional topographic low extending from northeastern Oklahoma to the Missouri River and coincides with a freshwater-saltwater transition zone (fig. 1) where freshwater from the Ozark system mixes with saltwater from the Western Interior Plains aguifer system (Hays and others, 2016; Jorgensen and others, 1996). The bottom of the Ozark

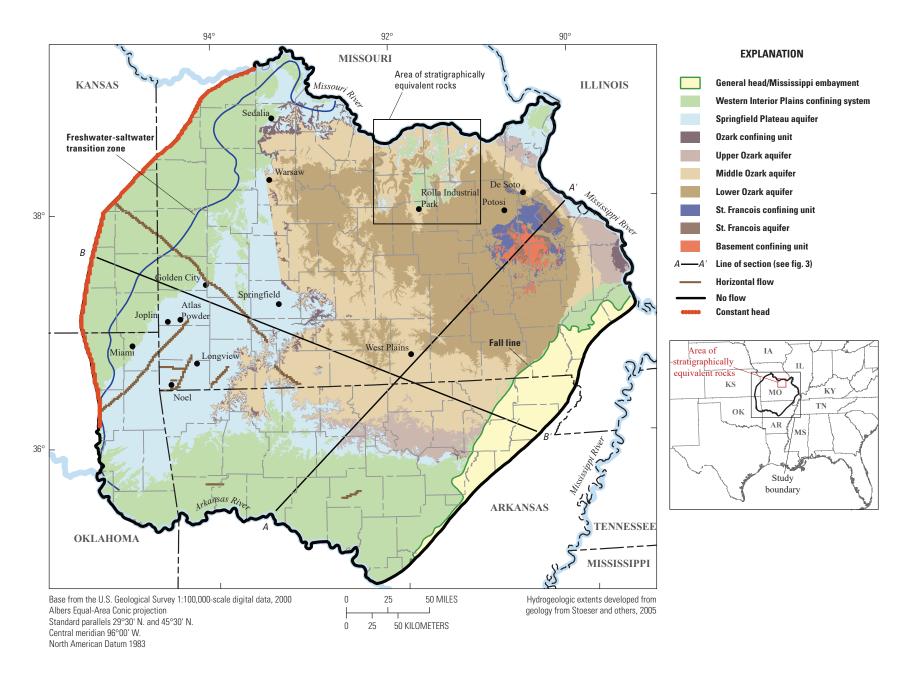


Figure 1. Surficial geology and structural features of the Ozark Plateaus aquifer system and boundary conditions for the numerical model of the Ozark Plateaus aquifer system.

system is bounded by metamorphic and igneous rocks of Precambrian age (Basement confining unit) that underlie much of the midwestern aquifers in the central United States (Jorgensen and others, 1993).

Hydrogeologic Setting

The Ozark system is characterized by uplifted plateaus composed of relatively flat-lying sedimentary rocks of Paleozoic age that drape over basement rocks of Precambrian age. The hydrogeologic framework consists of interbedded carbonate and clastic units ranging in age from Cambrian to Early Pennsylvanian (Hays and others, 2016; Jorgensen and others, 1996; Westerman and others, 2016a) (fig. 2). Sandstone and karstified limestone and dolostone units serve as primary aquifers, and shale or dense dolostone generally act as confining units. In ascending order, the units include the Basement confining unit, St. Francois aquifer, St. Francois confining unit, lower Ozark aquifer, middle Ozark aquifer, upper Ozark aquifer, Ozark confining unit, Springfield Plateau aquifer, and Western Interior Plains confining system (Hays and others, 2016; Jorgensen and others, 1993; Westerman and others, 2016a) (fig. 2). Substantial refinement of the hydrogeologic framework was accomplished by Westerman and others (2016a, b) by building on regional groundwater studies from Jorgensen and others (1993) and Imes and Emmett (1994) and many State- and smaller-scale studies (for example, Harvey and others, 1983; Howe and Koenig, 1961; Miller and Vandike, 1997). Westerman and others (2016a, b) compiled borehole data and interpreted lithologic information to assign hydrogeologic units for the study area, creating framework grids of hydrogeologic units' altitude and thickness. As part of the updated hydrogeologic framework, the Ozark aquifer was refined into three units (lower, middle, and upper) based on unique hydraulic properties (Westerman and others, 2016a, b), which contributed to a more refined groundwater-flow model than those used in earlier studies.

The Basement confining unit is generally buried under 1,700 to 2,700 feet (ft) of Paleozoic-age rocks throughout the Ozark system (except where exposed, fig. 1), and the top of the unit dips sharply towards the south (Westerman and others, 2016b) (fig. 3). The unit exhibits very low permeability owing to the igneous and metamorphic rocks that compose the basement complex (Jorgensen and others, 1993) and therefore was not explicitly modeled for this study. Because the Basement confining unit forms the structural base for the Ozark system, however, the geometry affects the presence, thickness, and structure of the overlying sedimentary units (Westerman and others, 2016a). The geometry of the Basement confining unit—including relative thickness, dip, and degree of faulting also exerts control on groundwater flow and karst development in overlying sedimentary strata (Brahana and others, 2009), such that the hydrogeologic framework from Westerman and others (2016a, b) provides an important updated coverage of Basement confining unit depth across the Ozark system.

The basal hydrogeologic unit of the Ozark system is the St. Francois aquifer of Cambrian age (fig. 2), which has a median thickness of 291 ft (Westerman and others, 2016b) and is composed of permeable sandstones and dolostones (Hays and others, 2016). Although wells penetrating the aquifer yield water, the St. Francois aquifer is generally not used beyond its outcrop area near the St. Francois Mountains because of shallower sources of water available throughout much of the Ozark system (Hays and others, 2016; Imes and Emmett, 1994). The St. Francois aquifer is confined throughout much of its extent where overlain by the St. Francois confining unit. The St. Francois confining unit of Cambrian age has a median thickness of 228 ft (Westerman and others, 2016a) and is composed of low-permeability shale, siltstone, dolostone, and limestone (Hays and others, 2016).

The Ozark aquifer includes productive dolostone units of the lower Ozark aquifer (median thickness of 885 ft), denser and relatively lower permeability dolostones of the middle Ozark aquifer (median thickness of 416 ft), and the mixed lithology of limestone, dolostone, shale, and limited sandstone units of the upper Ozark aquifer (median thickness of 590 ft) (Hays and others, 2016; Imes and Emmett, 1994; Westerman and others, 2016a) (fig. 2). The lower Ozark aquifer is generally the most productive part of the Ozark aquifer owing to the enhanced secondary and tertiary porosity and permeability from karst formations (Hays and others, 2016). Wells penetrating the Ozark aquifer yield between 50 and 100 gallons per minute (gal/min), but yield can increase to more than 1,000 gal/min where wells penetrate the lower Ozark aquifer (Adamski and others, 1995). The Ozark aquifer is generally unconfined where rocks crop out in the Salem Plateau and confined where overlain by the Ozark confining unit (figs. 1 and 3). The lower Ozark aquifer is broadly confined where overlain by the low-permeability units of the middle Ozark aquifer. The Ozark confining unit is relatively thin (median thickness is 42 ft; Westerman and others, 2016a) to absent in some areas, which permits hydraulic connection of the underlying Ozark aquifer and overlying Springfield Plateau aquifer (fig. 2). Lithology of the Ozark confining unit varies throughout the study area, but is generally composed of low-permeability limestone, sandstone, and shale units (Hays and others, 2016).

The uppermost aquifer of the Ozark system is the Springfield Plateau aquifer of Mississippian age, which consists of limestone with varying chert abundance and has a median thickness of 237 ft (Hays and others, 2016; Westerman and others, 2016a). Hydraulic properties vary owing to the variable chert content, fracture networks, and conduits (Hays and others, 2016). Well yields reflect the anisotropic hydraulic properties, such that yields of 10 to 100 gal/min are observed in more porous and permeable zones compared to yields of less than 2 gal/min where only primary porosity occurs (Hays and others, 2016; Kresse and others, 2014). The Springfield Plateau aquifer is generally unconfined throughout much of its extent where it crops out in the Springfield Plateau, except where overlain by the Western Interior Plains confining system (fig. 1).



Era	System	Southeastern Missouri	Southwestern Missouri	Southeastern Kansas	Northeastern Oklahoma	Northern Arkansas		oed in t	nfining units he report Hydrogeologic system	Model layer
	Pennsylvanian	Pleasanton Formation ¹ Marmaton Group ¹ Cherokee Group ¹	Kansas City Group Pleasanton Formation Marmaton Group Cherokee Group	Kansas City Group Pleasanton Group Marmaton Group Cherokee Group	Marmaton Group Cabaniss Group Krebs Group Atoka Formation Bloyd Shale Hale Formation	McAlester Formation Hartshorne Sandstone Atoka Formation Bloyd Shale Hale Formation			ning system ²	
Paleozoic	ippian	Vienna Limestone ^{1,4} Tar Springs Sandstone ^{1,4} Glen Dean Limestone ^{1,4} Hardinsburg Sandstone ^{1,4} Golconda Formation ^{1,4} Cypress Formation ^{1,4} Paint Creek Formation ¹ Yankeetown Sandstone ¹ Renault Formation ^{1,4} Aux Vases Sandstone ¹ Ste. Genevieve Limestone ³ St. Louis Limestone ³	Fayetteville Shale Batesville Sandstone Hindsville Limestone Carterville Formation St. Louis Limestone	St. Louis Limestone	Pitkin Limestone Fayetteville Shale Batesville Sandstone Hindsville Limestone Moorefield Formation	Pitkin Limestone Fayetteville Shale Batesville Sandstone Moorefield Formation			Western Interior Plains confining system ²	Layer 1
Pale	Mississippian	Salem Limestone ³ Warsaw Limestone ³ Keokuk Limestone ³ Burlington Limestone ³ Fern Glen Limestone ³	Salem Limestone Warsaw Limestone Keokuk Limestone Burlington Limestone Elsey Formation Reeds Spring Formation Pierson Formation	Salem Limestone Warsaw Limestone Keokuk Limestone Burlington Limestone Fern Glen Limestone	Keokuk Limestone⁴ Boone Formation Reeds Spring Member St. Joe Limestone Member	Boone Formation Reeds Spring Member St. Joe Limestone Member	Springfield Plateau aquifer		Ozark Plateaus aquifer system ⁵	Layer 2
	an	Chouteau Limestone Hannibal Shale Bachelor Formation ⁴ Bushberg Sandstone Glen Park Limestone Chattanooga Shale	Northview Shale Sedalia Limestone Compton Limestone Chattanooga Shale	Chouteau Limestone Chattanooga Shale	Northview Shale ⁴ Compton Limestone ⁴ Woodford Shale Chattanooga Shale	Chattanooga Shale	Ozark confining unit		Ozark Plateaus	Layer 3
	Devonian	St. Laurent Limestone Grand Tower Limestone Clear Creek Chert ⁴ Little Saline Limestone Bailey Limestone	Callaway Formation ⁴ Fortune Formation ⁴		Sallisaw Formation Frisco Limestone	Clifty Limestone Penters Chert	Upper	Ozark aquifer		Layer 4

	Silurian	Bainbridge Limestone Sexton Creek Limestone ⁴			St. Clair Limestone	Lafferty Limestone St. Clair Limestone Brassfield Limestone				
Paleozoic	Ordovician	Girardeau Limestone Orchard Creek Shale Thebes Sandstone Maquoketa Shale Cape Limestone Kimmswick Limestone Decorah Formation Plattin Limestone Rock Levee Formation Joachim Dolomite Dutchtown Formation St. Peter Sandstone Everton Formation Smithville Formation Powell Dolomite	Kimmswick Limestone Plattin Limestone Joachim Dolomite St. Peter Sandstone Everton Formation Smithville Formation Powell Dolomite	Powell Dolomite	Sylvan Shale Fernvale Limestone Viola Limestone Fite Limestone Tyner Formation Burgen Sandstone Smithville equivalent Powell Dolomite	Cason Shale Fernvale Limestone Kimmswick Limestone Plattin Limestone Joachim Dolomite St. Peter Sandstone Everton Formation Smithville Formation Powell Dolomite	Upper	Ozark aquifer	Ozark Plateaus aquifer system ⁵	Layer 4
Pal		Cotter Dolomite Jefferson City Dolomite	Cotter Dolomite Jefferson City Dolomite	Cotter Dolomite Jefferson City Dolomite	Cotter Dolomite Jefferson City Dolomite	Cotter Dolomite Jefferson City Dolomite	Middle		ateau	Layer 5
		Roubidoux Formation Gasconade Dolomite Van Buren Formation ⁴ Gunter Sandstone Member ⁴	Roubidoux Formation Gasconade Dolomite Gunter Sandstone Member ⁴	Roubidoux Formation Gasconade Dolomite Van Buren Formation ⁴ Gunter Sandstone Member ⁴	Roubidoux Formation Gasconade Dolomite Van Buren Formation ⁴ Gunter Sandstone Member ⁴	Roubidoux Formation Gasconade Dolomite Van Buren Formation Gunter Sandstone Member	Lower		Ozark Pla	Layer 6
		Eminence Dolomite Potosi Dolomite	Eminence Dolomite Potosi Dolomite	Eminence Dolomite Potosi Dolomite	Eminence Dolomite Potosi Dolomite	Eminence Dolomite Potosi Dolomite				
	Cambrian	Doe Run Formation Derby Formation Davis Formation	Doe Run Formation Derby Formation Davis Formation	Doe Run equivalent Derby equivalent Davis equivalent	Doe Run equivalent Derby equivalent Davis equivalent	Doe Run equivalent Derby equivalent Davis equivalent	St. Franc confinir unit			Layer 7
	Ca	Bonneterre Formation Reagan Sandstone ⁴ Lamotte Sandstone	Bonneterre Formation Reagan Sandstone ⁴ Lamotte Sandstone	Bonneterre Dolomite Reagan Sandstone Lamotte Sandstone	Bonneterre Dolomite Reagan Sandstone Lamotte Sandstone	Bonneterre Dolomite Reagan equivalent Lamotte Sandstone	St. Franc aquifer			Layer 8 Layer 9
			Precambrian igned	ous and metamorphic rocks	3		Baseme confinir unit			

¹Geologic unit in southeastern Missouri that is stratigraphically equivalent to geologic units in the Western Interior Plains confining system but not part of the confining system.

Figure 2. Generalized correlation of Paleozoic-age stratigraphic units, regional hydrogeologic units of the Ozark Plateaus aquifer system, and corresponding model layer numbers (modified from Clark and others, 2018, and Hays and others, 2016).

²The Western Interior Plains confining system also includes younger sediments west of the study area.

³Geologic unit in southeastern Missouri that is stratigraphically equivalent to geologic units in the Springfield Plateau aquifer but not part of the aquifer.

⁴Unit follows usage of the Missouri Division of Geology and Land Survey, the Kansas Geological Survey, or the Oklahoma Geological Survey.

The Western Interior Plains aquifer system deeply buried in the western part of the study area included where permeable carbonate rocks in the subsurface are equivalents of the aquifers of the Ozark Plateaus aquifer system (Miller and Appel, 1997).

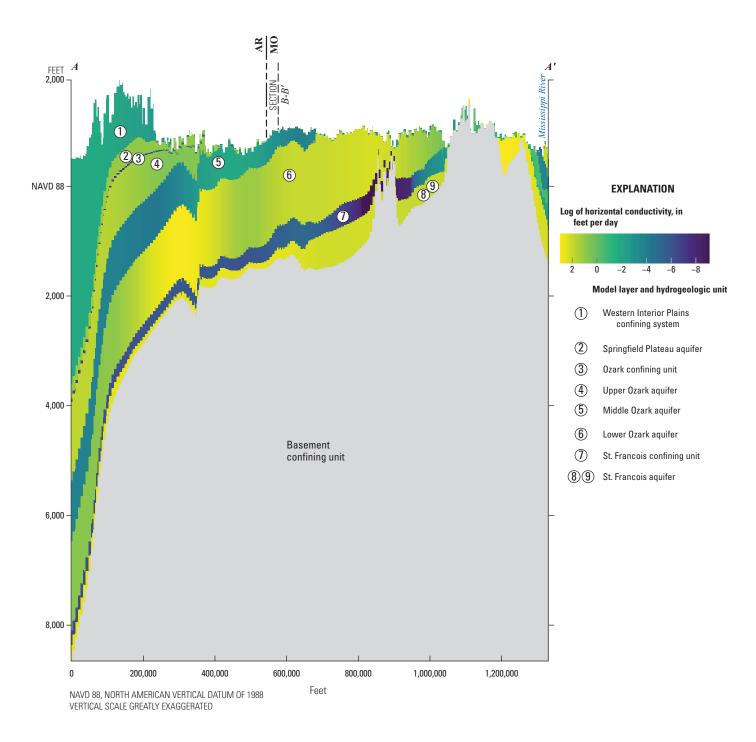


Figure 3. Hydrogeologic units of the Ozark Plateaus aquifer system (Ozark system) using the updated hydrogeologic framework from Westerman and others (2016a, b). *A, A–A'* shows the increased thickness of the Western Interior Plains confining system and dip of Ozark system units towards the southern boundary of the study area. *B, B–B'* shows the relation between Ozark system units and Western Interior Plains aquifer system units to the west and post-Paleozoic-age aquifer units to the east. Refer to figure 1 for cross-section locations.

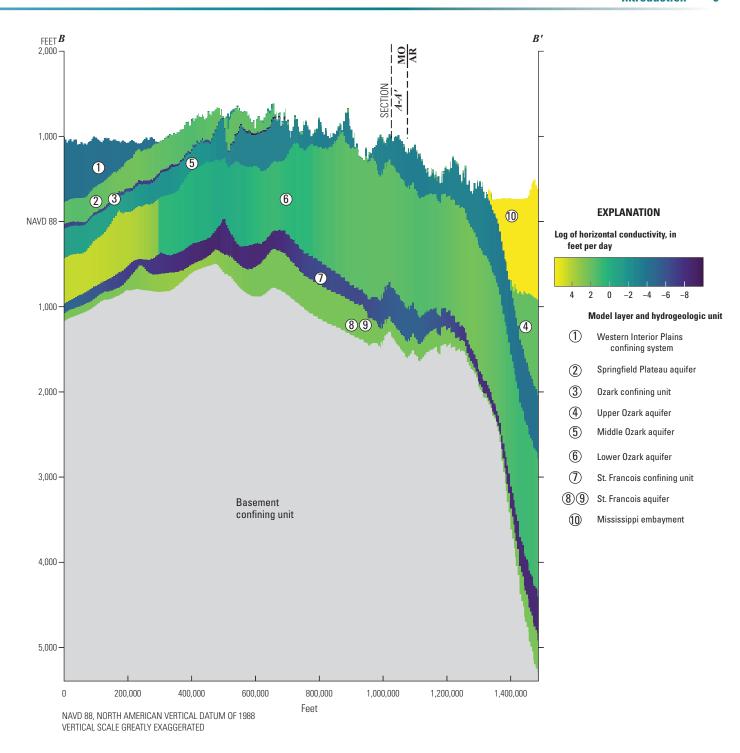


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The Ozark system is regionally overlain by the Western Interior Plains confining system in the southern and western extents of the study area (fig. 1). The confining system is mostly composed of shale with lesser amounts of limestone and sandstone. The Western Interior Plains confining system can provide a locally important water supply (especially at shallow depths of less than 300 ft where fracturing and weathering have enhanced porosity and permeability), but yields are generally low such that the unit acts regionally as a confining layer (Jorgensen and others, 1993; Westerman and others, 2016a). The unit has a median thickness of 542 ft in the study area (Westerman and others, 2016a). At the southern boundary of the study area, the Western Interior Plains confining system thickens as underlying units of the Ozark system dip steeply to the south (fig. 3).

Surface water and groundwater flow away from a central axis of uplift, which extends from the Ozark dome (located in the St. Francois Mountains in southeastern Missouri) to the tristate region of southeastern Kansas, southwestern Missouri, and southeastern Oklahoma, outwards towards the boundaries of the study area (Imes and Emmett, 1994; Jorgensen and others, 1996; Nottmeier, 2015). Extensive brittle deformation occurred with uplift (Hudson, 2000) and was followed by periods of dissolution and karst development of carbonate units (Brahana and others, 2009; Hays and others, 2016; Kresse and others, 2014). Karst processes enhanced a network of fractures and faults, creating marked permeability contrasts and aquifer anisotropy that are characteristic of the Ozark system. Primary porosity of aquifers is generally low, and secondary and tertiary porosities related to fractures and dissolution-enlarged conduits provide zones of relatively higher permeability (Hays and others, 2016). Additionally, the groundwater system is connected to surface water through ponors (natural openings in the bottom of a karst sink or basin), losing- and gaining-stream reaches, sinkholes, springs, and caves (Hays and others, 2016; Knierim and others, 2015).

Groundwater Use

Substantial refinement to USGS county-level groundwater-use estimates for the Ozark system was achieved through compilation and statistical modeling of a site-specific water-use dataset (Knierim and others, 2016, 2017). The USGS has compiled and published a water-use census for the United States every 5 years since 1950 for State-level aggregations and since 1985 for county-level aggregations (U.S. Geological Survey, 2015), but the datasets must be disaggregated to sitespecific locations if groundwater-withdrawal rates are required at a finer scale. The paucity of water-use data prior to the mid-1900s was the primary challenge for creating a site-specific groundwater-use record for the Ozark system (Knierim and others, 2017). Groundwater use likely increased in a nonlinear pattern following changes in well-drilling technology, but historical groundwater use was statistically modeled by using a linear extrapolation of groundwater-withdrawal rates from the mid-1900s back to an assumed groundwater-withdrawal

rate of 0 million gallons per day (Mgal/d) in 1900 (Knierim and others, 2017). Despite the limitations in available data, the refined water-use record combined site-specific groundwater-withdrawal rates and USGS county-level groundwater-use estimates into a 111-year dataset that reflected realistic pumping from 1900 through 2010 from the hydrogeologic units that compose the Ozark system (Knierim and others, 2016, 2017). Yearly groundwater-withdrawal rates from Knierim and others (2016, 2017) were adjusted during 6-month model stress periods, April through September (representing spring and summer) and October through March (representing fall and winter), to reflect seasonal groundwater use observed by Wittman and others (2003) (Clark and others, 2018).

Groundwater is withdrawn from the Ozark system for public supply, domestic, agricultural (including irrigation and aquaculture), and livestock use and for non-agricultural use (including industrial, thermoelectric power generation, mining, and commercial) (fig. 4B). Groundwater withdrawals totaled approximately 380 Mgal/d in 2010 (Knierim and others, 2017; U.S. Geological Survey, 2015). The Ozark system provides an important drinking-water supply for people living in the Ozark Plateaus because public supply and domestic use combined constitute the largest groundwater use, totaling 243 Mgal/d in 2010, or approximately 64 percent of the total groundwater used that year (fig. 4B) (Knierim and others, 2017). The patterns of groundwater use generally reflect land use across the Ozark Plateaus, which is predominantly forest (48 percent) and agriculture (40 percent), with localized urban development (6 percent) (Hays and others, 2016). Groundwater used for agriculture and livestock totaled approximately 107 Mgal/d in 2010 (22.4 percent for agriculture and 5.7 percent for livestock, fig. 4B) and tended to include many relatively smaller withdrawals in hay, pasture, and cropland areas (Knierim and others, 2016, 2017). Non-agricultural use totaled approximately 31 Mgal/d, or approximately 8 percent of the total groundwater use (fig. 4B) and tended to include relatively larger withdrawals in urban or mining areas (Knierim and others, 2016, 2017). Missouri, the State that covers the most area within the Ozark Plateaus (fig. 1), had the greatest amount of groundwater use in the study area in 2010 (fig. 4A).

In 2010, most groundwater used (55 percent) was withdrawn from the lower Ozark aquifer (fig. 4C), which also included 57 percent of the wells used for modeling groundwater use (Knierim and others, 2017), and is a regionally important aquifer within the Ozark system (Hays and others, 2016; Imes and Emmett, 1994; Miller and Vandike, 1997). Although each State withdrew groundwater primarily from the lower Ozark aquifer, the proportion varied by region, with the area of Kansas within the Ozark system study area withdrawing the greatest proportion of the State total from the lower Ozark aquifer and Arkansas withdrawing the least (fig. 4E). Groundwater withdrawals were second highest from the middle Ozark aquifer (20.9 percent of the total, fig. 4C), with larger proportions of withdrawals in Missouri (22.2 percent of the State total, fig. 4E) and Arkansas (17.1 percent of the State total, fig. 4E).



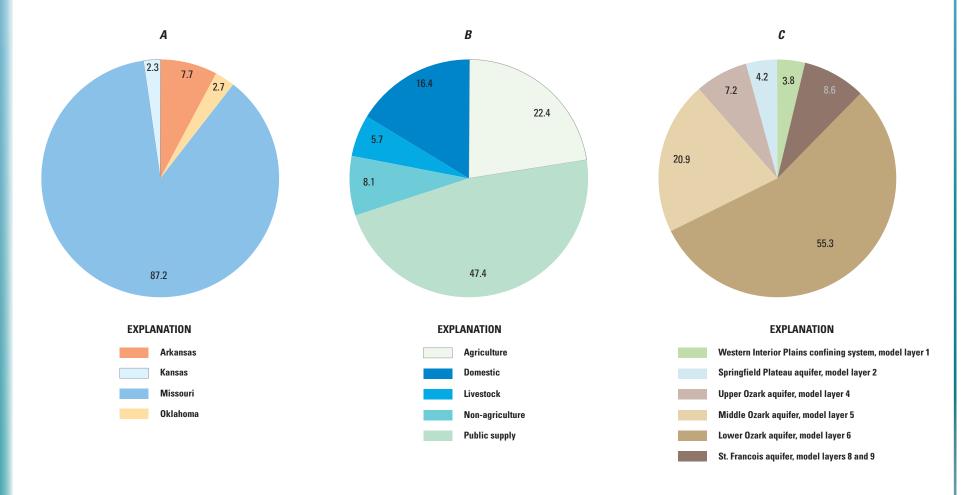


Figure 4. Modeled groundwater use in 2010 from the Ozark Plateaus aquifer system by *A*, State, *B*, water-use division, and *C*, hydrogeologic unit and model layer. Water use is also shown for each State in the study area by *D*, water-use division and *E*, hydrogeologic unit and model layer. Groundwater-withdrawal rates from Knierim and others (2016); explanation of methods available from Knierim and others (2017).

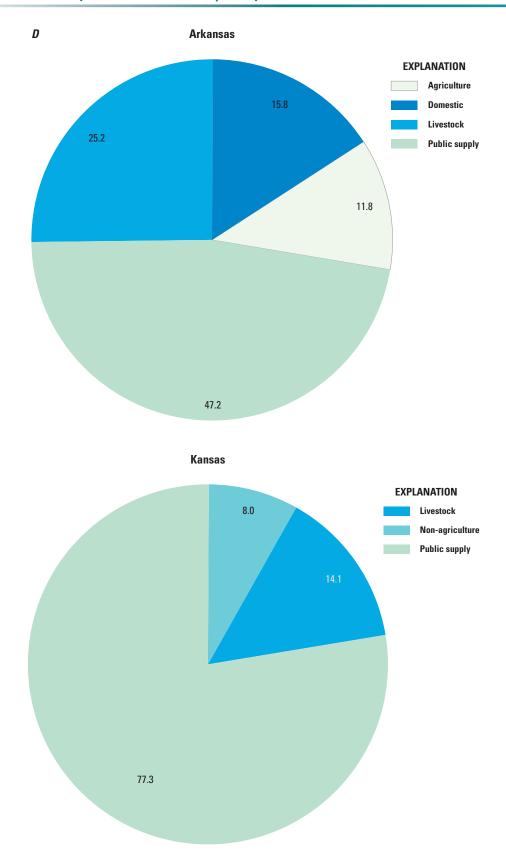


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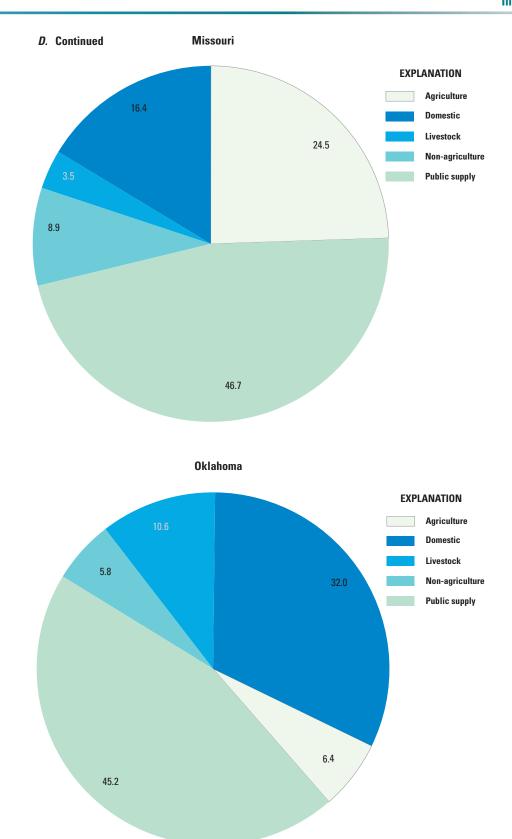


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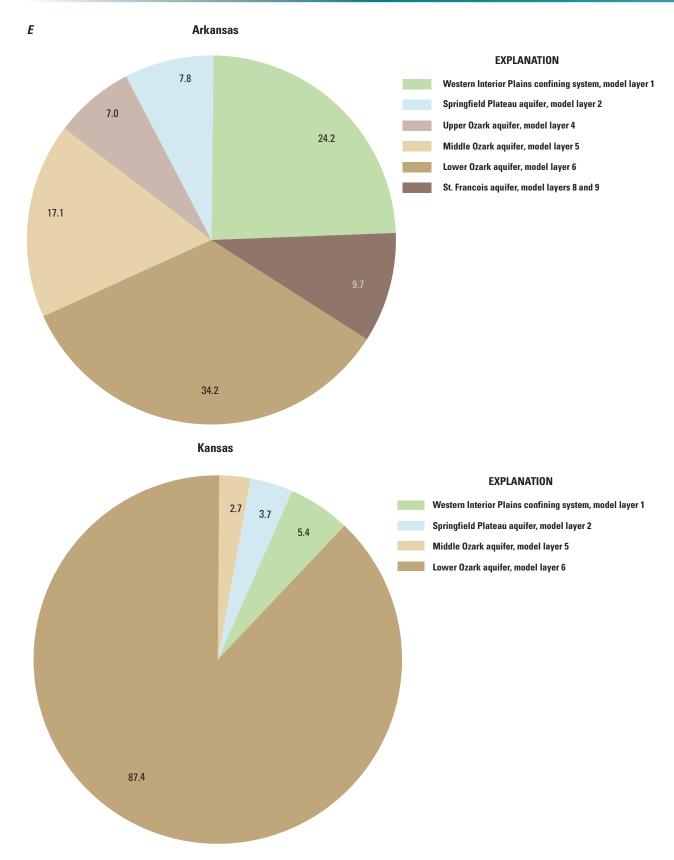


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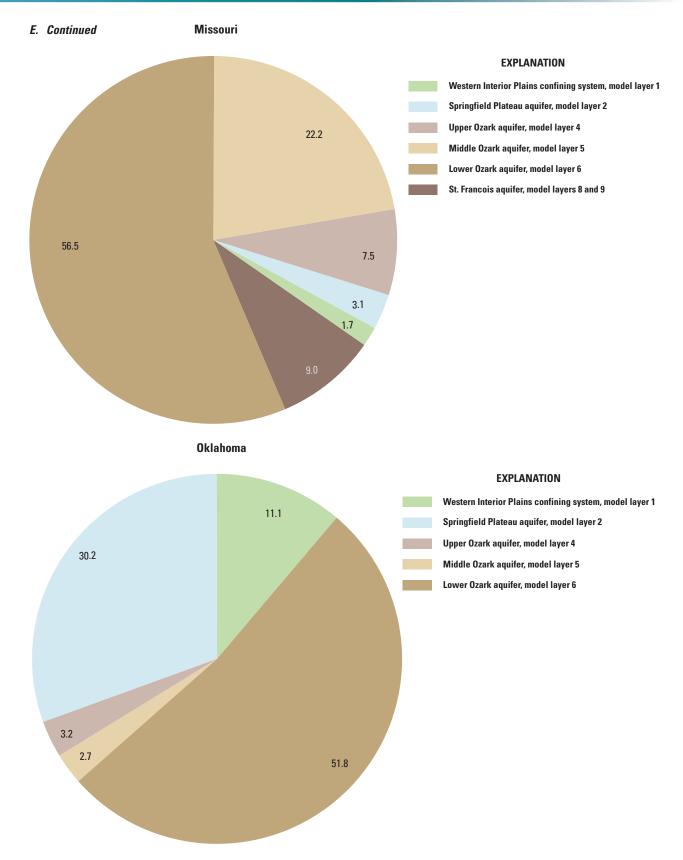


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The middle Ozark aquifer was previously conceptualized as a confining unit owing to low-permeability dolostone (Hays and others, 2016); however, the calibrated groundwater-flow model included horizontal conductance values within the same range as those of the upper Ozark aquifer (Clark and others, 2018), and groundwater use was second only to the lower Ozark aquifer (fig. 4*C*). The middle Ozark aquifer therefore serves as an important groundwater resource, despite lower permeability than some of the more karstified units such as the Springfield Plateau aquifer or the lower Ozark aquifer.

The statistically modeled site-specific water-use record had a relatively high uncertainty of 38 percent based on nonlinear statistical methods (Knierim and others, 2017). Similar to other groundwater-flow models, water use remains a challenging component of the hydrologic budget (Clark and others, 2013), but is of paramount importance to better quantify the availability of water resources. Of particular challenge in the Ozark system, groundwater use is a small component of the regional hydrologic budget (Hays and others, 2016), but groundwater withdrawals from carbonate units with low primary porosity and storage can create steep, localized cones of depression around pumping centers (Richards, 2010). Therefore, correctly locating pumping wells within model cells can be difficult and affects the position and magnitude of modeled drawdown.

Methods

The Ozark groundwater-flow model version 1.0 (v1.0) (Clark and others, 2018), also archived in Duncan and others (2018) has been modified for this report, hereinafter referred to as model version 1.1 (v1.1) and archived in Duncan and others (2019). The model v1.1 was created to evaluate the conceptual groundwater flow of predevelopment and changes in postdevelopment groundwater flow, effects of climate variability, and a groundwater monitoring network. The scenarios described in this report extend the model simulation through September 2060 and use forecasted precipitation based on multiple global circulation models and increases in pumping based on historical trends. The model v1.1 was used to evaluate existing monitoring wells as an example of data-worth analysis through the use of model prediction uncertainty (Doherty, 2016; Fienen and others, 2010), as implemented in PEST++ (Welter and others, 2015; White and others, 2015). Such monitoring networks are critical indicators of current, past, and future conditions of a groundwater resource.

Data Compilation

Data compilation for construction of the numerical groundwater-flow model used in this report began in 2013 (Clark and others, 2018). Data compilation efforts were focused on six main components: (1) the hydrogeologic framework, (2) groundwater pumpage (water use), (3) hydraulic-head

observations, (4) surface-water flows, (5) aquifer properties, and (6) net recharge. The database of information used to construct the hydrogeologic framework represented in model v1.0 and v1.1 includes lithologic, geophysical, driller description, and well-cutting logs (Westerman and others, 2016a, b). Groundwater-pumpage data included reported, estimated, and trend analysis of groundwater pumpage for as much as 100 years of data distributed to more than 140,000 groundwater-well locations (Knierim and others, 2017). Hydraulic-head data for model v1.0 and v1.1 consisted of more than 19,000 groundwater-level altitudes from the USGS National Water Information System (USGS, 2015). Computation of streamflow or stream-seepage values were evaluated from 81 named streams (Clark and others, 2018; Knierim and others, 2015). Aquifer properties for each hydrogeologic unit were evaluated based on available aquifer test information, literature values for similar hydrogeologic units, or previous groundwater-model studies. Net recharge to the outcrop areas of all hydrogeologic units was assigned initially through use of the Soil-Water-Balance (SWB) model (Westenbroek and others, 2010), augmented by estimates from the Empirical Water Balance method (Reitz and others, 2015, 2017), and refined through model calibration.

Numerical Model

A transient groundwater model v1.1 was developed for the Ozark system by using the modular three-dimensional finite-difference MODFLOW-NWT code (Niswonger and others, 2011) and calibrated by using PEST++ (Welter and others, 2015). The model consists of horizontally uniform 1-mi² cells of variable thickness. The finite-difference grid is oriented north-south and consists of 324 rows, 335 columns, and 9 layers. Hydraulic properties, such as conductivity and storage, vary by model cells areally and vertically to represent changes in aquifer properties owing to lithology (table 1.1 in appendix 1). The model simulates 116 years (January 1, 1900 to April 1, 2016) of system response divided into 79 stress periods to quantify groundwater resources, evaluate how groundwater resources have changed over time, and forecast the responses of the aquifer system to future stresses. The latter 50 stress periods, beginning around 1991, represent seasonal changes in recharge and pumping. As previously noted, these seasonal effects are important at local scales where short-term drought can result in large groundwaterlevel declines that are of concern to water managers.

The digital archive of the model includes an additional stress period at the end of the calibration period. This stress period is simulated as steady state and was included for preliminary evaluation of forecast uncertainty in model v1.0 (Clark and others, 2018), prior to development of the three scenarios used to quantify parameter and predictive uncertainty and data-worth and that are presented herein. The final stress period and associated forecast observations, therefore, are not included as part of this report.

Ozark system boundaries were represented as no-flow, general-head, or constant-head boundaries (fig. 1), depending on knowledge about groundwater interaction among adjacent systems. The top of the Basement confining unit is the lower boundary of the model and is represented as a no-flow boundary because the Basement confining unit generally has permeability that is orders of magnitude lower than that of the overlying aquifer system. Most boundaries at the edges of the study area were also represented as no-flow boundaries (fig. 1). Seepage-run studies indicate that the Ozark system loses groundwater to surface water (Knierim and others, 2015). Streams, therefore, are important hydrologic features in this system and were represented as head-dependent flux boundaries in the model by using the River Package (Harbaugh, 2005); streams generally serve as net sinks, or losses of water, from the Ozark system. Groundwater flow to and from adjacent systems is represented as a general-head boundary towards the southeastern part of the study area (fig. 1) (where groundwater exchanges between the Ozark system and Cretaceous- and Tertiary-age units of the Mississippi embayment), and is represented as a constant-head boundary at the western part of the study area (fig. 1) (where fresh and saline water mix at the freshwater-saltwater transition zone).

The western boundary coincides with a topographic lowland that is influenced by salt water from the Western Interior Plains aquifer system. The dense saltwater plunges beneath the freshwater flowing out from the study area, and mixing of freshwater and saltwater occurs at the western margins of the Ozark system as evidenced by saline springs, streams, and groundwater (Hays and others, 2016). The freshwater-saltwater transition zone along the western extent of the study area (fig. 1) is not modeled explicitly, but is represented as a constant-head boundary (additional detail in Clark and others, 2018) to allow indirect assessment of groundwater flow across the interface.

Modifications to the Numerical Model

Model v1.1 is a modified version of model v1.0, documented by Clark and others (2018) and Duncan and others (2018) to improve the simulation of groundwater flow. The datasets for the modified model v1.1 can be found online (Duncan and others, 2019). These adjustments to model v1.0 included increased density of pilot points in selected areas and more rigorous parameter estimation through focused observation weighting and extended iterations with PEST++. Because of the distribution of pumping stresses affecting local areas of interest, the density of pilot points was increased from a spacing of 15 miles (mi) to 3 mi in areas of Springfield, Joplin, and Noel, Mo., and Miami, Okla. (fig. 1). Head observations in central Missouri and Oklahoma collected after 1995 were given a higher weight than other observations. The parameter estimation process was also allowed to progress further than during previous attempts after it was recognized that nonlinear parameter derivatives may have been affecting

the ability of PEST++ to properly calculate the optimal set of parameters. These modifications produced a better fit to head and streamflow leakage observations, though some parameter values were forced beyond the range of what is thought to be reasonable values for the system. These parameters were manually adjusted to lie within a range of more reasonable values (based on aquifer properties), without substantially adversely affecting the fit to observed values.

Additional manual changes included replacing aquifer properties in layer 1 in the northern part of the Mississippi embayment and reducing recharge in stress periods 2 and 3. In model v1.0 (Clark and others, 2018), an area north of the embayment represents the Western Interior Plains confining system. In model v1.1, it was decided that much of that area would be better represented as part of the embayment. Recharge was reduced in stress periods 2 and 3 by setting it equal to the stress period 1 rate to better align with the conceptual understanding of the system. Without this reduction, storage would have increased in the system from 1900 to 1965 resulting in a slight rise in water levels, likely the result of early, lesser quality observations influencing the recharge parameters.

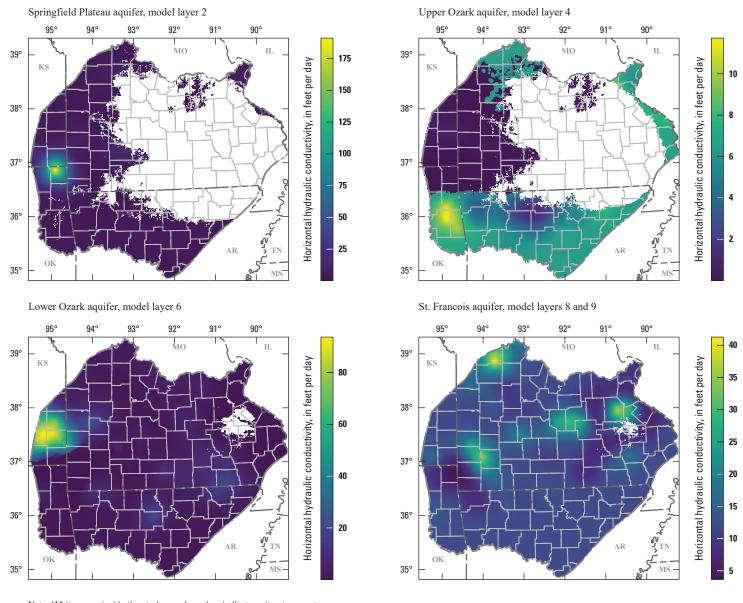
Two of the more substantive changes were made to the hydraulic conductivity values of the Springfield Plateau and lower Ozark aquifers (fig. 5). The maximum horizontal hydraulic conductivity of the Springfield Plateau aquifer increased from 6.48 to 191 feet per day (ft/d) in northeastern Oklahoma. Though this value appears high compared to the horizontal hydraulic conductivity values for the rest of the aguifer, northeastern Oklahoma is an area of intense mining that modified the aquifer structure, thereby changing the aquifer properties of the unit and greatly increasing the permeability on a local scale (Czarnecki and others, 2009). The maximum horizontal hydraulic conductivity of the lower Ozark aquifer increased to 93 ft/d in eastern Kansas. The changes in Kansas were driven completely by the automated parameter estimation procedure, yet the higher hydraulic conductivity values appear to correspond with previous evidence of higher transmissivity values in the area (Macfarlane and others, 2005; Reed and Burnett, 1985).

Simulated heads were generally in good agreement with observed hydraulic heads, with 66 percent of simulated values within 60 ft of the observed value, which is about 2 percent of the total range in head (2,578 ft). Simulated heads were computed for 19,044 observed hydraulic-head measurements from 6,682 wells within the Ozark model area. Values of mean, minimum, maximum, root mean square error (RMSE), and mean absolute error were computed from residuals for each year from 1900 through 2013 (table 1). RMSE, in feet, is determined by using the equation

$$RMSE = \frac{\sqrt{(h_o - h_s)^2}}{n} \tag{1}$$

where

 h_o is observed hydraulic head, in feet; h_s is simulated hydraulic head, in feet; and is number of observations.



Note: White areas inside the study area boundary indicate unit not present

Figure 5. Horizontal hydraulic conductivity of select model layers within the Ozark model area.

 Table 1.
 Summary of hydraulic-head residual statistics for calibration of the Ozark groundwater-flow model, version 1.1.

 $[RMSE, root\ mean\ square\ error; ---, no\ value]$

Year	Mean, in feet	Minimum residual, in feet	Maximum residual, in feet	RMSE, in feet	Mean absolute error, in feet	Number of observations	Range, in feet	Ratio of RMSE to range
1900	2	2	2	2	2	1	0	_
1905	-46	-109	-6	64	46	3	580	0.11
1906	-62	-62	-62	62	62	1	0	_
1907	-17	-96	62	81	79	2	7	11.53
1908	-21	-21	-21	21	21	1	0	_
1910	-16	-16	-16	16	16	1	0	_
1911	-22	-22	-22	22	22	1	0	_
1912	-26	-72	-3	42	26	3	175	0.24
1913	-59	-59	-59	59	59	1	0	_
1914	-75	-303	91	183	136	3	152	1.2
1915	-44	-181	44	81	59	6	709	0.11
1916	-145	-205	-88	152	145	5	445	0.34
1917	-72	-85	-51	74	72	3	59	1.25
1918	-19	-31	-7	23	19	2	89	0.25
1921	-72	-72	-72	72	72	1	0	_
1922	76	76	76	76	76	1	0	_
1923	-49	-95	24	71	65	3	321	0.22
1924	-28	-72	16	46	39	3	211	0.22
1925	-9	-69	40	46	37	3	488	0.09
1926	-71	-174	52	98	83	11	915	0.11
1927	-82	-133	-32	97	82	2	164	0.59
1928	20	-11	69	40	27	3	860	0.05
1929	-41	-132	-16	58	41	6	745	0.08
1930	-21	-231	58	100	66	6	447	0.22
1931	-21	-102	86	67	52	9	908	0.07
1932	4	-39	101	43	33	8	829	0.05
1933	3	-97	137	53	40	19	886	0.06
1934	-35	-183	117	71	48	34	931	0.08
1935	-18	-110	83	49	40	35	946	0.05
1936	-6	-223	110	53	41	167	1,024	0.05
1937	6	-99	217	59	43	71	1,214	0.05
1938	-12	-230	217	87	65	89	1,235	0.07
1939	-8	-207	192	69	50	92	1,021	0.07
1940	-8	-251	179	68	51	112	889	0.08
1941	-5	-237	203	75	54	101	1,024	0.07
1942	-30	-327	130	87	62	121	1,197	0.07
1943	-37	-273	142	85	66	41	822	0.1
1944	-30	-309	126	98	78	27	846	0.12
1945	-19	-303	99	79	52	26	989	0.08

Table 1. Summary of hydraulic-head residual statistics for calibration of the Ozark groundwater-flow model, version 1.1.—Continued [RMSE, root mean square error; —, no value]

Year	Mean, in feet	Minimum residual, in feet	Maximum residual, in feet	RMSE, in feet	Mean absolute error, in feet	Number of observations	Range, in feet	Ratio of RMSE to range
1946	-10	-269	186	77	56	75	1,053	0.07
1947	-22	-260	149	72	52	75	1,030	0.07
1948	-40	-270	92	86	62	66	956	0.09
1949	12	-215	663	141	68	46	1,948	0.07
1950	82	-204	555	248	156	58	1,478	0.17
1951	60	-223	557	203	109	64	1,327	0.15
1952	-60	-279	170	101	75	46	1,071	0.09
1953	-17	-272	769	170	95	60	1,596	0.11
1954	-59	-297	232	97	73	112	1,222	0.08
1955	-39	-341	208	93	66	166	1,195	0.08
1956	-53	-313	126	102	69	141	1,159	0.09
1957	-65	-300	198	116	81	163	1,065	0.11
1958	-42	-465	213	96	70	122	1,166	0.08
1959	-32	-266	318	86	62	135	1,096	0.08
1960	1	-307	284	81	58	316	1,193	0.07
1961	2	-356	459	98	68	270	1,509	0.06
1962	35	-227	466	122	74	381	1,342	0.09
1963	3	-270	473	95	61	261	1,365	0.07
1964	11	-238	271	76	57	451	1,236	0.06
1965	-4	-224	357	74	54	400	1,633	0.05
1966	-17	-251	162	68	51	252	1,149	0.06
1967	-17	-198	215	70	56	244	1,164	0.06
1968	28	-631	771	127	77	213	1,717	0.07
1969	-37	-500	189	119	73	130	1,157	0.1
1970	-19	-227	140	67	53	151	1,119	0.06
1971	-27	-343	132	81	61	118	1,046	0.08
1972	-24	-288	162	77	56	128	1,039	0.07
1973	-29	-312	163	76	57	230	1,103	0.07
1974	-48	-320	201	101	71	203	1,037	0.1
1975	-24	-293	373	79	48	197	1,528	0.05
1976	1	-244	142	45	28	184	1,100	0.04
1977	-21	-204	333	63	42	221	1,178	0.05
1978	-37	-314	108	67	47	285	1,119	0.06
1979	-39	-348	144	72	51	165	1,034	0.07
1980	-32	-313	517	88	57	165	1,441	0.06

Table 1. Summary of hydraulic-head residual statistics for calibration of the Ozark groundwater-flow model, version 1.1.—Continued [RMSE, root mean square error; —, no value]

Year	Mean, in feet	Minimum residual, in feet	Maximum residual, in feet	RMSE, in feet	Mean absolute error, in feet	Number of observations	Range, in feet	Ratio of RMSE to range
1981	-53	-312	91	72	60	482	995	0.07
1982	-56	-305	71	72	61	526	1,039	0.07
1983	-33	-202	194	70	56	146	827	0.09
1984	-54	-209	95	74	62	146	921	0.08
1985	-57	-210	64	76	63	166	995	0.08
1986	-64	-229	99	87	71	173	980	0.09
1987	-47	-317	605	97	72	276	1,845	0.05
1988	-34	-181	641	98	67	156	1,690	0.06
1989	-29	-309	408	84	60	248	1,570	0.05
1990	-17	-235	706	85	58	303	1,853	0.05
1991	2	-207	706	87	59	303	1,720	0.05
1992	10	-217	408	87	65	411	1,503	0.06
1993	18	-246	403	88	65	371	1,550	0.06
1994	-1	-173	190	64	47	77	1,006	0.06
1995	-12	-358	272	57	41	652	1,057	0.05
1996	-7	-233	189	74	54	125	1,133	0.07
1997	15	-126	120	52	37	58	1,057	0.05
1998	7	-258	217	65	46	346	1,049	0.06
1999	7	-194	169	60	47	189	665	0.09
2000	4	-265	248	83	60	658	926	0.09
2001	11	-786	285	67	43	628	1,795	0.04
2002	3	-276	155	55	38	738	1,169	0.05
2003	8	-264	215	56	40	765	1,130	0.05
2004	-17	-793	280	78	46	352	1,616	0.05
2005	-13	-257	90	59	41	298	1,041	0.06
2006	-18	-453	287	81	55	623	1,074	0.08
2007	-20	-633	211	71	49	452	1,547	0.05
2008	-33	-275	93	70	52	302	1,105	0.06
2009	-23	-221	92	63	47	440	1,089	0.06
2010	-30	-655	204	74	52	507	1,562	0.05
2011	-29	-263	106	70	53	396	1,111	0.06
2012	-34	-233	119	74	56	349	1,109	0.07
2013	-21	-242	85	70	54	64	846	0.08
all	-15	-793	771	80	55	19,044	2,578	0.03

The modifications made in model v1.1 generally improved the model fit to observed values. For example, the RMSE among all (19,044 comparisons) simulated and observed hydraulic heads is 80 in model v1.1 compared to 113 in model v1.0 (Clark and others, 2018). Additionally, the comparison of simulated stream leakage to estimated leakage improved from a coefficient of determination (R^2) of 0.59 to 0.81. Additional information regarding updated parameter values and the fit of the model to observed data is contained in appendix 1.

Conceptualization of the Hydrologic System

The Ozark system is conceptualized as a hydrologic budget, with inflows from precipitation and losing-stream reaches and lateral inflow from neighboring surface-water and groundwater systems (Hays and others, 2016). Outflows are to gaining-stream reaches and springs, lateral groundwater flow to neighboring systems, and withdrawals for water use (Hays and others, 2016). The Ozark system is assumed to be at or near hydrologic equilibrium, so net gains and losses of water, and thus the hydrologic budget, are balanced. When there is a surplus of recharge, the additional water can flow into storage, numerically representing a loss of groundwater from the aquifer system, which is represented by negative values in the hydrologic budget. Likewise, when there is a deficit of recharge, water released from storage provides a source of water to the aquifer system, which is represented by positive values in the hydrologic budget. Thus, groundwater storage can serve as both an inflow and outflow to the groundwater system. As water in storage is depleted, the water level will decline until an additional inflow is introduced or until the area dries up. Evaluation of areas where the inflows and outflows do not balance can indicate where stresses exist or are developing. Additionally, there may be both inflow and outflow in localized parts of the system for a single component of the hydrologic budget, but budget components for the entire aquifer system are conceptualized as net fluxes, where the overall flux will have a net positive (inflow) or negative (outflow) value. Predevelopment conditions include the period prior to 1900 when steady-state conditions were achieved through outflows to streams, springs, and neighboring systems; after 1900, outflows additionally included groundwater withdrawals (Knierim and others, 2017) under transient conditions and associated changes in aquifer storage (fig. 6).

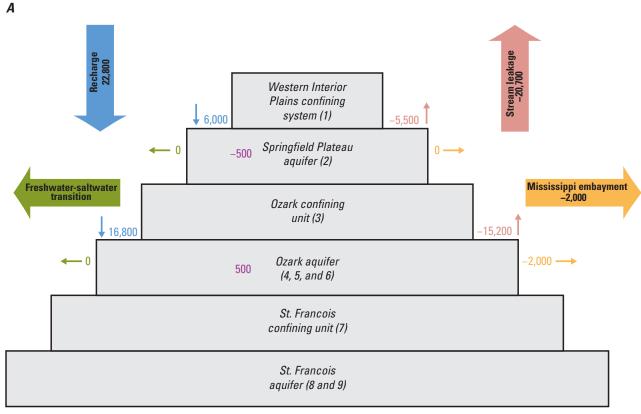
Recharge from precipitation is the largest source of freshwater to the Ozark system. Average annual precipitation varies from 28 to 63 inches per year (in/yr) in the study area, with an average of 44 in/yr (Hays and others, 2016). Most rainfall occurs during April, May, and June, and precipitation increases generally from north to south across the study area (Hays and others, 2016). Approximately 34,560 Mgal/d of

water, or 24 percent of precipitation in the area, recharges the Ozark system according to an SWB model developed for the Ozark system for the period 2005 to 2014 (Hays and others, 2016). Based on land-surface area for aquifers of the Ozark system, 66 percent (22,800 Mgal/d) of the total recharge contributes to the Springfield Plateau and upper, middle, and lower Ozark aquifers (fig. 6*A*) (Hays and others, 2016). Recharge was found to be variable across the Ozark Plateaus, depending on variables in the SWB model, including precipitation, temperature, vegetation, and soil properties (Westenbroek and others, 2010).

Because of the dome-like structure of the Ozark system, lateral groundwater and surface-water inflow are hypothesized to be small and there is a net volume of groundwater outflow at the boundaries of the Ozark system (fig. 6). In the western part of the study area, groundwater mixing occurs at the freshwater-saltwater transition zone (fig. 1), such that saline groundwater from the Western Interior Plains aquifer system contributes water to surface water and groundwater at the margins of the Ozark system (Hays and others, 2016). At the scale of a regional groundwater-flow model, this contribution is negligible and therefore not included as a net inflow for the conceptual Ozark system hydrologic budget. Although groundwater pumping could induce flow from the saline groundwater system in areas along the freshwater/saltwater transition zone (Czarnecki and others, 2009; Macfarlane and others, 2005), quantifying localized groundwater flow between the Ozark system and Western Interior Plains aquifer system is more suited to studies done at finer scales than represented in this regional model.

Groundwater outflow occurs along the western and eastern margins of the study area. Approximately 2,000 Mgal/d (or about 9 percent of recharge) of groundwater flows to neighboring groundwater systems and streams (Hays and others, 2016; Imes and Emmett, 1994; Mesko and Imes, 1995). Based on previous modeling efforts (Mesko and Imes, 1995), most of the groundwater outflow occurs along the eastern margins of the study area where Paleozoic-age units of the Ozark system are in hydraulic connection with Tertiary-age units of the Mississippi embayment system (figs. 6*A* and *B*).

Groundwater-surface-water interaction is common in the Ozark system, with highly transmissive fractured karst conduits providing strong connections between groundwater and surface water. Streams alternate between gaining (receiving water from the groundwater system) and losing (losing water to the groundwater system) over relatively short distances as a function of time depending on precipitationdriven groundwater flow and river stage (Hays and others, 2016; Knierim and others, 2015). Springs also contribute to surface-water flow throughout the Ozarks. Analysis of the seepage-run dataset spanning 24 years (Knierim and others, 2015) indicated that, in total, interior Ozark system streams (that is, streams that are not at major boundaries of the Ozark system) are gaining and, gaining flows combined with springflow, receive approximately 20,700 Mgal/d from groundwater (figs. 6A and B) (Hays and others, 2016).



VALUES MAY NOT SUM TO REPORTED TOTALS DUE TO ROUNDING

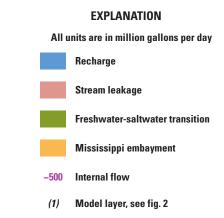
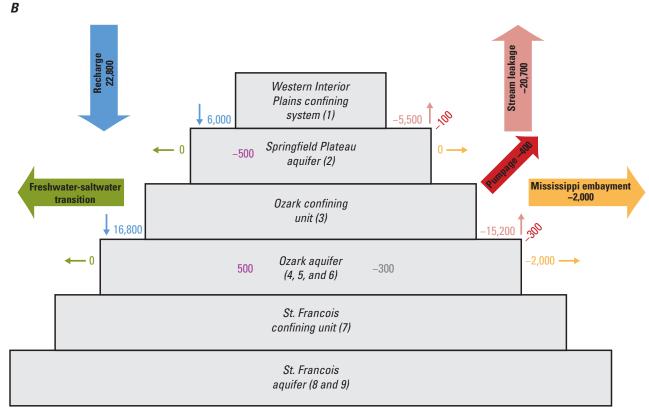


Figure 6. Hydrologic budget for conceptual model of Hays and others (2016) for *A*, predevelopment and *B*, postdevelopment conditions and numerical analysis in this report of *C*, predevelopment (before 1900) and *D*, average postdevelopment (April 1, 1996–October 1, 2015) conditions. Negative values indicate water discharged from the hydrogeologic unit; positive values represent water recharged to the hydrogeologic unit.



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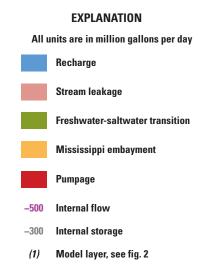
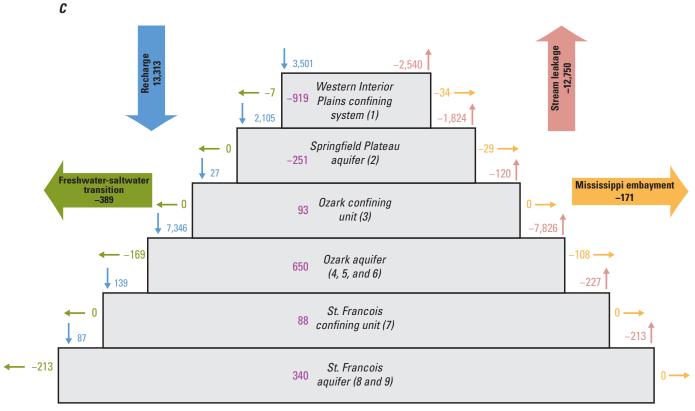


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VALUES MAY NOT SUM TO REPORTED TOTALS DUE TO ROUNDING

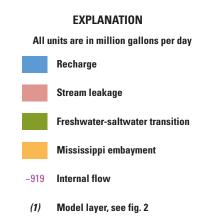
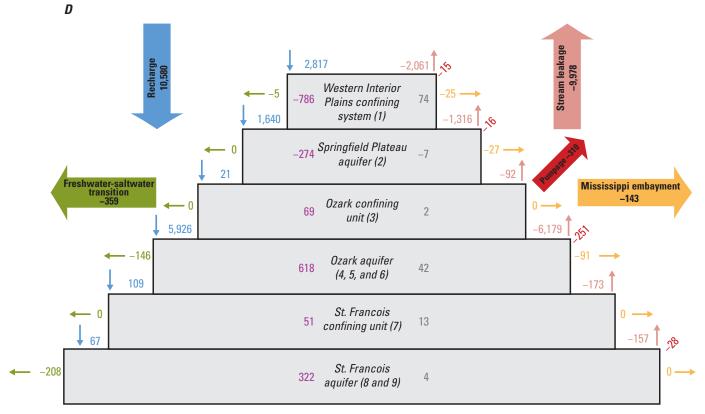


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VALUES MAY NOT SUM TO REPORTED TOTALS DUE TO ROUNDING

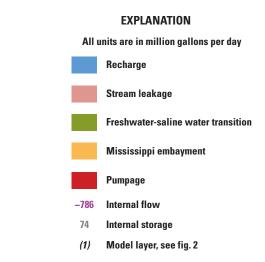


Figure 6. Hydrologic budget for conceptual model of Hays and others (2016) for *A*, predevelopment and *B*, postdevelopment conditions and numerical analysis in this report of *C*, predevelopment (before 1900) and *D*, average postdevelopment (April 1, 1996–October 1, 2015) conditions. Negative values indicate water discharged from the hydrogeologic unit; positive values represent water recharged to the hydrogeologic unit.—Continued

Therefore, about 91 percent of recharge to the Ozark system is returned to the surface through springs or streambed discharge. Groundwater maintenance of streams is important for ecosystems in the Ozark system because groundwater supports a variety of habitats and species and provides a time-averaged input of water to streams (Hays and others, 2016).

The 400 Mgal/d of groundwater withdrawn from the Ozark system in 2010 (fig. 6B) was approximately 2 percent of the total recharge rate. Because of low storage in aquifers, cones of depression with steep water-level gradients can develop quickly around pumping centers (Hays and others, 2016; Richards, 2010). Therefore, even though a pumping rate is small from a regional perspective, aquifer characteristics are such that high rates of local pumping have the potential to locally create shortages of water during even short periods of drought. Potentiometric maps of groundwater levels in the Ozark system with increased density of measured wells around urban pumping zones indicates localized cones of depression (Richards and Mugel, 2008) compared to regional potentiometric maps that are used to characterize the regional groundwater-flow system (Nottmeier, 2015). Therefore, site-specific water use on a scale finer than the Ozark system model grid may be important for accurately characterizing groundwater availability in urban areas.

Simulation of the Hydrologic System

Model v1.1 was used to assess changes in simulated hydrologic budget components from January 1, 1900, to April 1, 2016, at the regional scale to quantify hydrologic changes across the Ozark system. Model v1.1 was also used to simulate the potential future conditions for 2016–2060 under the baseline, pumping, and general circulation model (GCM) scenarios. While scenario conditions are simulated in an approximate manner, the groundwater-flow model results can be used to gain insights into the range of possible futures, allowing planning by resource managers and identification of future research needs.

Comparison of the Conceptualized and Simulated Hydrologic Systems

Comparing the conceptual hydrologic budget (Hays and others, 2016) with the numerical groundwater-flow model v1.1 budget provides a way to evaluate current understanding of the aquifer system in the context of a numerical solution to groundwater flow. Conceptual hydrologic budget components were estimated by Hays and others (2016) using previous groundwater modeling efforts, field research into groundwater—surface-water interaction, a water-use model based on reported groundwater withdrawals, and the component of precipitation gained as recharge from a SWB model. The numerical groundwater-flow model refines these estimates and improves the understanding of

regional groundwater flow and water availability in the Ozark system. Groundwater fluxes simulated in model v1.1 include predevelopment and average postdevelopment conditions for comparison to the conceptual model budget. The postdevelopment period discussed throughout the report spans from April 1, 1996, to April 1, 2016, and is emphasized for several reasons: (1) groundwater pumping steadily increased until 2010 (Clark and others, 2018; Knierim and others, 2016, 2017); (2) estimated net pumpage stabilized around means of 422 Mgal/d and 119 Mgal/d for summer and winter stress periods, respectively, during this postdevelopment timeframe; and (3), there was no appreciable trend in simulated net areal recharge during this period.

Comparison of Predevelopment Conditions Between the Conceptualized and Simulated Hydrologic Systems

Recharge calculated for the conceptual model hydrologic budget (using SWB) was 34,560 Mgal/d for the entire Ozark system, of which 6,000 Mgal/d recharged the Springfield Plateau aquifer and 16,800 Mgal/d recharged the upper, middle, and lower Ozark aquifers (Hays and others, 2016). Although the SWB model provides a thermodynamic budget approach to calculate recharge across an entire study area, SWB results for recharge were generally high compared to other estimates compiled for the conceptual model of the Ozark system (Hays and others, 2016). To provide a more conservative estimate of recharge for initial values in the groundwater-flow model, values calculated through annual regression-based methods (referred to as the Empirical Water Balance; EWB [Reitz and others, 2015, 2017]) were substituted for SWB values during model calibration (see Clark and others [2018] for methods). Recharge from EWB grids (averaged from 2000 to 2013) was 12,994 Mgal/d for the entire system, of which 3,324 Mgal/d and 9,670 Mgal/d recharged the Springfield Plateau aquifer and upper, middle, and lower Ozark aquifers, respectively (Reitz and others, 2017). Recharge simulated in model v1.1 was lower than recharge estimated by SWB or EWB: 2,105 Mgal/d for the Springfield Plateau aquifer and 7,346 Mgal/d for the upper, middle, and lower Ozark aquifers (plus approximately 108 Mgal/d from precipitation that falls on the outcrop area of stratigraphically equivalent rocks in north-central Missouri [fig. 1] is included with recharge in fig. 6C).

Although not representing the same periods, recharge from the groundwater-flow model version 1.1 and EWB were more similar than the recharge values estimated by SWB methods even after adjustment through parameter estimation, such that EWB provides reliable, continuous estimates of recharge. Previous groundwater-flow models of the Ozark system also estimated lower recharge values compared to SWB methods that use principles similar to SWB (Imes and Emmett, 1994). Recharge calculated from SWB may be better thought of as soil drainage—or water that drains from the soil

zone—and incorporating surface geologic information into recharge calculations may provide context for the amount of soil drainage that realistically contributes recharge to underlying groundwater systems. For example, in the EWB approach, surficial geology was found to improve estimates of hydrologic budget components, including quick-flow runoff, evapotranspiration, and recharge (Reitz and others, 2017). Therefore, soil-focused approaches to estimating recharge may generally overestimate recharge compared to methods that calibrate to a balanced water budget, such as EWB methods or numerical groundwater-flow models.

In the conceptual hydrologic budget, the groundwater transferred among aquifers of the Ozark system was primarily estimated as 500 Mgal/d of vertical flow from the Springfield Plateau aquifer into units of the Ozark aquifer (Hays and others, 2016) (figs. 6A and B). One of the benefits of using a groundwater-flow model is that estimates of groundwater flux among hydrogeologic units within a groundwater system can be more easily quantified than if no model is used. Based on model v1.1, the Springfield Plateau aquifer loses approximately 251 Mgal/d, the Ozark aquifer receives 650 Mgal/d, and the St. Francois aquifer receives 340 Mgal/d from other units within the Ozark system for predevelopment conditions (fig. 6C). Groundwater fluxes among aquifers calculated with model v1.1 are different from previous regional groundwater-flow model results; Imes and Emmett (1994) calculated that 757 Mgal/d was lost from the Springfield Plateau aquifer and 190 Mgal/d was lost from the Ozark aguifer. Additionally, much more groundwater flows into the St. Francois aquifer (340 Mgal/d) in model v1.1 (fig. 6C) compared to 24 Mgal/d as reported by Imes and Emmett (1994), possibly owing to more

accurate representation of the aquifers through refined discretization (model v1.1 uses two layers to represent the St. François aquifer).

Interaction among the Ozark system and neighboring surface-water and groundwater systems was conceptualized as a net outflow, with inflows being minimal because of the geometry of the Ozark system (fig. 3*B*) (Hays and others, 2016). Lateral groundwater outflow was estimated to be 2,000 Mgal/d for the conceptual hydrologic budget (Imes and Emmett, 1994; Mesko and Imes, 1995), compared to approximately 560 Mgal/d (fig. 6*C*; table 2) of groundwater exiting the Ozark system through the freshwater-saltwater transition zone and to the east to the Mississippi embayment in model v1.1.

In model v1.1, most (389 Mgal/d) of the simulated 560 Mgal/d exiting the system as lateral groundwater flow leaves the Ozark system along the northwestern margin of the study area near the freshwater-saltwater transition zone (fig. 1). Of the 389 Mgal/d of groundwater that flows to constant head boundaries at the freshwater-saltwater transition zone, approximately 55 percent (213 Mgal/d) flows from the St. Francois aquifer and 44 percent (169 Mgal/d) flows from the Ozark aquifer. Based on an end-member mixing model using chloride data for the conceptual hydrologic budget (Hays and others 2016), up to 20 percent of groundwater in the freshwater-saltwater transition zone was contributed from the neighboring Western Interior Plains aquifer system (Hays and others, 2016). Based on simulated groundwater flow in model v1.1, mixing of groundwater in the freshwater-saltwater transition zone does not represent a net influx of water into the Ozark system because 389 Mgal/d of groundwater leaves the Ozark system through the constant-head boundary (fig. 6C).

Table 2. Hydrologic budget for the numerical groundwater-flow model for the predevelopment period (before 1900) and minimum, average, standard deviation, and maximum values for the postdevelopment period (April 1, 1996, to April 1, 2016).

[Values are in million gallons per day with positive values signifying net inflows to the groundwater system and negative values signifying net outflows]

	Time period						
Hydrologic budget component		Postdevelopment					
ilyarologio baagot component	Predevelopment	Minimum	Average	Standard deviation	Maximum		
Recharge	13,205	1,387	10,580	6,465	23,467		
Lateral groundwater outflow							
Freshwater-saltwater transition zone	-389	-377	-359	9	-346		
Embayment	-171	-239	-143	46	-73		
Discharge to streams	-12,750	-14,720	-9,978	2,013	-6,588		
Groundwater pumping	0	-934	-310	219	-65		
Aquifer storage	0		128	4,791			
Storage release		0			7,228		
Storage replenishment		-8,222			0		

The remainder of net groundwater outflow (171 Mgal/d) occurs through the unconsolidated units of the Mississippi embayment on the eastern margin of the study area (fig. 3B), including the McNairy-Nacatoch aquifer of Cretaceous-age and Tertiary-age units. Approximately 20 percent (34 Mgal/d) of the remaining 171 Mgal/d discharges from the Western Interior Plains confining system near the fall line (fig. 1), and 63 percent (108 Mgal/d) discharges from the Ozark aquifer (fig. 6C). However, discharge from the individual hydrogeologic units of the Ozark system to the Mississippi embayment is best considered as the summed value (171 Mgal/d) because of limitations in the hydrogeologic framework at system boundaries. Groundwater outflow to the Mississippi embayment may be overestimated because of the simulated direct connection between Ozark system aquifers and the Mississippi embayment in model v1.1. The direct hydraulic connection is an artifact of model construction; parts of the Western Interior Plains confining system, Springfield Plateau, Ozark confining unit, and Ozark aquifer layers are replaced by properties representing the Mississippi embayment in the southeastern model area. Groundwater flow into the general-head boundary through the Mississippi embayment could also represent discharge into streams, as was shown through seepage studies of streams along the fall line (Mesko and Imes, 1995). The interaction of groundwater between Paleozoic-age units of the Ozark system and Cretaceous-age and younger units of the Mississippi embayment remains an important question because of the high amount of groundwater use from alluvial aquifers and limitations in groundwater-flow models at model boundaries.

Groundwater discharge to streams and springs was conceptualized as a major loss of water from the Ozark system, accounting for approximately 91 percent of recharge (Hays and others, 2016). Based on seepage-run studies, largemagnitude spring discharges, and balance among hydrologic budget components, approximately 20,700 Mgal/d of water moved from Ozark system aquifers to streams (figs. 6A and B) according to the conceptual model of Hays and others (2016). Based on the model v1.1, 12,750 Mgal/d discharged to streams across the Ozark system (excluding the Mississippi embayment area), accounting for 95 percent of simulated recharge (13,205 Mgal/d) during the predevelopment period (fig. 6C). Streams overlying the outcrop area of the Springfield Plateau aquifer (fig. 1) received 1,824 Mgal/d from groundwater or approximately 87 percent of recharge to the aquifer (fig. 6C). The Ozark aquifer discharged more groundwater to streams (7,826 Mgal/d) than was received by recharge over the outcrop area of the Ozark aquifer (7,346 Mgal/d) because the Ozark aquifer received an additional source of water (650 Mgal/d) from other model layers. The high degree of groundwater—surface-water interaction typical of karst hydrogeology is reflected in the calibrated hydrologic budget of the Ozark system by the large portion of recharge that enters the groundwater system and then discharges back to surface water (figs. 6C and D). However, streams were modeled as net sinks in the

groundwater-flow model, such that finer-scale groundwater—surface-water interaction was not explicitly modeled. For example, single stream reaches changing between gaining and losing, as have been observed in field-scale studies (Knierim and others, 2015), were not represented at scales smaller than several miles. Future modeling efforts may benefit from a better understanding of the surface-water connection with the groundwater system, especially because groundwater outflow to streams and springs is such a large portion of the hydrologic budget and groundwater is an important source of recharge for streams.

Comparison of Postdevelopment Conditions Between the Conceptualized and Simulated Systems

Generally, model v1.1 hydrologic-budget components for average postdevelopment (1996–2016) recharge, lateral groundwater outflow (including outflow to constant-head and general-head boundaries), and discharge to streams were similar to the predevelopment values (table 2), such that comparisons to the conceptual hydrologic budget are similar. Over the postdevelopment period in model v1.1, recharge ranged from 1,387 to 23,467 Mgal/d, reflecting seasonal variability represented by 6-month time steps as the model calibrated to groundwater-level altitudes. The largest groundwater outflow was discharge to streams, which ranged from 6,588 to 14,720 Mgal/d.

Water use for the conceptual model budget was computed by using USGS and State estimates of groundwater-withdrawal rates and was 380 Mgal/d in 2010 (Knierim and others, 2016, 2017), which corresponds to approximate (rounded) values of 100 and 300 Mgal/d for the Springfield Plateau and Ozark aquifers, respectively (fig. 6B). Groundwater-withdrawal rates from the Ozark model v1.1 for the average postdevelopment period averaged 310 Mgal/d for all layers (fig. 6D; table 2) and ranged from 65 Mgal/d during the fall and winter (October through March) to 934 Mgal/d during the spring and summer (April through September). Groundwater withdrawals in 2010, according to model v1.1, were 472 Mgal/d from the Ozark system during the active pumping season of spring and summer, with 23 Mgal/d withdrawn from the Springfield Plateaus aquifer and 384 Mgal/d withdrawn from the Ozark aquifer. The model v1.1 groundwater withdrawals were 94 Mgal/d across the Ozark system in the winter of 2010, with 4 Mgal/d withdrawn from the Springfield Plateaus aquifer and 77 Mgal/d withdrawn from the Ozark aquifer. Groundwaterwithdrawal values from 2010 are reported for direct comparison to the conceptual hydrologic budget in Hays and others (2016) and modeled water use from Knierim and others (2016, 2017). Model v1.1 results indicate lower groundwater use on average compared to groundwater use in the conceptual budget (fig. 6); for example, 283 Mgal/d was the annual average in 2010 compared to 380 Mgal/d in the conceptual hydrologic budget (Knierim and others, 2016, 2017).

The majority of groundwater withdrawals were from the lower Ozark aquifer, which in the summer of 2010 accounted for 62.4 percent of total withdrawals (fig. 7A). This ratio was relatively uniform across the study area except for Arkansas, where only 33.6 percent of groundwater withdrawals were from the lower Ozark aquifer (fig. 7B). In Arkansas, the middle Ozark aquifer supplied 16.7 percent of the demand, and the remaining 49.7 percent was distributed among the upper Ozark aquifer, the Springfield and St. Francois aguifers, and the local sands and fractures within the Western Interior Plains confining system. Oklahoma differs slightly from the other areas in that 22.8 percent of groundwater withdrawals are from the Springfield Plateaus aquifer (fig. 7D), which is likely because the Springfield Plateaus aquifer is sufficiently productive and present at land surface in much of the Oklahoma area simulated by model v1.1.

As inflows and outflows changed throughout the postdevelopment period under transient conditions, groundwater was released from storage (inflow) as water levels declined or groundwater storage was replenished (outflow) as water levels rose. Aquifer storage represents the quantity of water in an aquifer, and available storage volume may act as a source or sink for groundwater as stresses on an aquifer system change (Hays and others, 2016). In the conceptual hydrologic budget, changes in storage were assumed to balance groundwater use, which was the only flux that differed between predevelopment and postdevelopment periods (figs. 6A and B). Storage fluxes were much greater in model v1.1 than can be attributed to only changes in groundwater withdrawals: storage ranged from 7,228 Mgal/d flowing into the system (release from storage) to 8,222 Mgal/d removed from the system (replenishment to storage) suggesting that changes in recharge may have a much greater influence on storage change than groundwater withdrawals (table 2).

Hydrologic Budget—Groundwater Availability

As discussed previously, the hydrologic budget for the Ozark system is balanced between net inflows and outflows of groundwater. During predevelopment, steady-state conditions, this balance is achieved through inflows from recharge and outflows to streams, springs, and neighboring systems. After 1900, outflows additionally included groundwater withdrawals (Knierim and others, 2017) under transient conditions and associated changes in aquifer storage (figs. 6*B* and *D*).

Changes in the Hydrologic Budgets Over Time

The simulation of groundwater flow using model v1.1 provides a regional hydrologic budget from 1900 to 2016 (fig. 8). The inflows and outflows throughout the

early period of the simulation, from 1900 to about 1965 are relatively uniform, with inflow from recharge of about 13,000 Mgal/day. Much of this inflow of water is discharged through streams in the system to balance the hydrologic budget (fig. 8A). Changes in storage over time (from outflows to inflows) reflect the large variability in recharge. If recharge decreases, water levels will decrease, discharge to streams will also decrease, and groundwater may be released from aquifer storage to provide an inflow to the aquifer system. Conversely, when recharge increases, water levels will increase, discharge to streams will also increase, and storage can be replenished (numerically representing a loss of groundwater from the aquifer system). The uniformity of flows during the early period (1900–1965) is partially a product of the model development. Because less information about pumping and water-level observations is available for the early period than for 1966-2016, only two stress periods are used to define the time from 1900 to 1965; these stress periods each use average values of recharge and pumping. The net change in groundwater storage throughout this early period is essentially zero. After 1965, variability in recharge increases as the temporal resolution of the simulation increases, with stress periods representing 12to 6-month periods rather than multiple years. The period after 1965 also corresponds to an approximate doubling of withdrawals until the late 1990s (fig. 8B). After the 1990s, average withdrawals approximately double again through the end of the history matching period (2016). With these increases in withdrawals, there is a corresponding decrease, though smaller in magnitude, in discharge through the constant head boundary, which represents flow through the freshwater-saltwater transition zone along the western edge of the model boundary. This decrease in flow may essentially account for captured water that is withdrawn from wells rather than discharging further west. Conversely, a similar decrease in discharge to streams after 1965 appears to correspond to fluctuations in recharge. The decrease in flow to the constant heads could also be related to the fluctuations in recharge or to a combination of changes in recharge and withdrawals.

Most of the groundwater withdrawals from the Ozark system occur in Missouri, with more than a third of withdrawals from southwest Missouri (fig. 9). Kansas uses the most groundwater from the lower Ozark aquifer, with lesser amounts from the Western Interior Plains confining system where use has declined over the last three decades. Oklahoma's second largest source of groundwater is from the Springfield Plateau aquifer, with slight increases in the 1990s followed by relatively level average withdrawals. Midway through the simulation period (1940 to the late 1990s), some of the largest withdrawals in Arkansas are from the uppermost model layer representing the Western Interior Plains confining system.

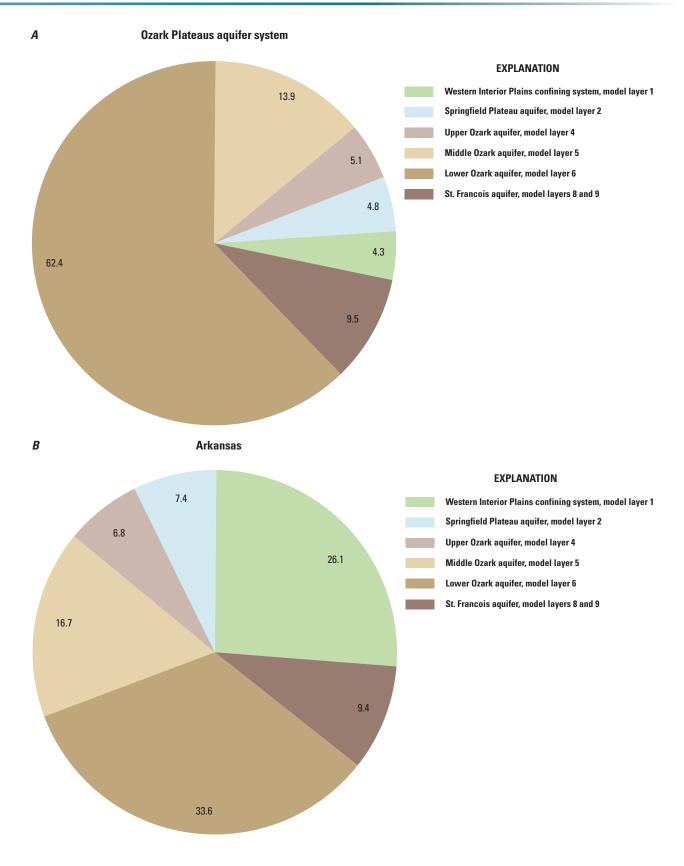


Figure 7. Distribution of groundwater withdrawals by percentage in summer 2010 by hydrogeologic unit and model layer for *A*, the Ozark Plateaus aquifer system and each geographic area in the Ozark Plateaus aquifer system: *B*, Arkansas, *C*, Kansas, *D*, Oklahoma, *E*, Missouri, and *F*, southwest Missouri.

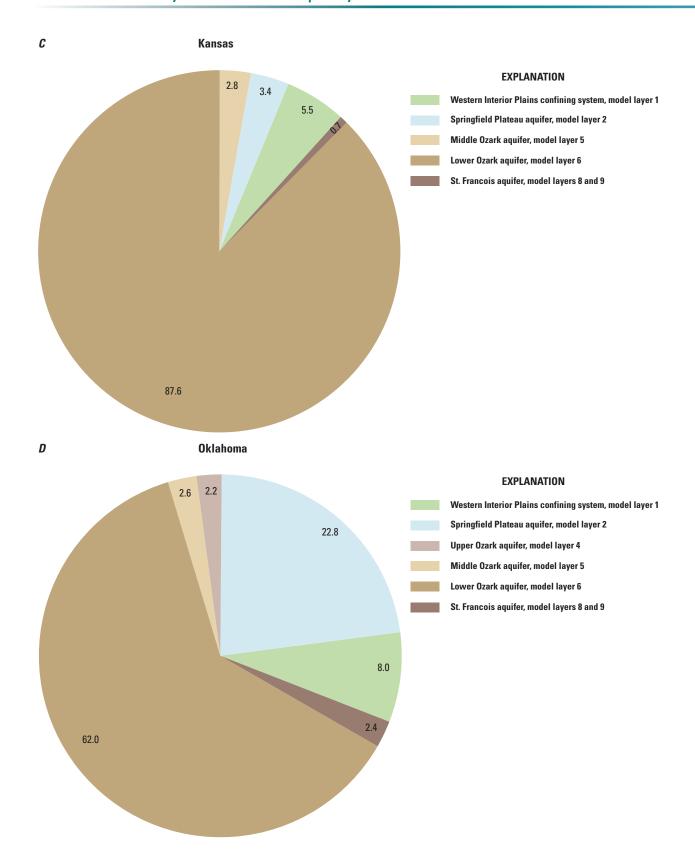


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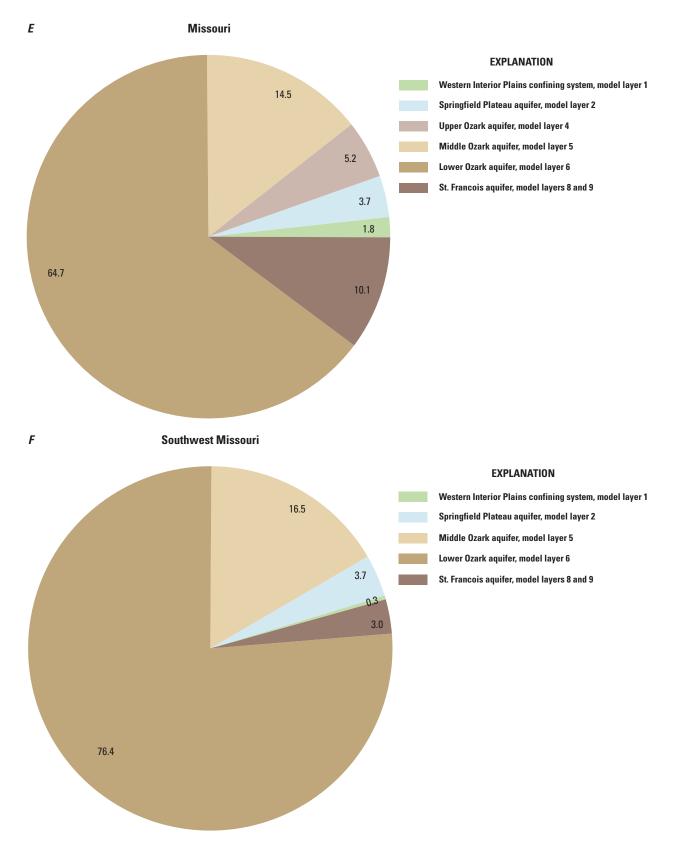


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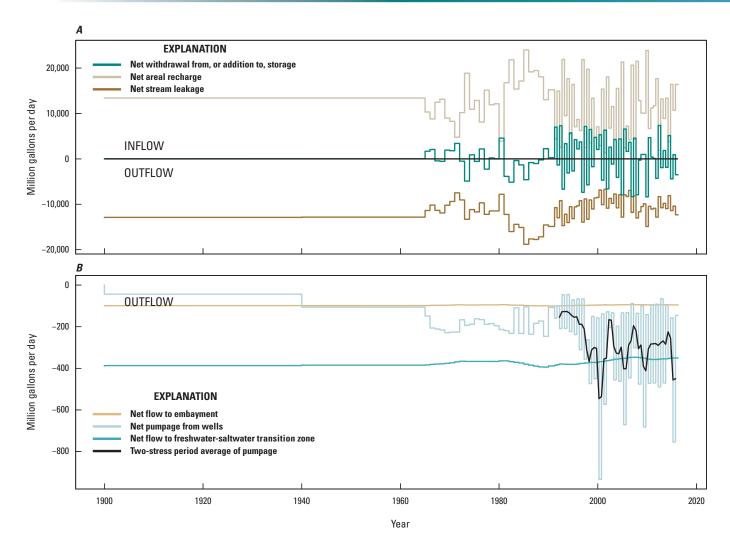


Figure 8. Groundwater-flow budget for Ozark model version 1.1 of *A*, large water-budget components and *B*, small water-budget components.



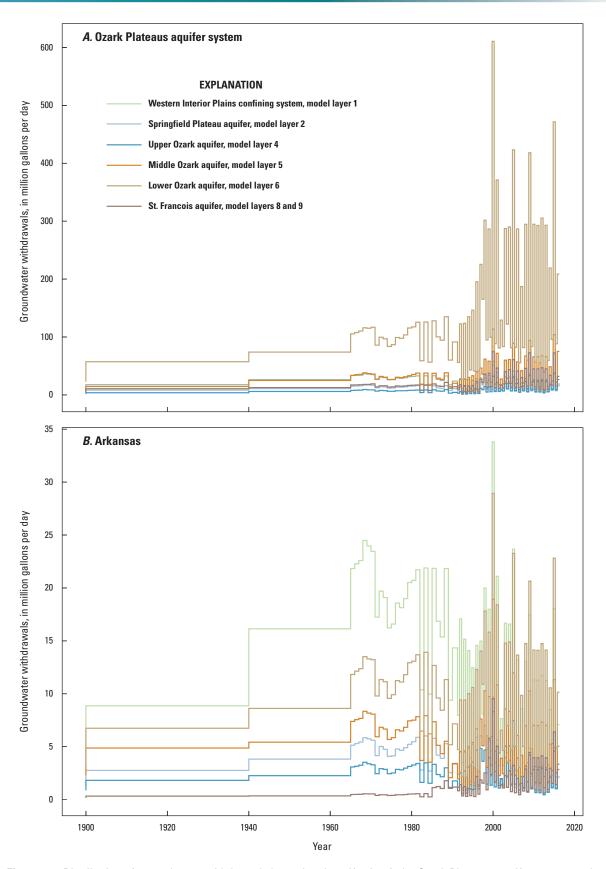


Figure 9. Distribution of groundwater withdrawals by regional aquifer for *A*, the Ozark Plateaus aquifer system and each geographic area in the Ozark Plateaus aquifer system: *B*, Arkansas, *C*, Kansas, *D*, Oklahoma, *E*, Missouri, and *F*, southwest Missouri.

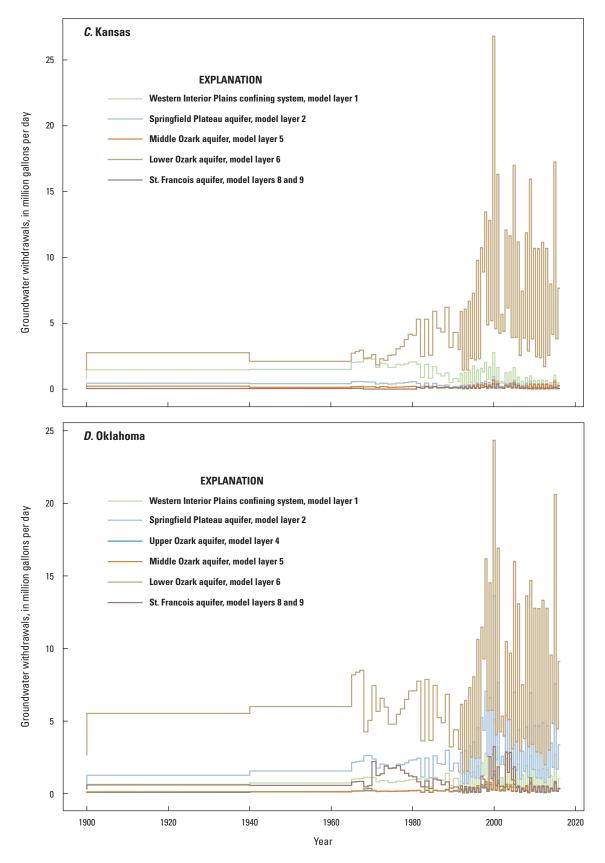


Figure 9. Distribution of groundwater withdrawals by regional aquifer for *A*, the Ozark Plateaus aquifer system and each geographic area in the Ozark Plateaus aquifer system: *B*, Arkansas, *C*, Kansas, *D*, Oklahoma, *E*, Missouri, and *F*, southwest Missouri.—Continued

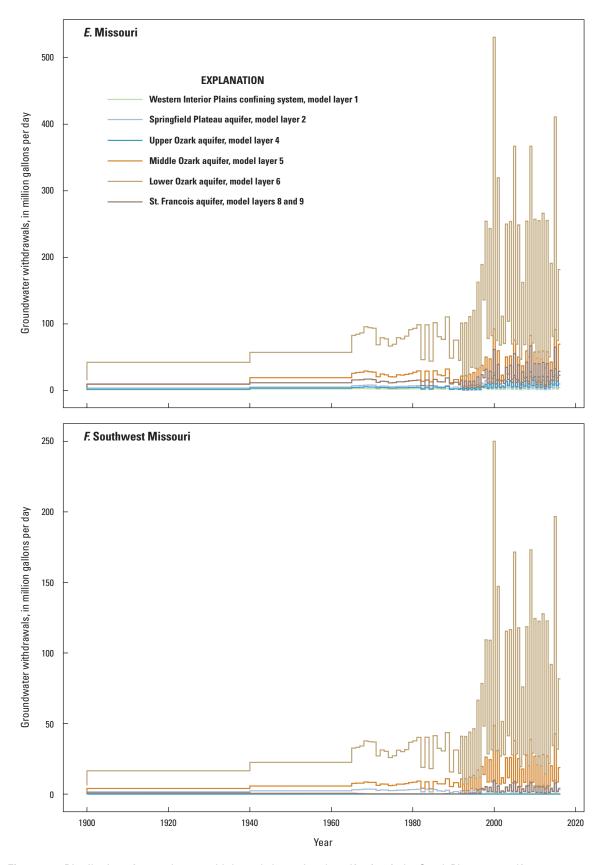


Figure 9. Distribution of groundwater withdrawals by regional aquifer for *A*, the Ozark Plateaus aquifer system and each geographic area in the Ozark Plateaus aquifer system: *B*, Arkansas, *C*, Kansas, *D*, Oklahoma, *E*, Missouri, and *F*, southwest Missouri.—Continued

Changes in Groundwater Storage and Water Levels

In general, cumulative storage change in the Ozark system appears to fluctuate from replenishment (loss of water from system) to extraction (gain of water to system) (fig. 10). The fluctuation in storage appears to be more closely linked to changes in recharge than pumping. When the cumulative departure from average recharge (where average recharge is 11.6 Bgal/d for the model period 1900 to 2016) is analyzed, recharge from predevelopment to 1965 is above average, and recharge from 1965 to about 1980 is below average (fig. 10A). Recharge is consistently above average from 1980 to about 1988, after which recharge declines through the mid-2000s. Conversely, groundwater withdrawals generally increase throughout the simulation (fig. 9), but the increase in aboveaverage pumping is steady until the late 1990s; the change in pumping does not appear to correlate with the change in storage over the same time period (fig. 10A).

Regionally, simulated changes in groundwater storage correspond more to changes in recharge than to increases in pumping (fig. 10). Prior to 1965, there is relatively little change in storage and pumping, and little to no change in water levels except for an area in northeastern Oklahoma where water levels declined by as much as 300 ft (fig. 11). From 1965 to about 1980, a negative cumulative departure from average recharge results in groundwater release from storage (shown as positive cumulative departure from average in storage over that period on fig. 10), which corresponds to several localized declines in water levels (fig. 12). Some of the largest declines are in northeastern Oklahoma as a continuation of existing declines from historical pumping.

Christenson and others (1990) note that the first wells drilled in the lower Ozark aquifer in northeastern Oklahoma flowed at land surface, but by 1981 the water level was 471 ft below land surface. From about 1980 to 1988, above-average recharge results in increased groundwater storage (shown as negative cumulative departure from average in storage on fig. 10) and groundwater levels (fig. 13). After 1988, groundwater is extracted from storage until about 2009, which corresponds to a positive slope in cumulative departure from average recharge, and after 2009, changes in storage fluctuate less dramatically (fig. 10A). The cumulative storage change until 2009 results in groundwater-level declines similar to those seen in 1980, though the declines are larger in magnitude and in spatial scope for the areas of northeastern Oklahoma and southwestern Missouri (fig. 14), where some water levels decline more than 400 ft from predevelopment.

Though the largest area of water-level declines occurs in southwestern Missouri (fig. 13), the change in water level does not necessarily correspond to a comparably large change in storage. When comparing the area of southwestern Missouri to the entire model area, cumulative storage change is less than 500 billion gallons, compared to cumulative pumping amounts of more than 1,500 billion gallons by the end of the calibration period (March 31, 2016), likely because the groundwater in this part of the Ozark system is under confined aquifer conditions. Storage values tend to be low, ranging from 3.29×10^{-7} to 4.7×10^{-5} 1/ft, under confined aquifer conditions, corresponding to findings by Hays and others (2016): "Because of the relatively minor volume of groundwater stored in the carbonate units, declines in aquifer recharge, such as occur during periods of drought, can create substantial decreases in water availability at seasonal time scales."



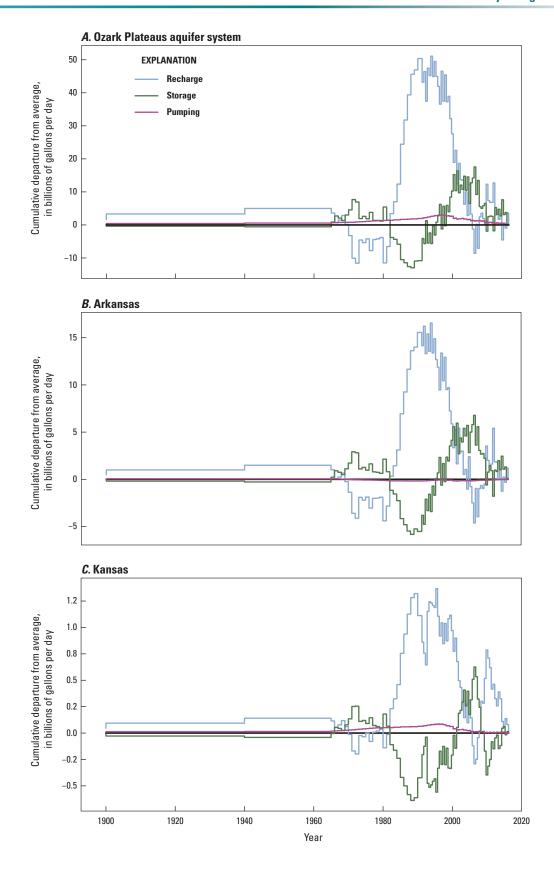


Figure 10. Cumulative recharge, pumping, and storage for *A*, the Ozark Plateaus aquifer system and each geographic area in the Ozark Plateaus aquifer system: *B*, Arkansas, *C*, Kansas, *D*, Oklahoma, *E*, Missouri, and *F*, southwest Missouri.

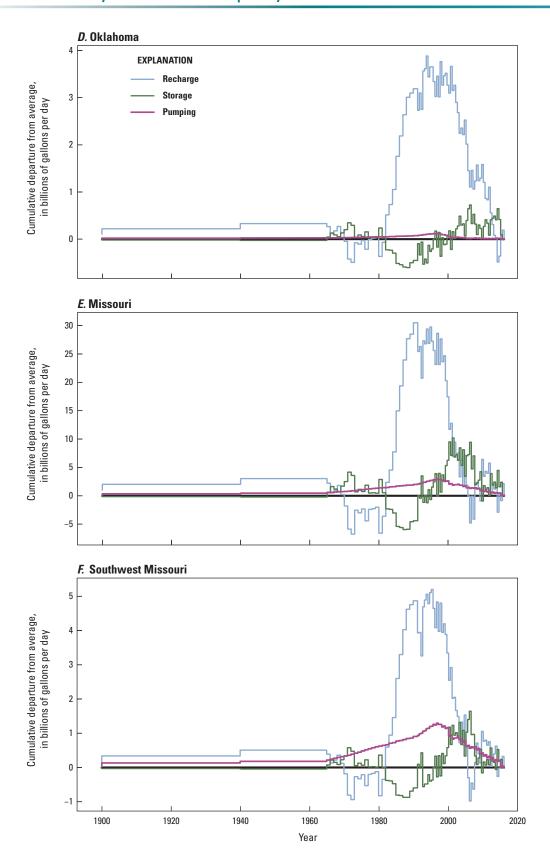


Figure 10. Cumulative recharge, pumping, and storage for *A*, the Ozark Plateaus aquifer system and each geographic area in the Ozark Plateaus aquifer system: *B*, Arkansas, *C*, Kansas, *D*, Oklahoma, *E*, Missouri, and *F*, southwest Missouri.—Continued

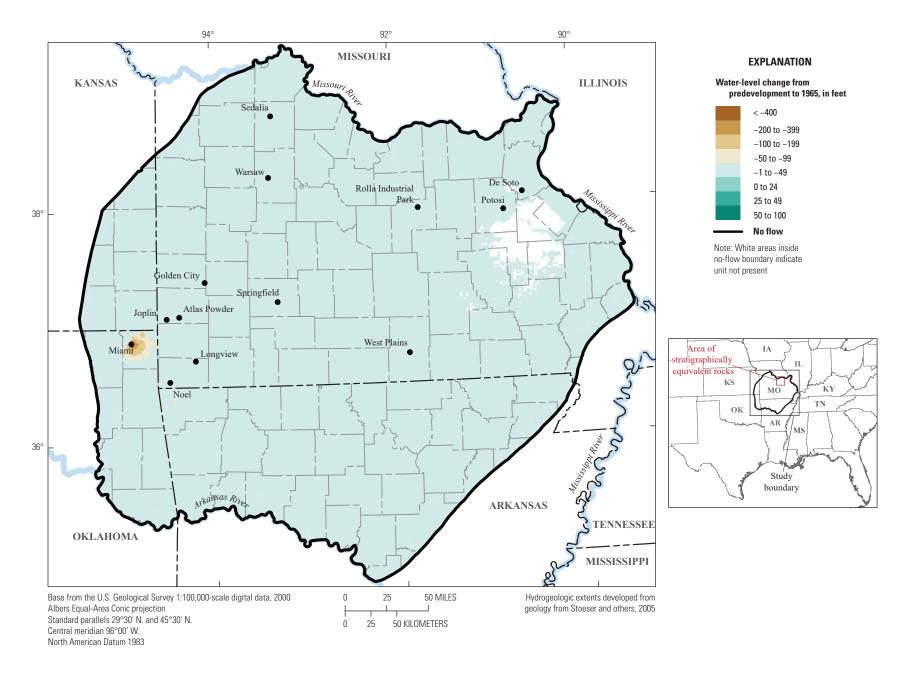


Figure 11. Water-level change from predevelopment to 1965 for the Ozark Plateaus aquifer system.

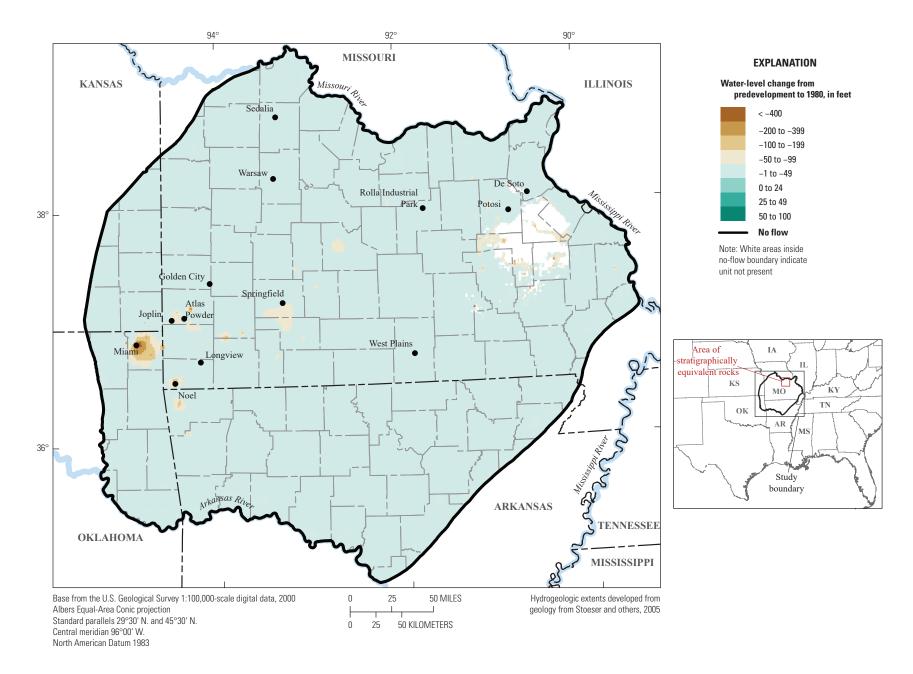


Figure 12. Water-level change from predevelopment to 1980 for the Ozark Plateaus aquifer system.

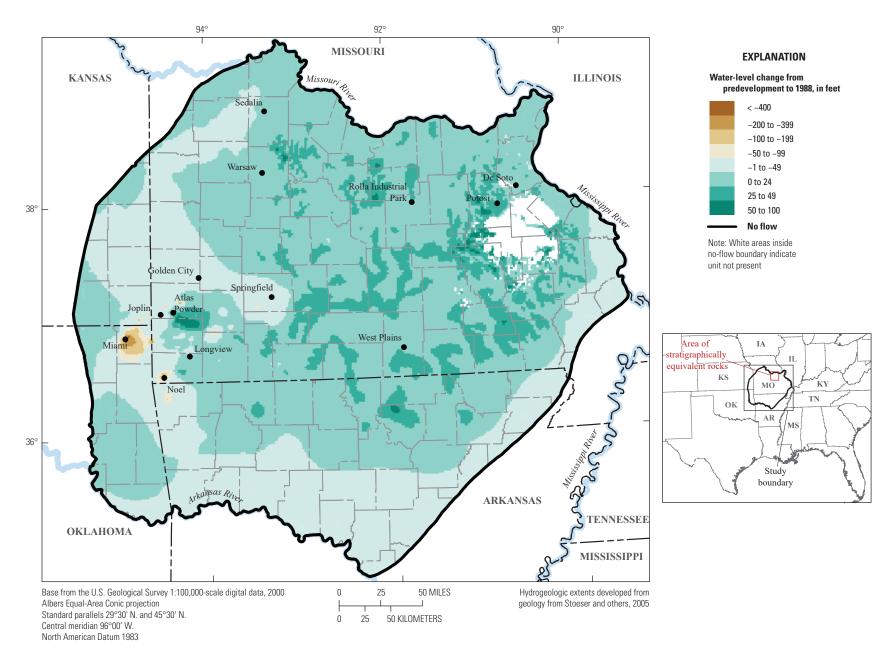


Figure 13. Water-level change from predevelopment to 1988 for the Ozark Plateaus aquifer system.

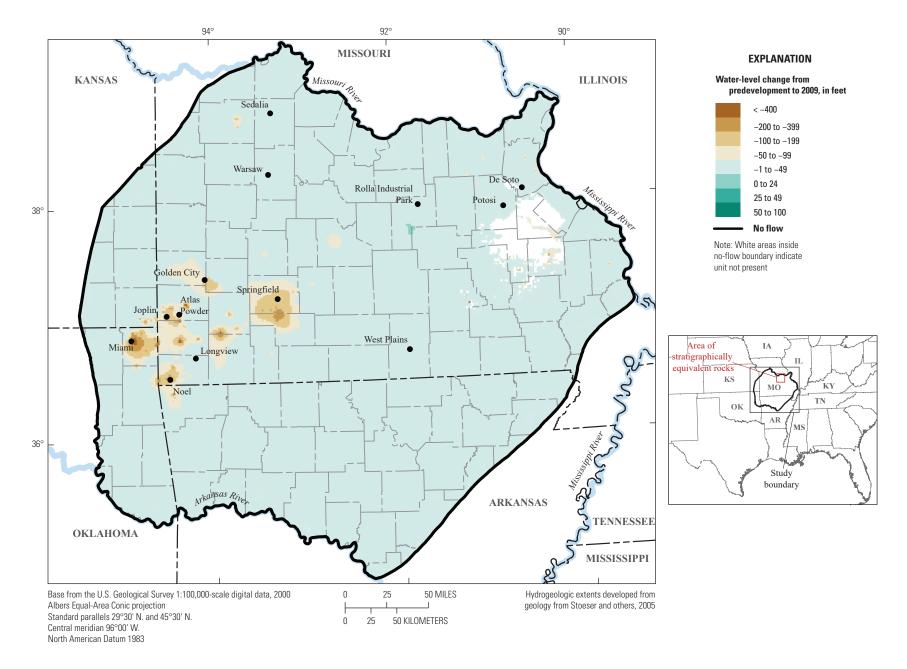


Figure 14. Water-level change from predevelopment to 2009 for the Ozark Plateaus aquifer system.

Evaluation of Potential Future Conditions

The effects of variability in groundwater withdrawals and long-term climate change on the groundwater availability of the Ozark system are a concern for communities and resource managers in the area. Groundwater withdrawal varies from year to year, but is generally expected to increase with population, industrial, and agricultural needs. Most climate models predict warmer air temperatures by midcentury in the Ozark Plateaus area, especially from midspring through early fall (Alder and Hostetler, 2013). Less agreement on the magnitude of precipitation exists among climate models, although the multi-model average indicates near-historical amounts of total annual precipitation, with changes in timing, including slight increases in winter and spring precipitation (Alder and Hostetler, 2013). Additionally, the geographic division between the more arid western United States and more humid eastern United States at the 100th meridian is predicted to shift eastward (Seager and others, 2017), potentially affecting the hydrologic budget of the western extent of the Ozark system if aridity increases. While future changes in the amount of aquifer recharge appear uncertain, increasing temperatures, especially in the summer months, imply increasing water demand, and some climate models predict increases in the frequency and intensity of drought and the intensity of extreme precipitation events. In low-storage aquifers, such as the Ozark system, even short, intense periods of drought can cause large declines in water levels. Conversely, wet periods will correspond to relatively rapid recovery.

Three scenarios were developed to simulate potential future conditions and the potential effects on the hydrologic system and availability of groundwater resources. For each scenario, the inflow and outflow budget terms were extracted from the numerical model. Water-level change was evaluated from predevelopment to October 1, 2060. The 45-year future conditions period was divided into 89 stress periods, each 6 months in length—April through September (representing spring and summer pumping and recharge conditions) and October through March (representing fall and winter pumping and recharge conditions). The seasonal conditions were simulated to provide information on the range in storage that is affected by changes in pumping and recharge. For simplicity, each scenario is summarized as follows.

- Baseline scenario—extension of the average (1996 to 2016) seasonal pumping and recharge values.
- Pumping scenario—extension of the average (1996 to 2016) seasonal recharge values and increases in pumping following the historical trend for the period 2016–2060 of up to 120 percent of the 1996 to 2016 average seasonal pumping values.

• GCM scenario—extension of the average (1996 to 2016) seasonal pumping values and variable recharge based on seasonal averages of soil water storage from a water-balance model using temperature and precipitation data from multiple general circulation models.

Baseline and Pumping Scenario Simulation Assumptions and Limitations

The baseline scenario was developed by using average summer and winter recharge and average summer and winter pumping conditions from 1996 to 2016 as discussed in the section "Comparison of Postdevelopment Conditions between the Conceptualized and Simulated Systems." All other parameter values and boundary conditions remain constant for each winter and summer stress period throughout the 45-year simulation.

The pumping scenario used the same summer and winter recharge as the baseline scenario and pumping increased based on a second-order polynomial trend of historical pumping developed by Knierim and others (2017). Pumping rates were increased at all existing wells uniformly, though under actual future conditions, the number of wells would likely increase to extract the additional water. The errors in this formulation are somewhat mitigated because the model cells are 1 mile per side, so effectively, the model represents net conditions in each square mile. A limitation of using only existing wells is that the effects of installing wells in other parts of the study area or drilling into deeper aquifers are not represented.

Climate Change Simulation Assumptions and Limitations

To simulate the effects of climate change, a potential future scenario was developed in which recharge from April 2016 to October 2060 varied according to changes in soil water storage as predicted by the water-balance model of Hostetler and Alder (2016), referred to as WBM. The WBM simulates changes in the monthly water balance driven by the National Aeronautics and Space Administration Earth Exchange Downscaled Climate Projections (NEX-DCP30) temperature and precipitation data from 30 of the 5th Climate Model Intercomparison Project (CMIP5) models. The 800-meter gridded NEX-DCP30 dataset represents statistically downscaled maximum and minimum air temperature and precipitation from the CMIP5 models (Thrasher and others, 2013). The WBM includes historical (1950-2005) and future (2006–2099) climate projections for two Representative Concentration Pathways (RCP) greenhouse gas (GHG) emission scenarios developed for the Fifth Assessment Report of the Intergovernmental Panel on Climate Change (2014).

Soil water storage simulated by using climate projections for RCP4.5 was used as a surrogate for recharge. For the RCP4.5 emissions scenario, atmospheric GHG concentrations are stabilized such that a radiative equivalent of 4.5 watts per square meter (about 650 parts per million carbon dioxide [CO₂] equivalent) is not exceeded after the year 2100. The gridded WBM dataset is available as bulk downloadable data (U.S. Geological Survey, 2018).

Estimates of recharge for model v1.1 were calculated by using a combination of methods, including annual regression-based methods and temporal modification during the history-matching process through the use of multipliers as was done in model v1.0 (Clark and others, 2018). For the same historical period of the calibrated groundwater model and the WBM, soil water storage from the WBM was several orders of magnitude larger than recharge from the calibrated groundwater model. The soil water storage time series was therefore used to scale the calibrated groundwater model recharge for future (2016–2060) 6-month (summer or winter) stress periods, such that recharge was comparable across calibration and scenario periods.

$$R_{wbm} = R_{avg} \left(\frac{SWS_{sp} - SWS_{avg}}{SWS_{avg}} + 1 \right)$$
 (2)

where

 R_{wbm} is the average (6-month) seasonal recharge, is the average (1996–2016) seasonal recharge for model v1.1, SWS_{sp} is the average (6-month) seasonal soil water storage from the WBM, and SWS_{avg} is the average (1996–2016) seasonal soil water storage from the WBM.

Recharge rates used in the GCM scenario were lower than recharge rates used in the baseline and pumping scenarios. The average summer recharge rate used in the GCM scenario was approximately half of that used in the baseline and pumping scenarios. Although lower, the average winter recharge rate used in the GCM scenario was much closer to that used in the baseline and pumping scenarios (approximately 90 percent). While summer and winter recharge rates used in the baseline and pumping scenarios remained constant over the 45-year simulation period, recharge rates used in the GCM scenario decreased. The winter recharge rate used in the GCM scenario declined faster over the 45-year period than the summer recharge rate, at an approximate rate of 11 Mgal/d instead of 3 Mgal/d, respectively.

Scenario Evaluation

The general patterns of water-level decline are similar for each scenario (figs. 15, 16, 17), but the greatest differences occur when climatic variation is simulated. For the baseline scenario, the areas of water-level decline in southwestern

Missouri and northeastern Oklahoma are only marginally different from those of 2009 (figs. 14, 15). In one area near head forecast 27, which is south of Springfield, Mo., the water-level decline from predevelopment is less in the baseline scenario than in 2009. Note that head forecasts are arbitrary numbers assigned to each head prediction for the purpose of differentiation and are discussed further in the section "Prediction Uncertainty." This water-level decline may be the extended result of a transition from groundwater use to surface-water supplies for a larger percentage of the demand in the area. Because stresses and recharge in the baseline scenario are on a constant winter/summer cycle for 45 years based on the average values from 1996 to 2016 (fig. 18A), the system approaches a new dynamic equilibrium. Reaching pseudo-equilibrium in a relatively short amount of time in groundwater systems with low storage capacity is not uncommon. This is reflected when evaluating the cumulative storage from 2016 to 2060; while seasonal fluctuation in storage continues, the overall storage change appears to gradually flatten, particularly for the Ozark aquifer (fig. 19*B*).

Areas of water-level decline in the pumping scenario are generally more expansive, and the decline is of greater magnitude with depth (fig. 16). Most areas of decline in southwestern Missouri, northeastern Oklahoma, and northwestern Arkansas are connected by an area of decline greater than 50 ft. Cumulative storage change for the pumping scenario increased from that of the baseline scenario, and the trend in storage depletion continues to increase slightly from 2016 through the end of the simulation as a result of continued increases in pumping (figs. 18*B*, 20).

More areas of water-level decline are evident in the GCM scenario compared to the baseline or pumping scenarios, though the magnitude of the depth is less than in the pumping scenario in many areas (fig. 17). Storage changes in the GCM scenario vary both seasonally and annually in response to short-term fluctuations in recharge (figs., 18C, 21). Overall storage change in the GCM scenario is greater than in the baseline and pumping scenarios, with a maximum cumulative depletion of about 4,000 billion gallons in the Ozark system (fig. 21A). The greatest storage depletion of the GCM scenario occurs within the Ozark aquifer and the Western Interior Plains confining system, likely because the changes in recharge directly affect the units with the largest area of exposed rock throughout the model area. Though the magnitude of water-level declines appears less in the GCM scenario than in the other scenarios for the Ozark aquifer, the amount of water removed from storage is greater because of the unconfined conditions of the units and the broad areal extent of the declines.

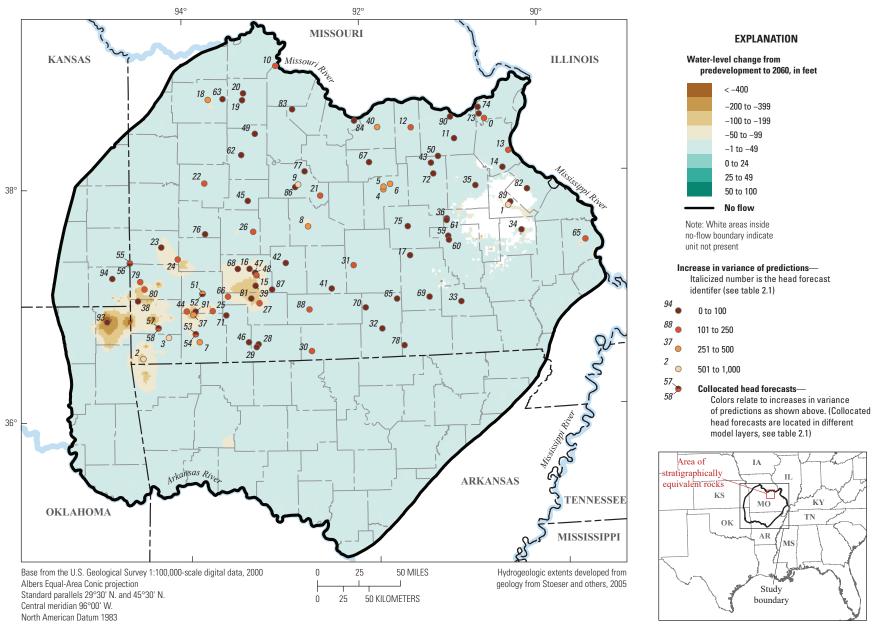


Figure 15. Water-level change from predevelopment to 2060 for the Ozark Plateaus aquifer for the baseline scenario. For each forecast location (as identified by head forecast identifier), summative percent increase in variance for predicted water levels at all forecast locations, when observation(s) at this location are excluded from the history matching dataset. Head forecast identifiers are arbitrary numbers assigned to each head prediction for the purpose of differentiation. See table 2.1 for head forecast identifier and associated model layer.

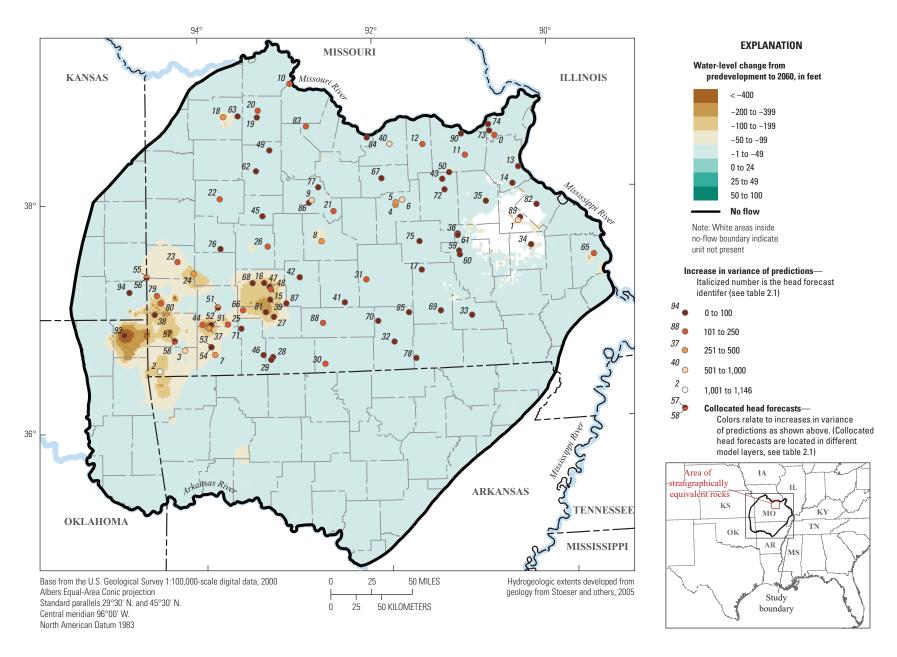


Figure 16. Water-level change from predevelopment to 2060 for the Ozark Plateaus aquifer for the pumping scenario. For each forecast location (as identified by head forecast identifier), summative percent increase in variance for predicted water levels at all forecast locations, when observation(s) at this location are excluded from the history matching dataset. Head forecast identifiers are arbitrary numbers assigned to each head prediction for the purpose of differentiation. See table 2.1 for head forecast identifier and associated model layer.

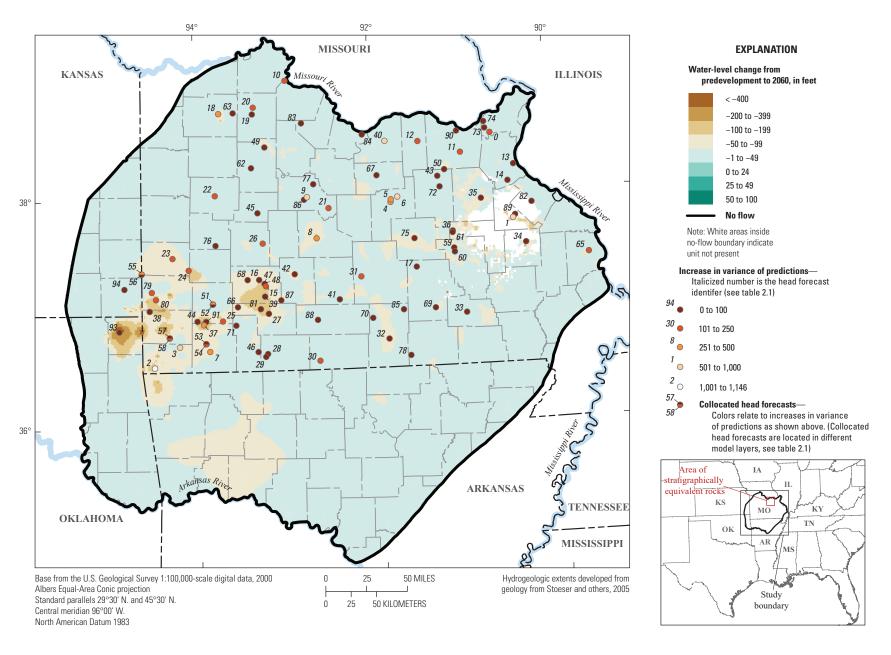


Figure 17. Water-level change from predevelopment to 2060 for the Ozark Plateaus aquifer for the general circulation model scenario. For each forecast location (as identified by head forecast identifier), summative percent increase in variance for predicted water levels at all forecast locations, when observation(s) at this location are excluded from the history matching dataset. Head forecast identifiers are arbitrary numbers assigned to each head prediction for the purpose of differentiation. See table 2.1 for head forecast identifier and associated model layer.

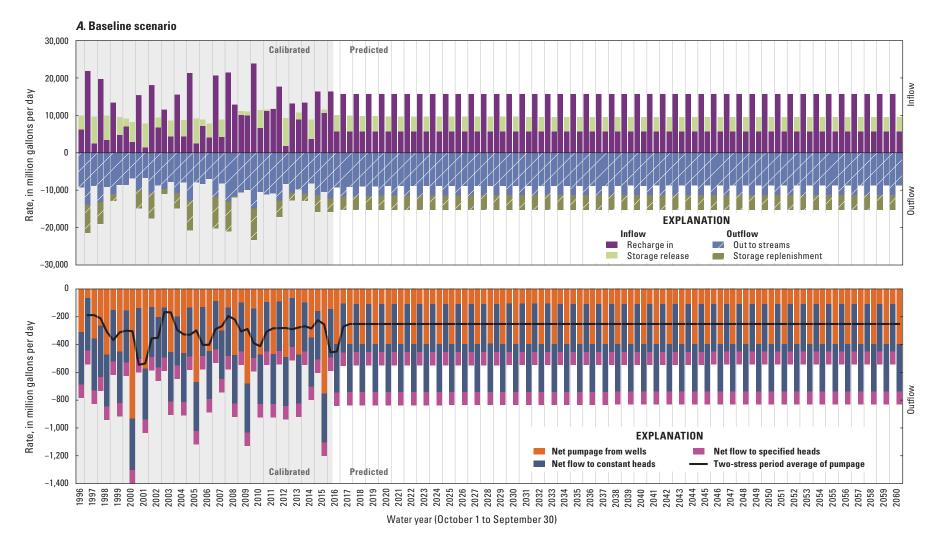


Figure 18. Hydrologic budget for each scenario developed to simulate potential future conditions from 2015 to 2060 in the Ozark Plateaus aquifer system: *A*, the baseline scenario, *B*, the pumping scenario, and *C*, the general circulation model scenario.

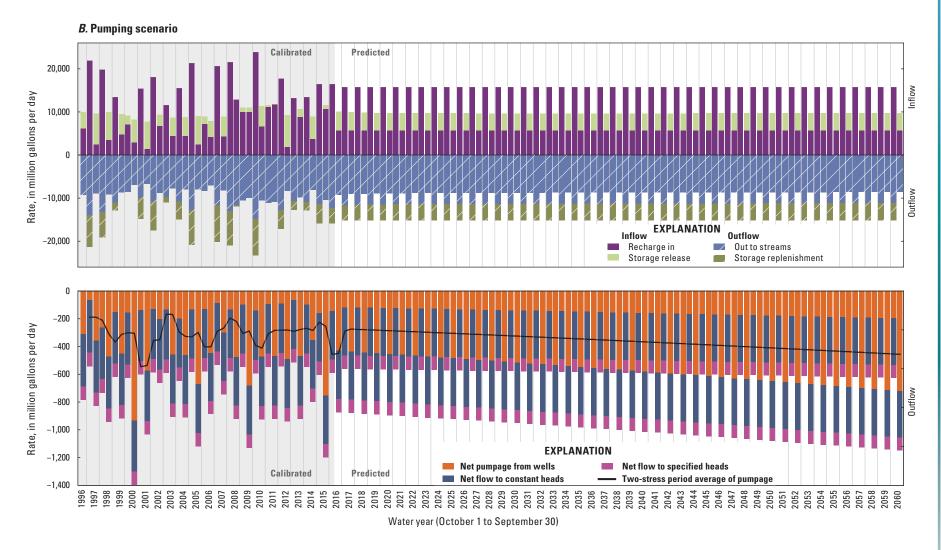


Figure 18. Hydrologic budget for each scenario developed to simulate potential future conditions from 2015 to 2060 in the Ozark Plateaus aquifer system: *A*, the baseline scenario, *B*, the pumping scenario, and *C*, the general circulation model scenario.—Continued

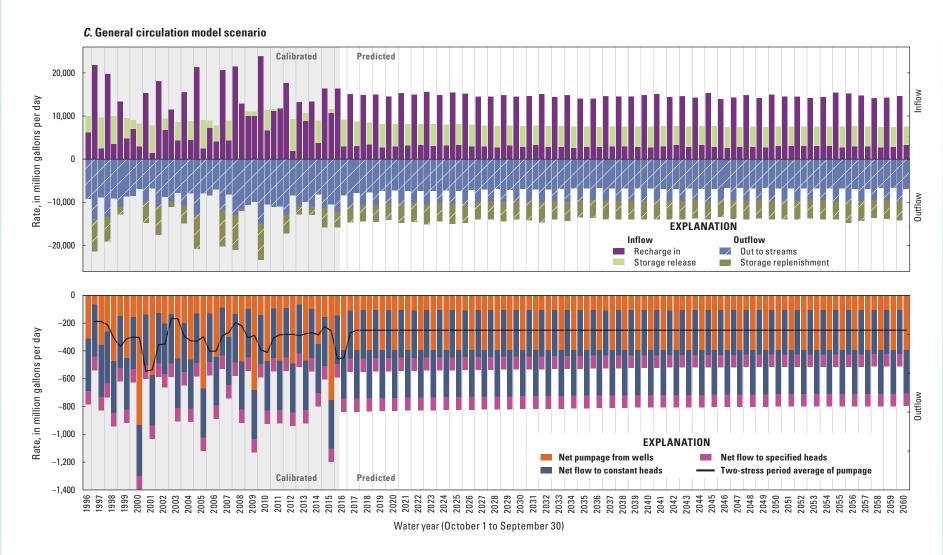
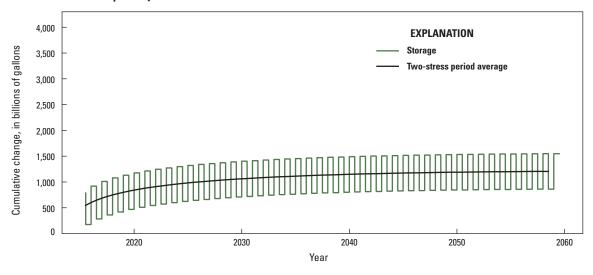


Figure 18. Hydrologic budget for each scenario developed to simulate potential future conditions from 2015 to 2060 in the Ozark Plateaus aquifer system: *A*, the baseline scenario, *B*, the pumping scenario, and *C*, the general circulation model scenario.—Continued

A. Ozark Plateaus aquifer system



B. Ozark aquifer

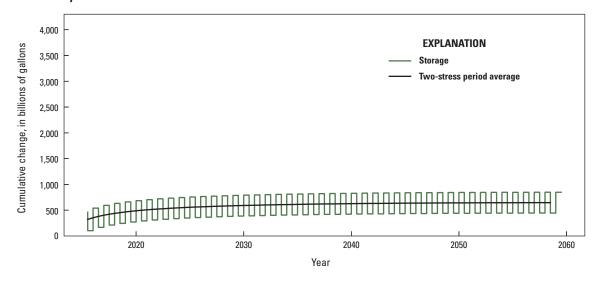
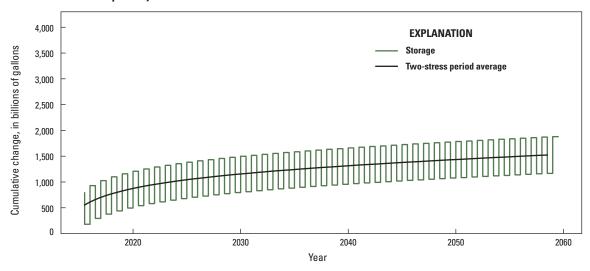


Figure 19. Storage change from 2015 to 2060 for the baseline scenario for the *A*, Ozark Plateaus aquifer system and *B*, Ozark aquifer.

A. Ozark Plateaus aquifer system



B. Ozark aquifer

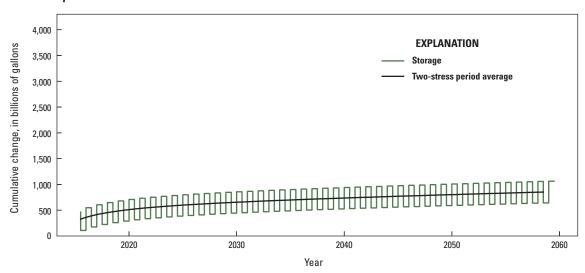
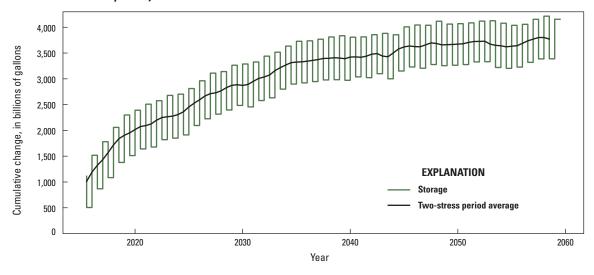


Figure 20. Storage change from 2015 to 2060 for the pumping scenario for the *A*, Ozark Plateaus aquifer system and *B*, Ozark aquifer.

A. Ozark Plateaus aquifer system



B. Ozark aquifer

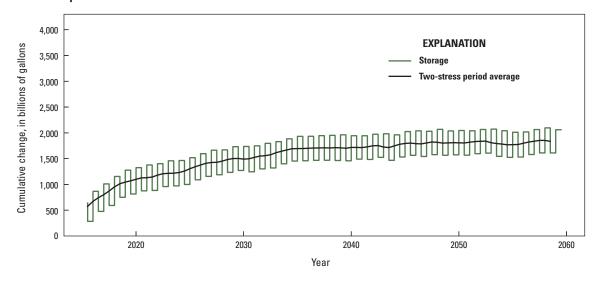


Figure 21. Storage change from 2015 to 2060 for the general circulation model scenario for the *A*, Ozark Plateaus aquifer system and *B*, Ozark aquifer.

Simulation Uncertainty

A numerical groundwater model may be adequately history matched according to traditional calibration measures, such as when the best-fit parameters yield an acceptable fit with the observation dataset and are in general agreement with aquifer-specific expert knowledge. However, many parameter values have a wide range in uncertainty even after history matching. If the forecasts (or predictions) of interest are sensitive to these uncertain parameter values, then substantial uncertainty about these forecasts may also remain following history matching. Therefore, linear uncertainty analysis was used to quantify parameter and predictive uncertainty when using the scenarios simulating future potential conditions. (See appendix 2 for an explanation and the computational details of linear uncertainty analysis.)

Parameter Uncertainty

In general, the reduction in uncertainty tends to increase for parameters in areas with numerous observations and stressed areas of the model. Reductions in the uncertainty of parameters related to aquifer properties were highly variable, ranging from no reduction in uncertainty to almost 98 percent (table 3). The history matching effort was helpful in informing many of the parameters that influence stream discharge to and from groundwater, specifically the streambed conductance multipliers for 8-digit hydrologic unit codes (table 3). The horizontal flow boundary multiplier and the constant head boundary parameters in the Western Interior Plains confining system and the lower Ozark aquifer were well informed by the observation dataset, whereas the constant head boundary parameter in the St. François aquifer was not (table 3). Parameters related to boundary conditions and aquifer properties are valuable in evaluating the conceptualization of the flow system. Collecting more information about parameters and collecting more observation information could potentially reduce the uncertainty of parameters, advance model development, and aid calibration exercises.

Spatial and temporal recharge multiplier parameter uncertainties are important for understanding how reliably recharge is estimated and implemented in the model. The same is true for the temporal well multiplier parameter uncertainty and pumping in the model. The linear uncertainty analysis indicated that history matching did not decrease the uncertainties of the different recharge and well parameters (table 3). Recharge from rainfall and local-scale water withdrawals from wells are the driving forces behind groundwater flow and heads in the Ozark Plateaus aquifer system. Increased knowledge of recharge processes and water withdrawals from the aquifer system could potentially decrease the uncertainty of these parameters, thereby benefiting model developments.

Prediction Uncertainty

Model parameters retain uncertainty even after they have been subjected to the history-matching process, and can therefore continue to affect predictions of interest. To more clearly distinguish the reliability of model predictions, it is necessary to evaluate uncertainty in predictions made by groundwater models based on best-fit parameters with explicit consideration of parameter uncertainty. (See appendix 2 for an explanation and the computational details of predictive linear uncertainty analysis.)

Head forecasts, representing the heads located at 94 real-time observation wells, were used as quantitative targets for the three predictive models (as described by the aforementioned three scenarios) and associated uncertainty analyses. Predictive uncertainty results from the baseline and GCM scenarios indicate that the prior uncertainty of model input parameters produced margins of error in excess of 100 ft for about half of the head predictions (figs. 22 and 23); under the pumping scenario, the prior uncertainty of model input parameters produced margins of error in excess of 100 ft for more than half of the head predictions (fig. 24). The history-matching process provided important information to inform prediction-sensitive model parameters. After the history-matching effort, more than half of the head forecasts in the pumping scenario had margins of error less than 25 ft (fig. 24). For the baseline scenario, the history-matching process resulted in 73 percent of the head forecasts having margins of error less than 25 ft (fig. 22). Results of the history-matching effort for the GCM scenario were similar to those of the baseline scenario (fig. 23). The reduction in uncertainty with respect to the head forecasts located at the 94 real-time observation wells implies that the observation dataset is informing those adjustable parameters that control these predictions and that these predictions can be made with some reliability.

Out of the 94 locations serving as positions for water-level predictions, 93 were also part of the observation dataset used in the history-matching effort. Reduction in predictive uncertainty is likely to be larger for predictions that are similar, either in location or type, to observations in the dataset employed in the history-matching process, especially if observations at prediction locations contain significant information pertaining to prediction-sensitive adjustable parameters. It is also important to note that the head forecasts at the 94 forecast locations were absolute predictions. Absolute values are more difficult to accurately predict than a difference, such as the difference in head from the start of the model simulation to the end of the model simulation.

Table 3. Percent reduction in uncertainty associated with groups of adjustable model parameters.

[* only one parameter in the group]

Parameter group	Parameter group description	Minimum	Maximum	Mean	Median
chd	Constant head boundary multipliers for the Western Interior Plains confining system, lower Ozark aquifer, and St. Francois aquifer	9.5	97.8	58.5	68.2
ghb*	General head boundary multiplier	1.7	1.7	1.7	1.7
hfb*	Horizontal flow boundary multiplier	90.9	90.9	90.9	90.9
hk2	Horizontal hydraulic conductivity of the Springfield Plateau aquifer	0	94.5	9.6	3.2
hk4	Horizontal hydraulic conductivity of the upper Ozark aquifer	0	85.4	6.9	1.4
hk6	Horizontal hydraulic conductivity of the lower Ozark aquifer	0	96.9	17	6.4
hk9	Horizontal hydraulic conductivity of the St. Francois aquifer	0	94.4	5.5	1.3
rch0	Recharge pilot points multipliers	0	92.5	16.1	5.4
sctsrch	Temporal recharge multipliers for scenario stress periods	0	0	0	0
scwel	Pumping multipliers for scenario stress periods	0	0	0	0
sfr	Streambed conductance multipliers for 8-digit hydrologic unit codes	0	98.7	68.5	84.1
ss1*	Specific storage of the Western Interior Plains confining system	85.1	85.1	85.1	85.1
ss2*	Specific storage of the Springfield Plateau aquifer	52.8	52.8	52.8	52.8
ss3*	Specific storage of the Ozark confining unit	10.2	10.2	10.2	10.2
ss4*	Specific storage of the upper Ozark aquifer	11.1	11.1	11.1	11.1
ss5*	Specific storage of the middle Ozark aquifer	55.3	55.3	55.3	55.3
ss6	Specific storage of the lower Ozark aquifer	0	97.8	6.1	1
ss7*	Specific storage of the St. François confining unit	74.2	74.2	74.2	74.2
ss9*	Specific storage of the St. François aquifer	87.8	87.8	87.8	87.8
sy1*	Specific yield of the Western Interior Plains confining system	46.8	46.8	46.8	46.8
sy2*	Specific yield of the Springfield Plateau aquifer	81	81	81	81
sy3*	Specific yield of the Ozark confining unit	68.3	68.3	68.3	68.3
sy4*	Specific yield of the upper Ozark aquifer	48.5	48.5	48.5	48.5
sy5*	Specific yield of the middle Ozark aquifer	64.5	64.5	64.5	64.5
sy6	Specific yield of the lower Ozark aquifer	0	62.4	1.1	0
tsrch	Temporal recharge multipliers for calibration stress periods	0	98.4	12.6	0
vk1	Vertical hydraulic conductivity of the Western Interior Plains confining system	0	78	20.2	14
vk3	Vertical hydraulic conductivity of the Ozark confining unit	0	90.9	6.9	1.5
vk5	Vertical hydraulic conductivity of the middle Ozark aquifer	0	97.3	16.9	6.7
vk7	Vertical hydraulic conductivity of the St. Francois confining unit	0	97.6	4	0.2
wel	Pumping multipliers for calibration stress periods	0	88	12.3	0

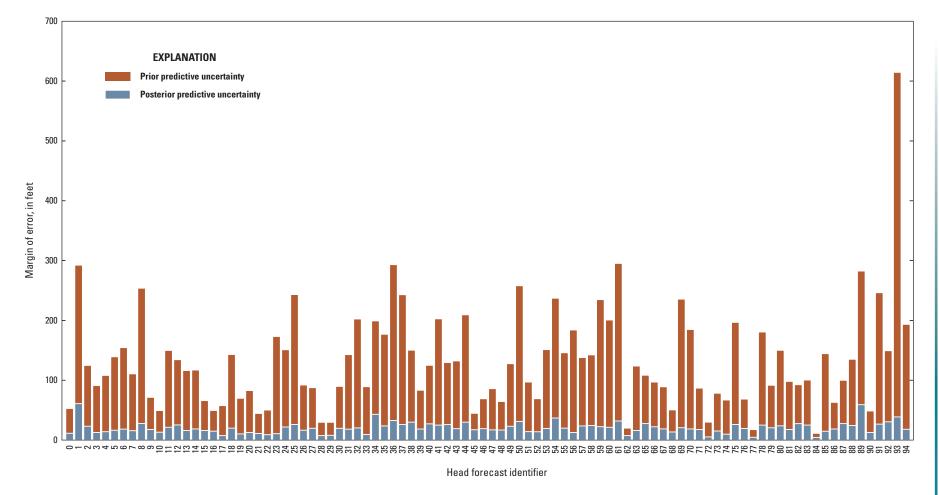


Figure 22. The margin of error, or radius, of the 95-percent credible interval for the prior and posterior predictive uncertainty of head during the 2016–60 model period for the baseline scenario at real-time observation wells of interest in the Ozark Plateaus aquifer system. Head forecast identifiers are arbitrary numbers assigned to each head prediction for the purpose of differentiation. See table 2.1 for head forecast identifier and associated model layer.

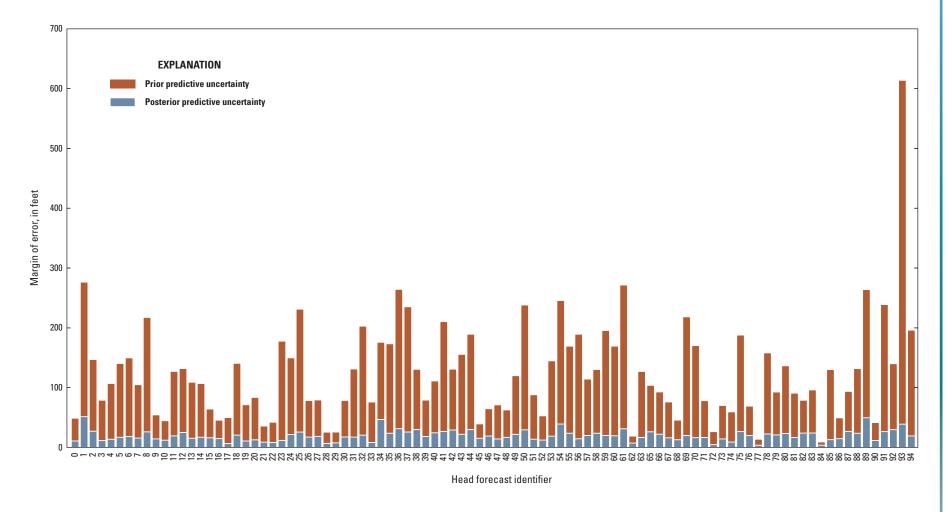


Figure 23. The margin of error, or radius, of the 95-percent credible interval for the prior and posterior predictive uncertainty of head during the 2016–60 model period for the general circulation model scenario at real-time observation wells of interest in the Ozark Plateaus aquifer system. Head forecast identifiers are arbitrary numbers assigned to each head prediction for the purpose of differentiation. See table 2.1 for head forecast identifier and associated model layer.

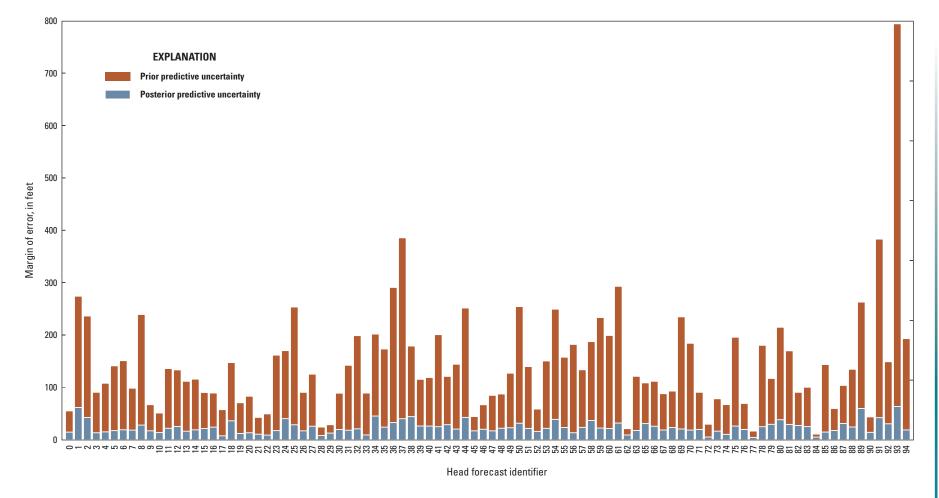


Figure 24. The margin of error, or radius, of the 95-percent credible interval for the prior and posterior predictive uncertainty of head during the 2016–60 model period for the pumping scenario at real-time observation wells of interest in the Ozark Plateaus aquifer system. Head forecast identifiers are arbitrary numbers assigned to each head prediction for the purpose of differentiation. See table 2.1 for head forecast identifier and associated model layer.

Data-Worth Analysis—Use of Numerical Models to Inform Groundwater Networks

Data worth can be divided into two main categories: (1) the worth of data pertaining directly to parameters, and (2) the worth of data pertaining indirectly to observations. Reducing the uncertainty of parameters in a model typically reduces the uncertainty of predictions of interest. Parameter uncertainty can be reduced either directly by collecting more or better information about parameters, or indirectly by collecting more or better information about observations that inform adjustable parameters estimated through the history-matching process. Thus, linear-based uncertainty analyses (White and others, 2016) were used to quantitatively evaluate which parameters or groups of parameters are contributing most to the prior and posterior uncertainty associated with the 94 head predictions. (See appendix 2 for further explanation of the linear uncertainty analysis used to evaluate the worth of data pertaining to parameters.)

Multiple groundwater-level monitoring networks exist in the Ozark system. In northern Arkansas, 50 to 60 wells in the Ozark system are measured on an approximate 3-year rotation (Czarnecki and others, 2014; Schrader, 2015). In Missouri, about 90 wells in the Ozark system are included in a realtime network (Missouri Department of Natural Resources, 2018). These wells and more were used to create a regional potentiometric map of the Ozark system in the winter of 2014–15 (Nottmeier, 2015). Many of the water levels used to construct the potentiometric surface representing the winter of 2014 conditions also were used as observations for calibration purposes in the model. The Ozark model was used to evaluate the worth of existing observations; for example, a subset of the real-time well locations was used as predictions within the scenarios presented in the report section "Evaluation of Potential Future Conditions."

The Worth of Data Pertaining Directly to Parameters

Linear-based uncertainty analysis was used to evaluate the parameter groups contributing most to the uncertainty for each of the 94 head forecasts. The parameter groups comprising riverbed conductance multipliers, vertical hydraulic conductivities for the middle Ozark aquifer and the Ozark confining unit, and horizontal hydraulic conductivities for the Springfield Plateau, lower Ozark, and St. Francois aquifers were common to all three scenarios and had the most influence on the head measurements at the evaluated locations. Because 87 percent of the head forecasts are located in the lower Ozark aquifer and 7 percent are located in the Springfield Plateau aquifer, this influence on the head measurements is expected. For all three scenarios, the horizontal hydraulic conductivity for the lower Ozark aquifer was the most important parameter

group for 20 percent of the head predictions of interest, contributing, on average, 40 percent to the predictive uncertainty at these head forecasts. Similarly, the vertical hydraulic conductivity parameter for the middle Ozark aquifer contributed most to predictive uncertainty at 13 percent of head forecasts, accounting for about 40 percent of the predictive uncertainty at these head forecasts. The riverbed conductance parameter played a larger role in predictive uncertainty, contributing approximately 60 percent, but at a smaller number of the head forecasts of interest (5 percent).

For all three scenarios, future recharge played an important role in determining the uncertainty at more than half of the 94 head forecasts. The temporal recharge multiplier contributed about 55 percent to the predictive uncertainty in head forecasts. If future groundwater-flow model forcings, such as recharge, are sufficiently uncertain, it may be more beneficial for model development to focus on understanding the effects of these future forcings rather than on traditional history-matching exercises designed to adjust temporally static model parameters, such as aquifer properties (Anderson and others, 2015; White and others, 2016). Independent measurement of sensitive parameters should reduce uncertainty in parameter values and therefore should improve predictions of groundwater levels in the future. The parameters with the highest value for future work are parameters that control recharge rates, the connectivity of rivers and streams to groundwater, lateral flow through the lower Ozark aquifer, and vertical flow through the middle Ozark aquifer.

The Worth of Data Pertaining Indirectly to Observations

Linear-based uncertainty analysis was also used to identify the most valuable head observations for predicting water levels at all forecast locations. For all three scenarios, the water level observations associated with forecast locations 2 and 3 were the two most important observation groups for reducing posterior uncertainty at all predictions of interest (figs. 15–17). These observations are informing the parameters to which the head forecasts are sensitive during the history-matching process and are important for reducing the margins of error for the 94 head forecasts. Conversely, the observations associated with forecast locations 93, 84, and 74 play a comparatively miniscule role in reducing predictive uncertainty for all three scenarios (figs. 15–17). Based on this example of data worth, these observations could be considered redundant in informing the existing groundwater monitoring network and may be targets for removal.

It should be noted that an analysis of data worth can also be applied to any potential, model relevant observations, such as proposed monitoring wells or stream leakage measurements. The above data worth analysis provides an example involving existing monitoring locations. A more rigorous evaluation of data worth across the Ozark model might include a uniform grid of potential observation locations or select stream reaches.

Challenges for Future Groundwater Availability Assessments—Lessons Learned

This groundwater assessment of the Ozark system provides the first transient model of regional groundwater flow encompassing the freshwater resources of the area. The information and knowledge of the system incorporated into this report was supported by previous investigations at regional and local scales. The regional assessments provided by Imes and Emmett (1994) provided the foundation upon which model v1.1 was developed. Nationwide, 12 regional groundwater availability assessments have been completed as of 2018 and 11 are in progress; these assessments include many of the Nation's primary aquifers. These regional assessments represent a large investment in time and resources to compile, interpret, and publish results. As with the previous work from the Regional Aquifer System Assessment (RASA), one of the great challenges is archiving this wealth of information in a way that is accessible in the future, and that can be easily used as the foundation for studies at higher spatial and temporal resolutions. Other regional studies conducted by the USGS have noted additional challenges related to error or uncertainty in the various inputs on which the simulations are built—recharge, pumping, evapotranspiration, stream interaction, observation information, aquifer properties, and model discretization are often mentioned. This study is no different, as many of these inputs contain some amount of spatial and temporal uncertainty. Therefore, the challenge of providing accessible data is extended in that the data should be accompanied by some measure of uncertainty. As studies evolve, some in the same areas as past simulations, the data should be improved, and the uncertainty reduced with versions of the information stored and accessed for future work. In this way, each pathway of information can be better adjusted for more accurate historical and spatial representation in groundwater-flow models for various scales. The building and use of accessible, evolving information may be one of the greater challenges facing future groundwater availability assessments.

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Appendix 1

This appendix contains additional information regarding updated parameter estimates (table 1.1) and model fit for the Ozark groundwater availability model, version 1.1. The model results were evaluated through the calculation of the mean residual for the hydraulic heads. The mean of residuals indicates model bias depending on the magnitude and direction of the mean away from zero (fig. 1.1). An additional assessment of model error and of model fit to the observation data is accomplished through a plot of the

observed water-level altitudes and simulated hydraulic head values (fig. 1.2). Simulated stream leakage and the 7-day, 2-year (7Q2) statistic values were used to assess the model estimates of discharge from the aquifer system to streams (fig. 1.3). As a final assessment of model fit, temporal trends of the model were examined through simulated and observed hydrographs in 10 selected wells completed in the Springfield Plateau aquifer, the Ozark aquifer, and the St. Francois aquifer (fig. 1.4).

Table 1.1. Final parameter estimates for the Ozark groundwater-flow model, version 1.1.—Continued

[All aquifer properties exclude Mississippi embayment values. ft/d, foot per day; ft, foot; ft²/d, foot squared per day]

Parameter description	Parameter description Parameter Value or range		Units	Model layer	
Hydraulic conductivity (horizontal)	hk1	6.14e-03 to 3.21e-01	ft/d	1	
Hydraulic conductivity (horizontal)	hk2	1.00 to 191.04	ft/d	2	
Hydraulic conductivity (horizontal)	hk3	2.00e-07 to 1.27e-01	ft/d	3	
Hydraulic conductivity (horizontal)	hk4	1.44e-03 to 1.17e+01	ft/d	4	
Hydraulic conductivity (horizontal)	hk5	1.44e-03 to 5.24e-01	ft/d	5	
Hydraulic conductivity (horizontal)	hk6	0.06 to 93.70	ft/d	6	
Hydraulic conductivity (horizontal)	hk7	1.00e-08 to 4.00e-01	ft/d	7	
Hydraulic conductivity (horizontal)	hk8	3.64 to 41.28	ft/d	8	
Hydraulic conductivity (horizontal)	hk9	3.64 to 41.28	ft/d	9	
Hydraulic conductivity (vertical)	vk1	6.14e-04 to 3.21e-02	ft/d	1	
Hydraulic conductivity (vertical)	vk2	0.10 to 19.10	ft/d	2	
Hydraulic conductivity (vertical)	vk3	2.00e-08 to 1.27e-02	ft/d	3	
Hydraulic conductivity (vertical)	vk4	1.44e-04 to 1.17e+00	ft/d	4	
Hydraulic conductivity (vertical)	vk5	1.44e-04 to 5.24e-02	ft/d	5	
Hydraulic conductivity (vertical)	vk6	6.37e-03 to 9.37e+00	ft/d	6	
Hydraulic conductivity (vertical)	vk7	1.00e-09 to 4.00e-02	ft/d	7	
Hydraulic conductivity (vertical)	vk8	0.36 to 4.13	ft/d	8	
Hydraulic conductivity (vertical)	vk9	0.36 to 4.13	ft/d	9	
Specific yield	sy1	4.55e-03	dimensionless	1	
Specific yield	sy2	3.83e-03	dimensionless	2	
Specific yield	sy3	0.01	dimensionless	3	
Specific yield	sy4	5.28e-04 to 4.71e-03	dimensionless	4	
Specific yield	sy5	5.28e-04	dimensionless	5	
Specific yield	sy6	2.69e-03 to 5.00e-02	dimensionless	6	
Specific storage	ss1	8.96e-06	1/ft	1	
Specific storage	ss2	3.49e-06	1/ft	2	
Specific storage	ss3	2.94e-06	1/ft	3	
Specific storage	ss4	3.90e-07 to 5.27e-07	1/ft	4	
Specific storage	ss5	3.90e-07	1/ft	5	

 Table 1.1.
 Final parameter estimates for the Ozark groundwater-flow model, version 1.1.—Continued

 $[All\ aquifer\ properties\ exclude\ Mississippi\ embayment\ values.\ ft/d,\ foot\ per\ day;\ ft,\ foot;\ ft^2/d,\ foot\ squared\ per\ day]$

Temporal recharge multipliers tsrch1 1.25 dimensionless multiple Temporal recharge multipliers tsrch2 1.25 dimensionless multiple Temporal recharge multipliers tsrch3 1.25 dimensionless multiple Temporal recharge multipliers tsrch4 0.96 dimensionless multiple Temporal recharge multipliers tsrch5 0.82 dimensionless multiple Temporal recharge multipliers tsrch6 1.16 dimensionless multiple Temporal recharge multipliers tsrch6 1.16 dimensionless multiple Temporal recharge multipliers tsrch8 0.84 dimensionless multiple Temporal recharge multipliers tsrch9 0.77 dimensionless multiple Temporal recharge multipliers tsrch9 0.77 dimensionless multiple Temporal recharge multipliers tsrch10 0.44 dimensionless multiple Temporal recharge multipliers tsrch11 0.95 dimensionless multiple Temporal recharge multipliers tsrch12 1.75 dimensionless multiple Temporal recharge multipliers tsrch12 1.75 dimensionless multiple Temporal recharge multipliers tsrch13 1.01 dimensionless multiple Temporal recharge multipliers tsrch14 1.20 dimensionless multiple Temporal recharge multipliers tsrch14 1.20 dimensionless multiple Temporal recharge multipliers tsrch15 0.75 dimensionless multiple Temporal recharge multipliers tsrch15 0.75 dimensionless multiple Temporal recharge multipliers tsrch16 1.41 dimensionless multiple Temporal recharge multipliers tsrch17 1.11 dimensionless multiple Temporal recharge multipliers tsrch18 1.13 dimensionless multiple Temporal recharge multipliers tsrch18 1.13 dimensionless multiple Temporal recharge multipliers tsrch20 1.56 dimensionless multiple Temporal r	Parameter description	Parameter name	Value or range	Units	Model layer	
Specific storage ss8 6.07e-07 1/ft 8 Specific storage ss9 6.07e-07 1/ft 9 Recharge pilot points reh 0.00e+00 to 8.34e+01 in/year multiple Temporal recharge multipliers tsrch1 1.25 dimensionless multiple Temporal recharge multipliers tsrch2 1.25 dimensionless multiple Temporal recharge multipliers tsrch3 1.25 dimensionless multiple Temporal recharge multipliers tsrch4 0.96 dimensionless multiple Temporal recharge multipliers tsrch5 0.82 dimensionless multiple Temporal recharge multipliers tsrch6 1.16 dimensionless multiple Temporal recharge multipliers tsrch8 0.84 dimensionless multiple Temporal recharge multipliers tsrch9 0.77 dimensionless multiple Temporal recharge multipliers tsrch10 0.44 dimensionless multiple Temporal recharge multipliers tsrch11 0.95 dimensionless multiple Temporal recharge multipliers tsrch12 1.75 dimensionless multiple Temporal recharge multipliers tsrch12 1.75 dimensionless multiple Temporal recharge multipliers tsrch14 1.20 dimensionless multiple Temporal recharge multipliers tsrch14 1.20 dimensionless multiple Temporal recharge multipliers tsrch14 1.20 dimensionless multiple Temporal recharge multipliers tsrch15 0.75 dimensionless multiple Temporal recharge multipliers tsrch14 1.20 dimensionless multiple Temporal recharge multipliers tsrch15 0.75 dimensionless multiple Temporal recharge multipliers tsrch15 1.41 dimensionless multiple Temporal recharge multipliers tsrch15 1.41 dimensionless multiple Temporal recharge multipliers tsrch16 1.41 dimensionless multiple Temporal recharge multipliers tsrch19 0.36 dimensionless multiple Temporal recharge multipliers tsrch20 1.56 dimensionless multiple Temporal recharge multipliers tsrch21 1.30 dimensionless multiple Temporal recharge multipliers tsrch21 1.30 dimensionless multiple Temporal recharge multipliers tsrch23 1.59 dimensionless multiple Tempo	Specific storage	ic storage ss6 3.59e-07		1/ft	6	
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Temporal recharge multipliers tsrch17 1.11 dimensionless multiple fremporal recharge multipliers tsrch18 1.13 dimensionless multiple fremporal recharge multipliers tsrch19 0.36 dimensionless multiple fremporal recharge multipliers tsrch20 1.56 dimensionless multiple fremporal recharge multipliers tsrch21 2.03 dimensionless multiple fremporal recharge multipliers tsrch21 1.43 dimensionless multiple fremporal recharge multipliers tsrch22 1.43 dimensionless multiple fremporal recharge multipliers tsrch23 1.59 dimensionless multiple fremporal recharge multipliers tsrch24 2.23 dimensionless multiple fremporal recharge multipliers tsrch25 1.78 dimensionless multiple fremporal recharge multipliers tsrch26 1.81 dimensionless multiple fremporal recharge multipliers tsrch26 1.81 dimensionless multiple fremporal recharge multipliers tsrch27 1.68 dimensionless multiple fremporal recharge multipliers tsrch28 1.21 dimensionless multiple fremporal recharge multiple fremporal rec	Temporal recharge multipliers	tsrch15	0.75	dimensionless	multiple	
Temporal recharge multipliers tsrch18 1.13 dimensionless multiple femporal recharge multipliers tsrch19 0.36 dimensionless multiple femporal recharge multipliers tsrch20 1.56 dimensionless multiple femporal recharge multipliers tsrch21 2.03 dimensionless multiple femporal recharge multipliers tsrch21 1.43 dimensionless multiple femporal recharge multipliers tsrch22 1.43 dimensionless multiple femporal recharge multipliers tsrch23 1.59 dimensionless multiple femporal recharge multipliers tsrch24 2.23 dimensionless multiple femporal recharge multipliers tsrch25 1.78 dimensionless multiple femporal recharge multipliers tsrch25 1.81 dimensionless multiple femporal recharge multipliers tsrch26 1.81 dimensionless multiple femporal recharge multipliers tsrch27 1.68 dimensionless multiple femporal recharge multipliers tsrch28 1.21 dimensionless multiple femporal recharge multipliers tsrch28 1.21 dimensionless multiple femporal recharge multiplers tsrch28 1.26 to 252,007.55 ft²/d multiple fereambed conductance huc105 1.46 to 702,377.67 ft²/d multiple fereambed conductance huc106 57.35 to 459,043.40 ft²/d multiple fereambed conductance huc107 4.90 to 83,232.99 ft²/d multiple fereambed conductance huc108 18.64 to 150,218.55 ft²/d multiple fereambed conductance huc108 18.64 to 150,218.55 ft²/d multiple fereambed conductance huc104 1.17 to 49,495.58 ft²/d multiple fereambed conductance	Temporal recharge multipliers	tsrch16	1.41	dimensionless	multiple	
Temporal recharge multipliers tsrch20 1.56 dimensionless multiple fremporal recharge multipliers tsrch21 2.03 dimensionless multiple fremporal recharge multipliers tsrch21 2.03 dimensionless multiple fremporal recharge multipliers tsrch22 1.43 dimensionless multiple fremporal recharge multipliers tsrch23 1.59 dimensionless multiple fremporal recharge multipliers tsrch23 1.59 dimensionless multiple fremporal recharge multipliers tsrch24 2.23 dimensionless multiple fremporal recharge multipliers tsrch25 1.78 dimensionless multiple fremporal recharge multipliers tsrch25 1.81 dimensionless multiple fremporal recharge multipliers tsrch26 1.81 dimensionless multiple fremporal recharge multipliers tsrch27 1.68 dimensionless multiple fremporal recharge multipliers tsrch28 1.21 dimensionless multiple fremporal recharge multipliers tsrch28 1.21 dimensionless multiple fremporal recharge multipliers tsrch28 1.21 dimensionless multiple fremporal recharge denductance huc103 2.25 to 252,007.55 ft²/d multiple fremporal recharge denductance huc105 1.46 to 702,377.67 ft²/d multiple fremporal conductance huc106 57.35 to 459,043.40 ft²/d multiple fremporal conductance huc107 4.90 to 83,232.99 ft²/d multiple fremporal conductance huc108 18.64 to 150,218.55 ft²/d multiple fremporal conductance huc108 18.64 to 150,218.55 ft²/d multiple fremporal conductance huc104 1.17 to 49,495.58 ft²/d multiple fremporal conductance huc114 1.17 to 49,495.58 ft²/d multiple fremporal conductance huc104 1.17 to 49,495.58 ft²/d multiple fremporal conductance huc105 huc104 1.17 to 49,495.58 ft²/d multiple fremporal conductance huc104 1.17 to 49,495.58 ft²/d multiple fremporal conductance huc105 huc104 1.17 to 49,495.58 ft²/d multiple fremporal conductance huc104 1.17 to 49,495.58 ft²/d multiple fremporal conductance huc105 huc104 1.17 to 49,495.58 ft²/d multiple fremporal conductance huc105 huc104 1.17 to 49,495.58 ft²/d multiple fremporal conductance huc105 huc104 ft²/d multiple fremporal conductance huc105 huc105 ft²/d multiple fremporal conductance h	Temporal recharge multipliers	tsrch17	1.11	dimensionless	multiple	
Femporal recharge multipliers tsrch20 1.56 dimensionless multiple femporal recharge multipliers tsrch21 2.03 dimensionless multiple femporal recharge multipliers tsrch22 1.43 dimensionless multiple femporal recharge multipliers tsrch23 1.59 dimensionless multiple femporal recharge multipliers tsrch24 2.23 dimensionless multiple femporal recharge multipliers tsrch24 1.78 dimensionless multiple femporal recharge multipliers tsrch25 1.78 dimensionless multiple femporal recharge multipliers tsrch26 1.81 dimensionless multiple femporal recharge multipliers tsrch26 1.81 dimensionless multiple femporal recharge multipliers tsrch27 1.68 dimensionless multiple femporal recharge multipliers tsrch28 1.21 dimensionless multiple femporal recharge multipliers tsrch28 1.21 dimensionless multiple ferembed conductance huc103 2.25 to 252,007.55 ft²/d multiple ferembed conductance huc105 1.46 to 702,377.67 ft²/d multiple ferembed conductance huc106 57.35 to 459,043.40 ft²/d multiple ferembed conductance huc107 4.90 to 83,232.99 ft²/d multiple ferembed conductance huc108 18.64 to 150,218.55 ft²/d multiple ferembed conductance huc108 18.64 to 150,218.55 ft²/d multiple ferembed conductance huc108 18.64 to 150,218.55 ft²/d multiple ferembed conductance huc114 1.17 to 49,495.58 ft²/d multiple ferembed conductance	Temporal recharge multipliers	tsrch18	1.13	dimensionless	multiple	
Temporal recharge multipliers tsrch21 2.03 dimensionless multiple femporal recharge multipliers tsrch22 1.43 dimensionless multiple femporal recharge multipliers tsrch23 1.59 dimensionless multiple femporal recharge multipliers tsrch24 2.23 dimensionless multiple femporal recharge multipliers tsrch25 1.78 dimensionless multiple femporal recharge multipliers tsrch25 1.81 dimensionless multiple femporal recharge multipliers tsrch26 1.81 dimensionless multiple femporal recharge multipliers tsrch27 1.68 dimensionless multiple femporal recharge multipliers tsrch27 1.68 dimensionless multiple femporal recharge multipliers tsrch28 1.21 dimensionless multiple ferembed conductance huc103 2.25 to 252,007.55 ft²/d multiple ferembed conductance huc105 1.46 to 702,377.67 ft²/d multiple ferembed conductance huc106 57.35 to 459,043.40 ft²/d multiple ferembed conductance huc107 4.90 to 83,232.99 ft²/d multiple ferembed conductance huc108 18.64 to 150,218.55 ft²/d multiple ferembed conductance huc104 1.17 to 49,495.58 ft²/d multiple ferembed conductance	Temporal recharge multipliers	tsrch19	0.36	dimensionless	multiple	
Temporal recharge multipliers tsrch21 2.03 dimensionless multiple femporal recharge multipliers tsrch22 1.43 dimensionless multiple femporal recharge multipliers tsrch23 1.59 dimensionless multiple femporal recharge multipliers tsrch24 2.23 dimensionless multiple femporal recharge multipliers tsrch25 1.78 dimensionless multiple femporal recharge multipliers tsrch25 1.81 dimensionless multiple femporal recharge multipliers tsrch26 1.81 dimensionless multiple femporal recharge multipliers tsrch27 1.68 dimensionless multiple femporal recharge multipliers tsrch27 1.68 dimensionless multiple femporal recharge multipliers tsrch28 1.21 dimensionless multiple ferembed conductance huc103 2.25 to 252,007.55 ft²/d multiple ferembed conductance huc105 1.46 to 702,377.67 ft²/d multiple ferembed conductance huc106 57.35 to 459,043.40 ft²/d multiple ferembed conductance huc107 4.90 to 83,232.99 ft²/d multiple ferembed conductance huc108 18.64 to 150,218.55 ft²/d multiple ferembed conductance huc104 1.17 to 49,495.58 ft²/d multiple ferembed conductance huc114 1.17 to 49,495.58 ft²/d multiple	Temporal recharge multipliers	tsrch20	1.56	dimensionless	multiple	
Temporal recharge multipliers tsrch22 1.43 dimensionless multiple formporal recharge multipliers tsrch23 1.59 dimensionless multiple fremporal recharge multipliers tsrch24 2.23 dimensionless multiple fremporal recharge multipliers tsrch25 1.78 dimensionless multiple fremporal recharge multipliers tsrch26 1.81 dimensionless multiple fremporal recharge multipliers tsrch26 1.81 dimensionless multiple fremporal recharge multipliers tsrch27 1.68 dimensionless multiple fremporal recharge multipliers tsrch28 1.21 dimensionless multiple fremporal recharge multipliers fremporal recharge multipliers tsrch28 1.21 dimensionless multiple fremporal recharge multipliers fremporal recharge multipliers tsrch28 1.21 dimensionless multiple fremporal recharge multipliers fremporal recharge multipliers tsrch28 1.21 dimensionless multiple fremporal recharge multipliers fremporal recharge multipliers tsrch28 1.21 dimensionless multiple fremporal recharge multipliers fremporal recharge multipliers tsrch27 1.68 dimensionless multiple fremporal recharge multipliers fremporal rech	Temporal recharge multipliers	tsrch21	2.03	dimensionless	multiple	
Temporal recharge multipliers tsrch23 1.59 dimensionless multiple tsrch24 2.23 dimensionless multiple tsrch25 1.78 dimensionless multiple tsrch25 1.78 dimensionless multiple tsrch25 1.81 dimensionless multiple tsrch26 1.81 dimensionless multiple tsrch27 1.68 dimensionless multiple tsrch27 1.68 dimensionless multiple tsrch27 1.68 dimensionless multiple tsrch28 1.21 dimensionless multiple tsrch29 thuc103 2.25 to 252,007.55 ft²/d multiple tsrch29 thuc105 1.46 to 702,377.67 ft²/d multiple tsrch29 thuc106 57.35 to 459,043.40 ft²/d multiple tsrch29 thuc107 4.90 to 83,232.99 ft²/d multiple tsrch29 thuc108 18.64 to 150,218.55 ft²/d multiple tsrch29 thuc108 18.64 to 150,218.55 ft²/d multiple tsrch29 thuc108 18.64 to 150,218.55 ft²/d multiple tsrch29 thuc114 1.17 to 49,495.58 ft²/d multiple tsrch29 thuc129	Temporal recharge multipliers	tsrch22	1.43	dimensionless	multiple	
Temporal recharge multipliers tsrch24 2.23 dimensionless multiple femporal recharge multipliers tsrch25 1.78 dimensionless multiple femporal recharge multipliers tsrch26 1.81 dimensionless multiple femporal recharge multipliers tsrch27 1.68 dimensionless multiple femporal recharge multipliers tsrch27 1.68 dimensionless multiple femporal recharge multipliers tsrch28 1.21 dimensionless multiple fitreambed conductance huc103 2.25 to 252,007.55 ft²/d multiple fitreambed conductance huc105 1.46 to 702,377.67 ft²/d multiple fitreambed conductance huc106 57.35 to 459,043.40 ft²/d multiple fitreambed conductance huc107 4.90 to 83,232.99 ft²/d multiple fitreambed conductance huc108 18.64 to 150,218.55 ft²/d multiple fitreambed conductance huc108 18.64 to 150,218.55 ft²/d multiple fitreambed conductance huc114 1.17 to 49,495.58 ft²/d multiple fitreambed conductance	Temporal recharge multipliers	tsrch23	1.59	dimensionless	multiple	
Temporal recharge multipliers tsrch25 1.78 dimensionless multiple fremporal recharge multipliers tsrch26 1.81 dimensionless multiple fremporal recharge multipliers tsrch27 1.68 dimensionless multiple fremporal recharge multipliers tsrch28 1.21 dimensionless multiple fremporal recharge multiple fremporal recharge multipliers tsrch28 1.21 dimensionless multiple fremporal recharge multipliers tsrch28 1.21 dimensionless multiple fremporal recharge multipliers tsrch28 1.21 dimensionless multiple fremporal recharge mult	Temporal recharge multipliers	tsrch24	2.23	dimensionless	multiple	
Temporal recharge multipliers tsrch26 1.81 dimensionless multiple femporal recharge multipliers tsrch27 1.68 dimensionless multiple femporal recharge multipliers tsrch28 1.21 dimensionless multiple fitterambed conductance huc103 2.25 to 252,007.55 ft²/d multiple fitterambed conductance huc105 1.46 to 702,377.67 ft²/d multiple fitterambed conductance huc106 57.35 to 459,043.40 ft²/d multiple fitterambed conductance huc107 4.90 to 83,232.99 ft²/d multiple fitterambed conductance huc108 18.64 to 150,218.55 ft²/d multiple fitterambed conductance huc114 1.17 to 49,495.58 ft²/d multiple fitterambed conductance huc114 1.17 to 49,495.58 ft²/d multiple fitterambed conductance	Temporal recharge multipliers	tsrch25	1.78	dimensionless	multiple	
Femporal recharge multipliers tsrch27 1.68 dimensionless multiple femporal recharge multipliers tsrch28 1.21 dimensionless multiple fitterambed conductance huc103 2.25 to 252,007.55 ft²/d multiple fitterambed conductance huc105 1.46 to 702,377.67 ft²/d multiple fitterambed conductance huc106 57.35 to 459,043.40 ft²/d multiple fitterambed conductance huc107 4.90 to 83,232.99 ft²/d multiple fitterambed conductance huc108 18.64 to 150,218.55 ft²/d multiple fitterambed conductance huc108 18.64 to 150,218.55 ft²/d multiple fitterambed conductance huc114 1.17 to 49,495.58 ft²/d multiple	Temporal recharge multipliers	tsrch26	1.81	dimensionless	multiple	
Temporal recharge multipliers tsrch28 1.21 dimensionless multiple Streambed conductance huc103 2.25 to 252,007.55 ft²/d multiple Streambed conductance huc105 1.46 to 702,377.67 ft²/d multiple Streambed conductance huc106 57.35 to 459,043.40 ft²/d multiple Streambed conductance huc107 4.90 to 83,232.99 ft²/d multiple Streambed conductance huc108 18.64 to 150,218.55 ft²/d multiple Streambed conductance huc114 1.17 to 49,495.58 ft²/d multiple		tsrch27		dimensionless	multiple	
Streambed conductance huc103 2.25 to 252,007.55 ft²/d multiple streambed conductance huc105 1.46 to 702,377.67 ft²/d multiple streambed conductance huc106 57.35 to 459,043.40 ft²/d multiple streambed conductance huc107 4.90 to 83,232.99 ft²/d multiple streambed conductance huc108 18.64 to 150,218.55 ft²/d multiple streambed conductance huc114 1.17 to 49,495.58 ft²/d multiple		tsrch28	1.21	dimensionless		
Streambed conductance huc105 1.46 to 702,377.67 ft²/d multiple streambed conductance huc106 57.35 to 459,043.40 ft²/d multiple streambed conductance huc107 4.90 to 83,232.99 ft²/d multiple streambed conductance huc108 18.64 to 150,218.55 ft²/d multiple streambed conductance huc114 1.17 to 49,495.58 ft²/d multiple					multiple	
Streambed conductance huc 106 57.35 to $459,043.40$ ft ² /d multiple streambed conductance huc 107 4.90 to $83,232.99$ ft ² /d multiple streambed conductance huc 108 18.64 to $150,218.55$ ft ² /d multiple streambed conductance huc 114 1.17 to $49,495.58$ ft ² /d multiple					multiple	
Streambed conductance huc 107 4.90 to $83,232.99$ ft ² /d multiple streambed conductance huc 108 18.64 to $150,218.55$ ft ² /d multiple streambed conductance huc 114 1.17 to $49,495.58$ ft ² /d multiple multiple					-	
Streambed conductance huc 108 18.64 to $150,218.55$ ft ² /d multiple streambed conductance huc 114 1.17 to $49,495.58$ ft ² /d multiple					multiple	
Streambed conductance huc114 1.17 to 49,495.58 ft²/d multiple					multiple	
, , ,						
multiple	Streambed conductance	huc116	0.17 to 291,771.00	ft²/d	multiple	

Table 1.1. Final parameter estimates for the Ozark groundwater-flow model, version 1.1.—Continued [All aquifer properties exclude Mississippi embayment values. ft/d, foot per day; ft, foot; ft²/d, foot squared per day]

Parameter description	Parameter name	Value or range	Units	Model layer	
Streambed conductance	huc117	2.27 to 326,834.09	ft²/d	multiple	
Streambed conductance	huc118	4.74 to 179,740.60	ft²/d	multiple	
Streambed conductance	huc119	8.26 to 208,010.33	ft²/d	multiple	
Streambed conductance	huc12	8.59 to 664,106.94	ft²/d	multiple	
Streambed conductance	huc120	2.81 to 20,511.80	ft²/d	multiple	
Streambed conductance	huc122	559,899.54	ft²/d	multiple	
Streambed conductance	huc130	7.84 to 383,902.08	ft²/d	multiple	
Streambed conductance	huc131	340.60 to 1,714.00	ft²/d	multiple	
Streambed conductance	huc132	3.91 to 38,401.01	ft²/d	multiple	
Streambed conductance	huc134	4.21 to 168,283.75	ft^2/d	multiple	
Streambed conductance	huc135	0.14 to 20,693.06	ft²/d	multiple	
Streambed conductance	huc136	28.30 to 13,389.55	ft²/d	multiple	
Streambed conductance	huc138	13.14 to 54,109.02	ft²/d	multiple	
Streambed conductance	huc140	0.55 to 63,070.31	ft²/d	multiple	
Streambed conductance	huc141	0.70 to 41,191.99	ft²/d	multiple	
Streambed conductance	huc146	0.52 to 75,094.76	ft²/d	multiple	
Streambed conductance	huc147	358.83 to 669,014.58	ft²/d	multiple	
Streambed conductance	huc149	10.20 to 64,893.25	ft²/d	multiple	
Streambed conductance	huc150	128.63 to 35,130.08	ft²/d	multiple	
Streambed conductance	huc151	4.35 to 46,252.46	ft²/d	multiple	
Streambed conductance	huc153	103.38 to 10,021.21	ft²/d	multiple	
Streambed conductance	huc154	0.05 to 322,894.70	ft²/d	multiple	
Streambed conductance	huc170	2.70 to 45,926.96	ft²/d	multiple	
Streambed conductance	huc176	8.50 to 207,959.66	ft²/d	multiple	
Streambed conductance	huc177	0.28 to 57,010.92	ft²/d	multiple	
Streambed conductance	huc178	4.49 to 90,697.67	ft²/d	multiple	
Streambed conductance	huc179	26.08 to 141,080.48	ft²/d	multiple	
Streambed conductance	huc180	33.70 to 12,512.50	ft²/d	multiple	
Streambed conductance	huc182	99.46 to 111,272.99	ft²/d	multiple	
Streambed conductance	huc183	4.39 to 93,831.63	ft²/d	multiple	
Streambed conductance	huc184	0.29 to 127,314.33	ft²/d	multiple	
Streambed conductance	huc187	9.17 to 70,127.45	ft²/d	multiple	
Streambed conductance	huc188	48.57 to 583,800.32	ft²/d	multiple	
Streambed conductance	huc208	949.59 to 52,659,311.40	ft²/d	multiple	
Streambed conductance	huc214	2.51 to 159,546.98	ft²/d	multiple	
Streambed conductance	huc218	0.15 to 681,577.72	ft²/d	multiple	
Streambed conductance	huc241	0.41 to 424,606.15	ft²/d	multiple	
Streambed conductance	huc242	2.19 to 54,295.00	ft²/d	multiple	
Streambed conductance	huc243	3.12 to 545,916.13	ft²/d	multiple	
Streambed conductance	huc251	15.25 to 26,320.59	ft²/d	multiple	

 Table 1.1.
 Final parameter estimates for the Ozark groundwater-flow model, version 1.1.—Continued

 $[All\ aquifer\ properties\ exclude\ Mississippi\ embayment\ values.\ ft/d,\ foot\ per\ day;\ ft,\ foot;\ ft^2/d,\ foot\ squared\ per\ day]$

Parameter description	Parameter name	Value or range	Units	Model layer
Streambed conductance	huc77	5.58 to 268,638.33	ft²/d	multiple
Streambed conductance	huc78	12.32 to 187,324.32	ft²/d	multiple
Streambed conductance	huc79	0.07 to 94,885.14	ft²/d	multiple
Streambed conductance	huc80	3.14 to 129,859.03	ft²/d	multiple
Streambed conductance	huc81	9.45 to 773,708.89	ft²/d	multiple
Streambed conductance	huc82	5.10 to 432,075.26	ft²/d	multiple
Streambed conductance	huc83	1.76 to 62,579.21	ft²/d	multiple
Streambed conductance	huc84	11.86 to 911,069.54	ft²/d	multiple
Streambed conductance	huc90	15.88 to 121,904.61	ft²/d	multiple
Streambed conductance	huc98	7.71 to 1,255,615.89	ft²/d	multiple
Streambed conductance	huc99	6.47 to 1,421,376.07	ft²/d	multiple
General head multiplier	ghb-1	0.68	dimensionless	multiple
Constant head multiplier	chd-1	0.81	dimensionless	multiple
Constant head multiplier	chd-6	0.77	dimensionless	multiple
Constant head multiplier	chd-9	0.58	dimensionless	multiple
Horizontal flow multiplier	hfb-1	3.93	dimensionless	multiple
Pumping multiplier	wel-0	1.00	dimensionless	multiple
Pumping multiplier	wel-1	1.48	dimensionless	multiple
Pumping multiplier	wel-2	1.29	dimensionless	multiple
Pumping multiplier	wel-3	1.08	dimensionless	multiple
Pumping multiplier	wel-4	1.44	dimensionless	multiple
Pumping multiplier	wel-5	1.45	dimensionless	multiple
Pumping multiplier	wel-6	1.44	dimensionless	multiple
Pumping multiplier	wel-7	1.53	dimensionless	multiple
Pumping multiplier	wel-8	1.48	dimensionless	multiple
Pumping multiplier	wel-9	1.41	dimensionless	multiple
Pumping multiplier	wel-10	1.01	dimensionless	multiple
Pumping multiplier	wel-11	1.13	dimensionless	multiple
Pumping multiplier	wel-12	1.07	dimensionless	multiple
Pumping multiplier	wel-13	0.89	dimensionless	multiple
Pumping multiplier	wel-14	0.89	dimensionless	multiple
Pumping multiplier	wel-15	0.98	dimensionless	multiple
Pumping multiplier	wel-16	0.94	dimensionless	multiple
Pumping multiplier	wel-17	0.96	dimensionless	multiple
Pumping multiplier	wel-18	1.02	dimensionless	multiple
Pumping multiplier	wel-19	1.01	dimensionless	multiple
Pumping multiplier	wel-20	1.04	dimensionless	multiple
Pumping multiplier	wel-21	0.48	dimensionless	multiple
Pumping multiplier	wel-22	1.02	dimensionless	multiple
Pumping multiplier	wel-23	0.45	dimensionless	multiple

Table 1.1. Final parameter estimates for the Ozark groundwater-flow model, version 1.1.—Continued [All aquifer properties exclude Mississippi embayment values. ft/d, foot per day; ft, foot; ft²/d, foot squared per day]

Parameter description	Parameter name	Value or range	Units	Model layer
Pumping multiplier	wel-24	1.00	dimensionless	multiple
Pumping multiplier	wel-25	0.76	dimensionless	multiple
Pumping multiplier	wel-26	0.71	dimensionless	multiple
Pumping multiplier	wel-27	1.00	dimensionless	multiple
Pumping multiplier	wel-28	1.01	dimensionless	multiple
Pumping multiplier	wel-29	0.45	dimensionless	multiple
Pumping multiplier	wel-30	0.94	dimensionless	multiple
Pumping multiplier	wel-31	0.60	dimensionless	multiple
Pumping multiplier	wel-32	0.45	dimensionless	multiple
Pumping multiplier	wel-33	0.61	dimensionless	multiple
Pumping multiplier	wel-34	0.45	dimensionless	multiple
Pumping multiplier	wel-35	0.67	dimensionless	multiple
Pumping multiplier	wel-36	0.70	dimensionless	multiple
Pumping multiplier	wel-37	0.72	dimensionless	multiple
Pumping multiplier	wel-38	0.66	dimensionless	multiple
Pumping multiplier	wel-39	0.88	dimensionless	multiple
Pumping multiplier	wel-40	0.63	dimensionless	multiple
Pumping multiplier	wel-41	0.98	dimensionless	multiple
Pumping multiplier	wel-42	2.44	dimensionless	multiple
Pumping multiplier	wel-43	1.25	dimensionless	multiple
Pumping multiplier	wel-44	1.37	dimensionless	multiple
Pumping multiplier	wel-45	1.12	dimensionless	multiple
Pumping multiplier	wel-46	1.37	dimensionless	multiple
Pumping multiplier	wel-47	2.22	dimensionless	multiple
Pumping multiplier	wel-48	1.15	dimensionless	multiple
Pumping multiplier	wel-49	1.36	dimensionless	multiple
Pumping multiplier	wel-50	1.07	dimensionless	multiple
Pumping multiplier	wel-51	0.47	dimensionless	multiple
Pumping multiplier	wel-52	1.10	dimensionless	multiple
Pumping multiplier	wel-53	1.05	dimensionless	multiple
Pumping multiplier	wel-54	1.61	dimensionless	multiple
Pumping multiplier	wel-55	1.03	dimensionless	multiple
Pumping multiplier	wel-56	1.04	dimensionless	multiple
Pumping multiplier	wel-57	1.46	dimensionless	multiple
Pumping multiplier	wel-58	1.02	dimensionless	multiple
Pumping multiplier	wel-59	0.95	dimensionless	multiple
Pumping multiplier	wel-60	0.65	dimensionless	multiple
Pumping multiplier	wel-61	0.64	dimensionless	multiple
Pumping multiplier	wel-62	1.00	dimensionless	multiple
Pumping multiplier	wel-63	0.99	dimensionless	multiple

 Table 1.1.
 Final parameter estimates for the Ozark groundwater-flow model, version 1.1.—Continued

 $[All\ aquifer\ properties\ exclude\ Mississippi\ embayment\ values.\ ft/d,\ foot\ per\ day;\ ft,\ foot;\ ft^2/d,\ foot\ squared\ per\ day]$

Parameter description	Parameter name	Value or range	Units	Model layer	
Pumping multiplier	wel-64	0.73	dimensionless	multiple	
Pumping multiplier	wel-65	1.40	dimensionless	multiple	
Pumping multiplier	wel-66	1.01	dimensionless	multiple	
Pumping multiplier	wel-67	0.93	dimensionless	multiple	
Pumping multiplier	wel-68	0.64	dimensionless	multiple	
Pumping multiplier	wel-69	0.93	dimensionless	multiple	
Pumping multiplier	wel-70	0.63	dimensionless	multiple	
Pumping multiplier	wel-71	0.97	dimensionless	multiple	
Pumping multiplier	wel-72	0.45	dimensionless	multiple	
Pumping multiplier	wel-73	0.93	dimensionless	multiple	
Pumping multiplier	wel-74	0.67	dimensionless	multiple	
Pumping multiplier	wel-75	0.69	dimensionless	multiple	
Pumping multiplier	wel-76	1.08	dimensionless	multiple	
Pumping multiplier	wel-77	1.50	dimensionless	multiple	
Pumping multiplier	wel-78	1.00	dimensionless	multiple	
Pumping multiplier	wel-79	1.00	dimensionless	multiple	

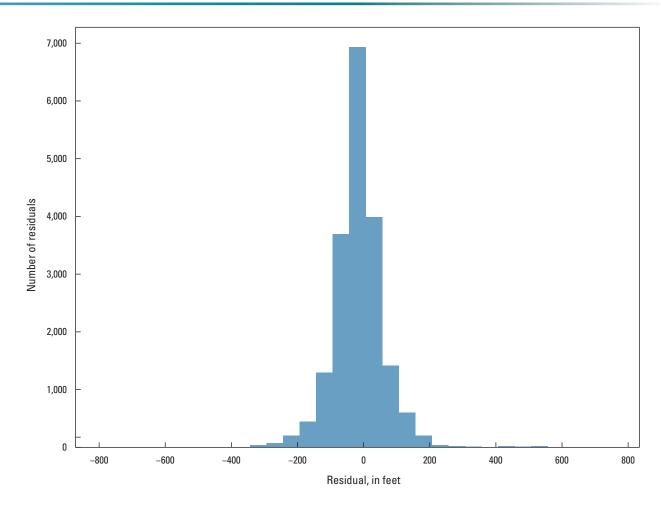


Figure 1.1. Distribution of unweighted hydraulic-head residuals.

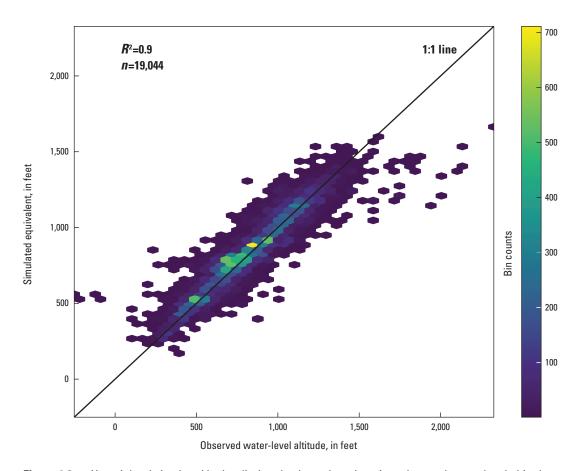


Figure 1.2. Unweighted simulated hydraulic-head values plotted against observed water-level altitudes.

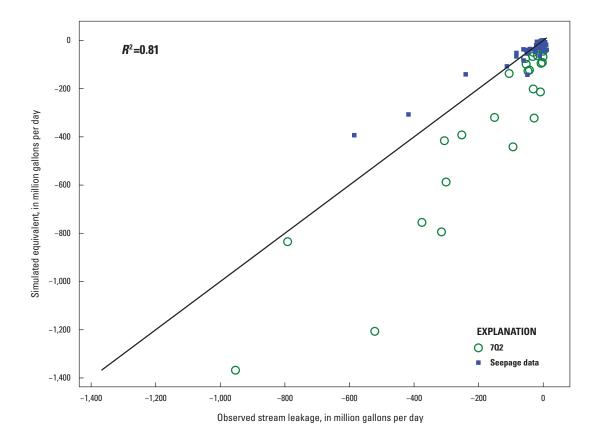


Figure 1.3. Simulated stream leakage plotted against observed stream leakage (seepage data from Knierim and others [2015]). The 7-day, 2-year (702) annual low-flow statistic is the annual 7-day minimum flow with a 2-year recurrence interval (nonexceedance probability of 50 percent).

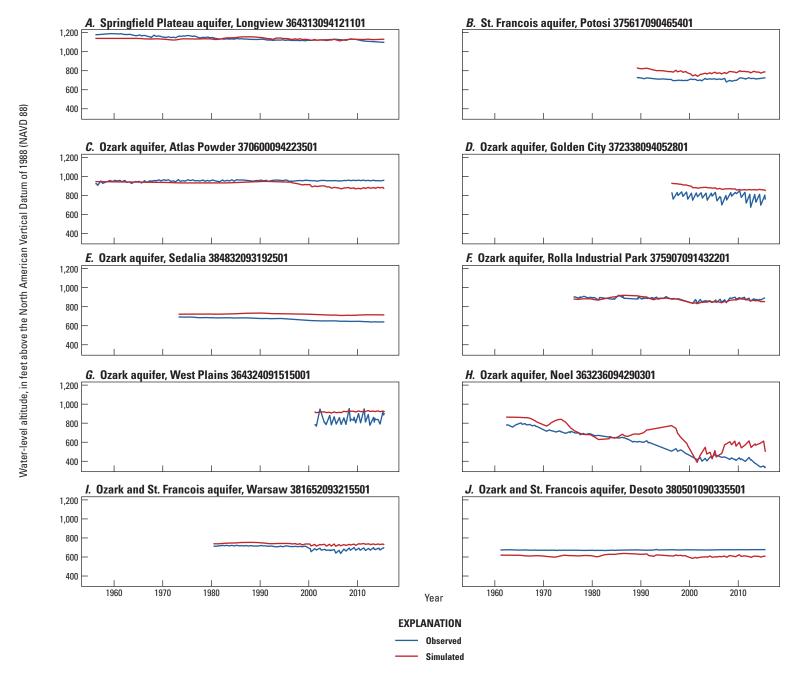


Figure 1.4. Simulated and observed water levels in select U.S. Geological Survey real-time observation wells (U.S. Geological Survey, 2015). The number above each hydrograph is the U.S. Geological Survey station number.

Appendix 2

This appendix contains additional information regarding the linear uncertainty analyses used to quantify: (1) parameter uncertainty, (2) prediction uncertainty, (3) the worth of data pertaining directly to parameters, and (4) the worth of data pertaining indirectly to observations, when using the three scenarios simulating future potential conditions. Head forecast identifiers are arbitrary numbers assigned to each head prediction for the purpose of differentiation. Some head predictions are located at the same latitude and longitude but are located in different model layers (table 2.1).

Parameter Uncertainty

The prior parameter uncertainty estimates for adjustable model parameters in the predictive analysis were based on a range of potential values for the parameters prior to the history-matching process. Typically, this stochastic information is derived from expert knowledge, literature values, field tests, or earlier models. History-matching produces the best-fit parameter values, and the posterior (after history matching) uncertainty of adjustable model parameters then can be estimated by using linear uncertainty analysis. Schur's complement (Meyer, 2000) for conditional uncertainty propagation (White and others, 2015) is

$$\overline{\Sigma_{\theta}} = \Sigma_{\theta} - \Sigma_{\theta} J^{T} \left(J \Sigma_{\theta} J^{T} + \Sigma_{\varepsilon} \right)^{-1} J \Sigma_{\theta}$$
 (2.1)

where

 $\overline{\Sigma_{\theta}}$ is the posterior parameter covariance matrix,

 Σ_{θ} is the prior parameter covariance matrix,

J is the Jacobian matrix of partial first derivatives of observations with respect to parameters, and

 Σ_{ε} is the covariance matrix of observation noise.

The second term in the above equation $(\Sigma_{\theta}J^T(J\Sigma_{\theta}J^T+\Sigma_{\varepsilon})^{-1}J\Sigma_{\theta})$ encapsulates the conditioning provided by the observations through linear mapping of information from observations to parameters via the Jacobian matrix. Use of equation 2.1 assumes (1) a linear relationship between adjustable parameters and model-simulated observation equivalents and (2) multivariate Gaussian (or log-Gaussian) distributions to describe the stochastic character of parameters, predictions, and observation noise (Fienen and others, 2010; White and others, 2015). Therefore, the primary quantitative metric for uncertainty in a linear framework is variance (Doherty and others, 2010).

Comparison of the prior and posterior variances associated with adjustable model parameters illustrates how the observations used in the history-matching process informs the adjustable model parameters. Similar prior and posterior variances for a given parameter imply that the observation dataset contributed little to inform that specific parameter. Conversely, a posterior variance that is smaller than the prior variance for a given parameter implies that the parameter is well informed by the observation dataset.

For example, the generalized head boundary multiplier has a posterior variance similar to the prior variance, which indicates that the observation dataset failed to decrease the uncertainty through history matching (table 3, main text), likely because of the relatively few numbers of observations and stress effects in the Ozark system near the Mississippi embayment. Again, the reduction in uncertainty tends to increase in areas with larger numbers of observations and stressed areas of the model. Specific storage parameters of the St. François aguifer and the St. François confining unit are well informed by the observation dataset, but the horizontal hydraulic conductivity of the St. Francois aquifer and the vertical hydraulic conductivity of the St. François confining unit are not (table 3, main text). Head observations, therefore seem to provide more information for these deeper units than flow observations such as stream leakage, because most of the area of the units is below the influence of groundwatersurface-water interaction. Uncertainty reduction of parameters related to aquifer properties of the lower Ozark aquifer tends to be low, though the differences in the maximum and minimum reduction tend to be large, indicating that uncertainty at some pilot-point parameters was greatly reduced, likely in areas of higher stress with more nearby observations. Specific storage and specific yield parameters for the middle Ozark aquifer were only marginally informed by the observation dataset, potentially because observations are included in the confined and unconfined parts of the unit. Most of the horizontal hydraulic conductivity pilot-point parameters in the upper Ozark aquifer have posterior variances similar to the prior variances, which indicates that the observation dataset failed to decrease the uncertainties of these parameters through history matching (table 3, main text). The specific storage parameter for the St. Francois aquifer is well informed by the observation dataset, as the posterior variance is much lower than the prior variance. Similarly, the specific yield parameter for the Springfield Plateau aquifer is well informed by the observation dataset. Specific yield parameters for the Ozark confining unit are only marginally informed by the observation dataset. The specific storage parameter for the Western Interior Plains confining system is well informed by the observation dataset, and the specific yield parameter is only marginally informed by the observation dataset.

Differences between prior and posterior variances for most pilot-point parameters were mixed. Some pilot-point parameter groups had a higher proportion of larger reductions in posterior variances than other pilot-point parameter groups—for example, horizontal hydraulic conductivity in the lower Ozark aquifer and vertical hydraulic conductivities in

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 Table 2.1.
 Head forecast identifiers and associated model layers for the Ozark groundwater-flow model, version 1.1.

Head forecast identifier (see figs. 15–17)	Model layer	Head forecast identifier (see figs. 15–17)	Model layer	Head forecast identifier (see figs. 15–17)	Model layer
0	6	31	6	62	6
1	9	32	6	63	6
2	6	33	6	65	6
3	2	34	9	66	6
4	6	35	9	67	6
5	6	36	6	68	6
6	6	37	6	69	6
7	6	38	6	70	6
8	6	39	6	71	6
9	6	40	6	72	6
10	2	41	6	73	6
11	6	42	6	74	6
12	6	43	6	75	6
13	6	44	6	76	6
14	6	45	6	77	6
15	6	46	6	78	6
16	6	47	2	79	6
17	6	48	6	80	6
18	6	49	6	81	6
19	6	50	6	82	6
20	6	51	6	83	6
21	6	52	2	84	6
22	6	53	6	85	6
23	6	54	2	86	6
24	6	55	2	87	6
25	6	56	6	88	6
26	6	57	2	89	9
27	6	58	6	90	6
28	6	59	6	91	6
29	6	60	6	93	6
30	6	61	6	94	6

the middle Ozark aquifer and Western Interior Plains confining system (table 3, main text). On the other hand, some pilot-point parameter groups had few large reductions in posterior variances—for example, specific yield in the lower Ozark aquifer, horizontal hydraulic conductivity in the St. Francois aquifer, and vertical hydraulic conductivities in the Ozark confining unit and the St. Francois confining unit (table 3, main text).

Prediction Uncertainty

The best-fit parameters gathered through the history matching process represent minimum error variance parameters. That is, model parameters retain uncertainty even after they have been subjected to the history-matching process, and the parameters are still free to movement with constraints. Therefore, many different parameter sets can reproduce the observation dataset as well as the best-fit parameter set (non-unique solutions). Each of these different parameter sets, however, may produce different model predictions, especially when large posterior uncertainty remains in parameters to which these predictions are sensitive. Predictive linear uncertainty analysis was used to evaluate uncertainty in predictions made by groundwater models based on best-fit parameters with explicit consideration of parameter uncertainty.

To propagate parameter uncertainty to predictions of interest, the following equations (Doherty and others, 2010; White and others, 2015) were used:

$$\sigma_s^2 = y^T \Sigma_\theta \tag{2.2}$$

$$\overline{\sigma_s^2} = y^T \overline{\Sigma_\theta} y \tag{2.3}$$

where

is the sensitivity vector of prediction s with respect to each of the adjustable parameters, is the prior variance of prediction s, and all other terms are as previously defined in equation 2.1 for Schur's complement.

As previously stated, linear uncertainty analysis using Schur's complement assumes: (1) a linear relationship between adjustable parameters and model-simulated observation equivalents, and (2) multivariate Gaussian (or log-Gaussian) distributions to describe the stochastic character of parameters, predictions, and observation noise (Fienen and others, 2010; White and others, 2015). Predictive linear uncertainty analysis assumes that the history-matching process transfers information from the observation dataset to parameters to which predictions are sensitive, thereby reducing the uncertainty of the model predictions—for example, the posterior uncertainty about a prediction should be less than the prior uncertainty.

The prior parameter uncertainty estimates for the predictive analysis were based on a range of the potential values for the parameters prior to the history-matching process. Using equations 2.2 and 2.3, the prior parameter uncertainty estimates were propagated to the model predictions. To evaluate the effects of the history-matching process on the predictions of interest, equations 2.2 and 2.3 were combined with equation 2.1 to generate posterior prediction uncertainty estimates. The posterior prediction uncertainty estimates were then used to estimate the upper and lower bounds of the 95-percent credible interval for each model prediction. For the heads targeted as predictions of interest, the radius of the 95-percent credible interval represents a conservative estimate of the expected margin of error for the head forecasts under the three scenarios of potential future conditions. The history-matching process provided important information to inform prediction-sensitive model parameters, contributing to substantial decreases in the radii of the posterior 95-percent credible intervals around the head forecasts (when compared to the prior; figs. 22–24, main text).

The Worth of Data Pertaining Directly to Parameters

Evaluating which parameters or groups of parameters are contributing most to the uncertainty associated with a particular prediction quantifies the worth of the data and facilitates informed data collection. The uncertainty in parameters and predictions that remains after history matching depends upon the uncertainty in parameters prior to history matching and the information on parameters contained in the observation dataset used in the history-matching effort. Linear-based uncertainty analyses (White and others, 2016) were used to quantitatively evaluate which parameters or groups of parameters are contributing most to the prior and posterior uncertainty associated with the 94 head forecasts.

The posterior uncertainty associated with each prediction is calculated by systematically assuming perfect knowledge of a selected parameter or group of parameters, which is equivalent to reducing the uncertainty of that selected parameter or group of parameters. If a prediction is sensitive to this particular parameter or group of parameters, then the posterior variance—or uncertainty—associated with that prediction should also be reduced. Comparison of the posterior uncertainty calculated under the assumed perfect knowledge of a selected parameter or group of parameters to the posterior uncertainty calculated with the unaltered posterior parameter covariance matrix can be used to identify the dominant sources of parameter contribution to uncertainty for each prediction of interest.

The Worth of Data Pertaining Indirectly to Observations

Linear-based uncertainty analysis was also used to identify the most valuable head observations for predicting future conditions. The observations in the calibration dataset co-located with each of the 94 head forecasts at real-time observation wells were independently nominated as groups for removal (from the observation dataset used in the historymatching effort) for each of the three scenarios. The worth of the nominated observation or group of observations is measured as an increase in the posterior variance resulting from the loss of information, where the loss of information is the removed influence of the eliminated observations during the history-matching process. The percent increase in the posterior variance for each head forecast was calculated and then summed to gain a sense of how much predictive uncertainty increased for predicted water levels at all forecast locations. This type of data worth analysis is particularly useful in informing an existing groundwater monitoring network because it identifies which observations are most important in reducing predictive uncertainty for future water levels at all forecast locations.

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