SIMULATION OF STORM PEAKS AND STORM VOLUMES FOR SELECTED SUBBASINS IN THE WEST FORK TRINITY RIVER BASIN, TEXAS, WATER YEARS 1993–94

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Simulation of Storm Peaks and Storm Volumes for Selected Subbasins in the West Fork Trinity River Basin, Texas, Water Years 1993–94

By Timothy H. Raines

Abstract

A model parameter set for use with the Hydrologic Simulation Program—FORTRAN watershed model was developed to simulate storm peaks and storm volumes for the 28 subbasins of the West Fork Trinity River Basin upstream from Lake Worth, northwest of Fort Worth, Texas, from the calibration and testing of 5 gaged subbasins. These parameters can be transferred to the 23 ungaged subbasins. The model simulates storm runoff for a channel-routing model that can be used to improve reservoir operation during floods in the basin.

Rainfall and runoff data were collected from October 1, 1992, to September 30, 1994. A total of 55 storms were recorded at the 5 streamgage stations during the 24 months. Twelve different pervious land segments were defined based on types of soil, land cover, and watershed slope. A total of 20 process-related parameters were defined for each land segment, and 6 basin-related parameters were defined for each stream reach.

The mean absolute errors for the 5 subbasins for simulation of storm peaks range from 48.0 to 470 percent and for simulation of storm volumes range from 34.4 to 416 percent. A sensitivity analysis was done to determine what a change in a parameter value has on the largest storm peak and on the total storm volume. The model then was recalibrated and tested on the basis of the analysis of the sensitivity of parameters and on the analysis of the errors from the initial model calibration and testing. The mean absolute errors for the 5 subbasins using the recalibrated parameters for simulation of storm peaks range from 47.1 to 297 percent, and for simulation of storm volumes range from 27.6 to 193 percent.

The model produced better results for simulation of the larger storm peaks and storm volumes than for simulation of the smaller storm peaks and storm volumes, especially after an extended period of no runoff. The same range in errors can be expected when transferring the parameters to the 23 ungaged subbasins. Additional data collection and model refinement could decrease the range of expected model errors. More storm data and improved discharge rating curves could result in model parameters that account for the wide seasonal variations in runoff in the study area.

INTRODUCTION

Recent flooding (1990) in the West Fork Trinity River Basin upstream from Lake Worth, northwest of Fort Worth, Texas, has caused extensive damage to public and private property and has resulted in a need for improved flood forecasting to better manage reservoir operation. Although simulation models for flood routing in the main channel of the West Fork Trinity River and in reservoirs are available for the basin, rainfallrunoff data for the subbasins are lacking. Accurate estimates of storm runoff for the 28 subbasins that drain into the West Fork Trinity River Basin upstream from Lake Worth are needed for input into a channel-routing model. Previous attempts to use event-based simulation models have been unsatisfactory because the models were unable to accurately estimate the volume, magnitude, and timing of peak flows. A range in evaporation, infiltration, and storage capacities measured throughout the year results in a wide variation of storm runoff. Therefore, continuous-simulation models could provide

a better representation of seasonal variations of rainfall and runoff than event-based simulation models.

In 1991, the U.S. Geological Survey, in cooperation with the Tarrant County Water Control and Improvement District No. 1, began a study to develop a model parameter set to simulate storm peaks and volumes for selected subbasins in the West Fork Trinity River Basin. Specific objectives of the study were to (1) develop a set of parameters for a continuoussimulation model from the calibration and testing of 5 gaged subbasins; (2) transfer the parameter set to the remaining 23 ungaged subbasins of the West Fork Trinity River Basin upstream from Lake Worth; and (3) use the model to simulate storm runoff in the basin to improve reservoir operation during floods.

Purpose and Scope

This report describes the simulation of storm peaks and storm volumes for 5 gaged subbasins in the West Fork Trinity River Basin upstream from Lake Worth. Rainfall and runoff data from 55 storms used in this report were collected during water years 1993–94 (October 1, 1992, to September 30, 1994). Each subbasin was characterized by twelve various pervious land segments that were defined based on types of soil, land cover, and watershed slope and was subdivided into two to three reaches for input to a continuous-simulation model. A total of 20 processrelated parameters were defined for each land segment, and 6 basin-related parameters were defined for each stream reach.

Description of Study Area

The study area is in the north-central part of Texas (fig. 1). The West Fork Trinity River Basin upstream from Lake Worth drains about 2,100 square miles (mi²). The basin is divided into 28 major subbasins that range from 15.8 to 227 mi² and contains three major reservoirs: Lake Bridgeport, Eagle Mountain Lake, and Lake Worth (fig. 2).

The study area is characterized by a moderate climate with hot, dry summers; warm, wet autumns; cool, dry winters; and warm, wet springs. Mean annual temperature for the study area is 64 °F (degrees Fahrenheit) with monthly means ranging from 44 °F in January to 85 °F in July. Mean annual precipitation, mostly as rainfall, totals about 28 inches (in.) (National Oceanic and Atmospheric Administration, 1990). Rainfall in the study area is generated from frontal systems and convective heating. The frontal systems in spring and fall produce moderate- to high-intensity, long-duration storms that generally result in the maximum storm peaks for the year. Convective thunderstorms produce widely scattered, high-intensity, shortduration storms that occur mostly in summer.

Soils in the basin consist mainly of the Windthorst-Duffau and the Truce-Bonti primary series. The Windthorst-Duffau series consists of deep, sandyloam soils formed in weakly cemented sandstone that has moderately low permeability. A representative soil profile consists of 10 in. of sandy loam overlying 30 in. of sandy-clay loam. The Truce-Bonti series consists of deep, clay-loam soils formed in strongly cemented sandstone and clay that has low permeability. A representative soil profile consists of 9 in. of fine sandy loam overlying 31 in. of clay (U.S. Department of Agriculture, Soil Conservation Service, 1977, 1978, 1981). Land cover is characterized by alternating prairies and woodlands consisting of mesquite, juniper, shinnery oak, post oak, and live oak trees. The rangeland is used for grazing beef and dairy cattle, whereas the cropland is used for growing peanuts and grain sorghum. The average watershed slope varies from gently sloping to sloping, or about 2 to 5 percent.

Description of Simulation Model

The Hydrologic Simulation Program— FORTRAN (HSPF) (U.S. Environmental Protection Agency, 1992) is a continuous-simulation model using a conceptual framework to represent infiltration, evaporation, interception storage, surface runoff, interflow, and base flow on a pervious land segment (PERLND) and retention storage and surface runoff on an impervious land segment (IMPLND). Each user-defined land segment represents its own unique hydrologic response system based on soil type, land cover, watershed slope, or other important basin characteristic. These land segments do not need to be contiguous. The runoff from each land segment is moved through a system of reaches or reservoirs (RCHRES) using storage routing. In addition to runoff, water-quality concentrations for several constituents can be simulated for each land segment.

The HSPF model uses input from three types of data: time series, process-related model parameters, and basin-related model parameters. Continuous time series of precipitation and potential evaporation are needed to execute the model. Point-precipitation data



Figure 1. Location of West Fork Trinity River Basin.



----- Subbasin boundary



measured by raingages are assumed to be uniform over a land segment. Potential evaporation data can be estimated from measured pan evaporation or computed using minimum and maximum temperatures. Measured runoff time series are used for model calibration and testing. The 20 process-related model parameters listed in table 1 (at end of report) represent the physical processes of soil infiltration, soil moisture, evapotranspiration (ET), interception storage of plants, interflow recession, ground-water recession, and surface runoff for each land segment. The process-related model

4 Simulation of Storm Peaks and Storm Volumes for Selected Subbasins in the West Fork Trinity River Basin, Texas, Water Years 1993–94 parameters for each land segment are adjusted to calibrate the model. The following parameters can be varied by month to account for seasonal variations: interception storage capacity (CEPSC), interflow inflow (INTFW), interflow recession rate (IRC), lower zone ET (LZETP), Manning's n for assumed overland flow plane (NSUR), and upper zone nominal storage (UZSN). The HSPF users manual (U.S. Environmental Protection Agency, 1992) provides a more complete description of each parameter.

The six basin-related model parameters listed in table 2 (at end of report) define the areal extent of each land segment, the reach length, and a table of values (FTABLE) of surface area, volume, and discharge as a function of depth for each reach of the watershed. These parameters represent the physical characteristics of each reach of a watershed and generally remain unchanged during calibration of the model.

Only one parameter set was developed for the 5 gaged subbasins to establish a level of confidence in transferring the parameter set to the 23 ungaged subbasins. Accuracy might be improved by calibrating each basin separately; but calibrating each basin separately; but calibrating each basin separately would not provide a method for or confidence in transferring the parameters to the ungaged subbasins. In this study, two gaged subbasins were used for model calibration and the remaining three gaged subbasins were used for model testing. Error and sensitivity analyses were done. The calibration of HSPF was facilitated by the use of an expert system interface developed by Lumb and others (1994) that provided graphics, error statistics, and advice on which parameters to change.

Regional Basin Characteristics

The HSPF simulation run time increases proportionately to the number of land segments used; therefore, the number of land segments used needs to be such that a balance is struck between accuracy and computation time. Geographic information system (GIS) coverages of soil type, land cover, watershed slope, roads, and urban areas were analyzed by the cooperator for the entire basin to identify basin characteristics that could produce unique hydrologic responses (Alice Godbey, Tarrant County Water Control and Improvement District No. 1, written commun., 1993). Two groups of soil types, three groups of land cover, and two groups of watershed slopes were identified to represent 12 unique pervious land segments. The two groups of soil types were classified as low permeability (less than 2 inches per hour [in/hr]) and moderately low permeability (greater than 2 in/hr). Crop, forest, and pasture characterized the three main land-cover groups. The watershed-slope group was divided into flat (slopes less than 3 percent) and steep (slopes greater than 3 percent). Because less than 2 percent of the entire watershed contains roads and urban areas, no impervious land segments were used.

Gaged Subbasins

Because minimal data existed for the 28 subbasins, a network of raingage and streamgage stations was installed to obtain rainfall and runoff data to calibrate and test the continuous-simulation model. Five of the 28 subbasins were selected for data collection on the basis of their representative soil type, land cover, and slope for the entire study area. The drainage areas for the 5 gaged subbasins range from 36.0 to 62.6 mi² (table 3 at end of report).

Each gaged subbasin was subdivided into two to three reaches (figs. 3-7). The basin-related model parameters for each gaged subbasin are listed in table 4 (at end of report). The areas of each pervious land segment were computed for each reach using GIS (Alice Godbey, Tarrant County Water Control and Improvement District No. 1, written commun., 1993). Beans Creek and Big Creek consist mostly of soil group 1, whereas Garrett Creek, Salt Creek, and Walnut Creek consist mostly of soil group 2. The three land-cover groups are fairly well distributed in each of the five subbasins. Most watershed slopes are flat (less than 3 percent) for the five subbasins. The reach length was measured from U.S. Geological Survey (USGS) 7.5 minute quadrangle topographic maps. A channel cross section was measured for each reach to compute the FTABLE values (David Marshall, Tarrant County Water Control and Improvement District No. 1, written commun., 1993). The surface area as a function of depth was computed by multiplying the average channel width by the reach length. The volume as a function of depth was computed by multiplying the average cross-sectional area by the reach length. The discharge as a function of depth was either taken from the rating table at the streamgage station or was estimated using Manning's equation (Chow and others, 1988). The basin-related parameters were not adjusted during the model calibration process.

Flood-stage data were measured at the five subbasins with float, tape, and stilling-well systems that





were installed during February to September 1992. Each streamgage station also included telephone communication equipment for remote data access. The streamgages were classified as partial-record gages and measured only stage above an index elevation. As a result, no base-flow data were recorded. Because the objective of this study was to simulate storm peaks and storm volumes, the assumption was that the partialrecord streamgages would provide adequate data. Discharge rating tables were developed for each streamgage station using a water-surface-profile (WSPRO) model (Shearman, 1990). The theoretical rating tables were refined when a few direct discharge measurements were available. An effort was made to make more direct flood measurements, but because of a small number of large floods, the short duration of runoff, and the occurrence of flood peaks in the late evening and early morning, few flood measurements





Figure 4. Data-collection network for the Big Creek subbasin.



Figure 5. Data-collection network for the Garrett Creek subbasin.

were made during the study period. No flood measurements were available to refine the rating table for Big Creek. Continuous channel modification at Salt Creek during the study period required repeated shifting of the initial rating table. Rainfall was measured with a network of 10 tipping-bucket raingage stations (installed and operated by the cooperator) in the 5 gaged subbasins with 1 raingage for about every 25 mi². The raingage network was supplemented by two to four daily observer stations in each subbasin. Some rainfall data were lost because of instrumentation failure due to lightning strikes during the study period. These data were estimated using the rainfall totals measured at the nearby observer stations. Locations of the raingage and streamgage stations for each subbasin are shown in figures 3–7. Storm-runoff and rainfall data used in this report were collected from October 1, 1992, to September 30, 1994. Daily pan evaporation data measured at Lake Benbrook, about 15 miles (mi) south of Lake Worth, were used as representative data for the study area.

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SIMULATION OF STORM PEAKS AND STORM VOLUMES

To simulate storm peaks and storm volumes, a set of process-related model parameters was determined from calibration of data collected from the Beans Creek



Figure 7. Data-collection network for the Walnut Creek subbasin.

and Walnut Creek subbasins from October 1, 1992, to September 30, 1993 (water year 1993). The parameter set was tested temporally on the Beans Creek and Walnut Creek subbasins using the data from October 1, 1993, to September 30, 1994 (water year 1994), and spatially on the Big Creek, Garrett Creek, and Salt Creek subbasins using data collected during the entire 24-month study period. The temporal and spatial testing provided a level of confidence when transferring the parameters to ungaged subbasins. An error analysis was done to identify sources of error that are not explained by the simulation model. A sensitivity analysis was done to identify which parameters had the greatest effect on simulation results. The parameters

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were revised on the basis of the results of the error and sensitivity analyses.

Model Calibration and Testing

The HSPF model was calibrated using data for the Beans Creek and Walnut Creek subbasins collected from October 1, 1992, to September 30, 1993 (water year 1993). Initial estimates for the 20 process-related parameters were (1) based on the physical properties of soil, land cover, and slope in the subbasins (Chow and others, 1988); (2) assigned the default values listed in table 1; or (3) taken from a previous study (Dinicola, 1990). For each land segment, the default values were used as the estimates for the parameters AGWETP, BASETP, DEEPFR, and KVARY because no data were available to calibrate the base-flow recession parameters. The estimates for the parameters INFEXP and INFILD also remained the default values because there was no justification to change the defaults. The 14 calibrated annual parameters are listed in table 5 (at end of report), the 3 calibrated monthly parameters are listed in table 6 (at end of report), and the 3 initial condition values are listed in table 7 (at end of report). Each parameter varied by soil group, land-cover group, or slope group corresponding to the physical process the parameter represents. For example, the parameters LZSN, INFILT, INTFW, and UZSN varied by soil group and represented the different storage and infiltration capacities of the soil, whereas the parameters CEPSC, LZETP, and NSUR varied by land-cover group and represented the different surface roughnesses, interception storages, and evapotranspiration potentials of the land-cover groups. The parameters LSUR and SLSUR varied by the watershed-slope group. The parameters AGWRC and IRC were assumed to be uniform for all land segments. The parameters INTFW, IRC, and NSUR were not varied monthly because iterative model simulations determined that the parameter values had little or no effect in explaining seasonal variations in runoff for this study area. Values of the annual parameters AGWRC, INFILT, INTFW, IRC, LSUR, LZSN, NSUR, and SLSUR values of the monthly parameters CEPSC, LZETP, and UZSN were adjusted during the calibration process using the software program HSPEXP (Lumb and others, 1994). The values for the initial conditions AGWS, LZS, and UZS were initially estimated from Dinicola (1990) and were revised during calibration. These values were varied by soil group.

A total of 55 storms were recorded in the 5 gaged subbasins from October 1, 1992, to September 30, 1994; the minimum was 6 storms for the Big Creek subbasin, and the maximum was 16 storms for the Walnut Creek subbasin. The calibrated parameter set (tables 5– 6) was developed from 10 storms that occurred during water year 1993 in the Beans Creek and Walnut Creek subbasins. The remaining 45 storms were used to test the parameters temporally and spatially to assess the transferability of the parameter set to ungaged subbasins.

The measured and simulated times of peak, storm peaks, storm volumes, and errors for the calibration and testing simulations are listed in table 8 (at end of report). The simulated times of peak are within 3 hours of the measured times of peak for the 3 storms for the Beans Creek subbasin and the 7 storms for the Walnut Creek subbasin during water year 1993. The errors for storm peaks range from -60.8 to 214 percent, and the errors for storm volumes range from -66.7 to 111 percent for the 10 storms that were used for calibration (table 8). The maximum simulated storm peak and storm volume during water year 1993 for the Beans Creek subbasin are greater than the maximum measured values, whereas the maximum simulated storm peak and storm volume for the Walnut Creek subbasin are less than the maximum measured values. Five of the 10 storm peaks and 6 of the 10 storm volumes have an absolute error of less than 50 percent. The best agreement between the 10 measured and simulated hydrographs used for the model calibration is shown in figure 8 for Walnut Creek at Reno during September 13-14, 1993.

Five storms for the Beans Creek subbasin and 9 storms for the Walnut Creek subbasin during water year 1994 were used to test the parameters temporally. The simulated times of peak are within 3 hours of the measured times of peak for 13 of the 14 storms (table 8). The errors for storm peaks range from -98.6 to 416 percent, and the errors for storm volumes range from -87.8 to 258 percent.

Six storms for the Big Creek, 12 storms for the Garrett Creek, and 13 storms for the Salt Creek subbasins during both water years were used to test the parameters spatially (table 8). The simulated times of peak are within 3 hours of the measured time for 19 of the 31 storms. The errors for storm peaks range from -99.6 to 1,000 percent, and the errors for storm volumes range from -98.7 to 786 percent. For the testing period, 6 of the 8 maximum storm peaks of the water year for each subbasin and 4 of the 8 maximum storm volumes



Figure 8. Measured and simulated hydrographs for Walnut Creek at Reno, Texas, September 13-14, 1993.

of the water year for each subbasin have a simulated value higher than the measured value. Only 10 of the 45 storms used for testing have an absolute error of less than 50 percent for both storm peaks and storm volumes. One of the best agreements between measured and simulated hydrographs used for testing is for Garrett Creek near Paradise during May 11–13, 1994 (fig. 9).

The measured times of peak and the simulated times of peak for the 55 storms are shown in figure 10. The difference between the simulated and measured time of peak is within 3 hours for most storms; there was a tendency for the simulated peak to occur before the measured peak.

The measured storm peaks and the simulated storm peaks for the 55 storms are shown in figure 11. There is a fairly symmetric spread of simulated peaks greater than about 70 cubic feet per second (ft^3/s) about the match line; and there is a cluster of simulated storm peaks less than about 70 ft³/s that underestimates measured storm peaks in the 100- to 2,000-ft³/s range.



Figure 9. Measured and simulated hydrographs for Garrett Creek near Paradise, Texas, May 11-13, 1994.

The measured storm volumes and the simulated storm volumes for the 55 storms are shown in figure 12. Overall, there is a fairly symmetric spread of simulated volumes greater than about 0.02 in. about the match line; and there is a cluster of simulated storm volumes less than 0.02 in. that underestimates measured storm volumes in a range from about 0.04 to 0.30 in.

The mean absolute error and bias for the measured and simulated storm peaks and storm volumes for each water year for the five gaged subbasins are listed in table 9 (at end of report). Mean absolute error is the average of the absolute values of all storm errors; bias is the average of all storm errors. A small bias occurs when positive errors and negative errors are almost equal. For the calibration data from water year 1993, the mean absolute error and bias for the three storm peaks for the Beans Creek subbasin are 48.0 and 6.7 percent and for the seven storm peaks for the Walnut Creek subbasin are 63.2 and 6.9 percent. The mean absolute error and bias for storm volumes are 46.6 and -9.9 percent for the three storm peaks for the Beans Creek subbasin and are 34.4 and -2.2 percent for the seven storm volumes for the Walnut Creek subbasin. For the temporal-testing data from water year 1994, the mean



Figure 10. Measured times of peak and simulated times of peak using calibrated parameters.

absolute error and bias are 153 and 85.2 percent for the Beans Creek subbasin and are 81.0 and -21.9 percent for the Walnut Creek subbasin. The results are similar for storm volumes; the mean absolute error and bias are 132 and 68.6 percent for the Beans Creek subbasin and are 63.7 and -8.0 percent for the Walnut Creek subbasin. For the spatial-testing data from the other three subbasins tested, the mean absolute errors for storm peaks range from 68.3 to 470 percent, and the biases range from -47.2 to 401 percent. The mean absolute errors for storm volumes range from 58.7 to 416 percent, and biases range from -52.4 to 416 percent.

The mean absolute errors in storm volumes are less than the errors in storm peaks for every subbasin except Big Creek (table 9). The errors for testing simulations generally are larger than the errors for calibration simulations. The mean absolute errors also generally increase from water year 1993 to water year 1994 for the five subbasins. The monthly distribution of errors for storm peaks and storm volumes for the 24-month study period is listed in table 10 (at end of report). Forty of the 55 storms occurred in 4 months—February, May, September, and October. The remaining 15 storms are distributed through March, April, June, August, November, and December. No storms occurred in January and July. The distribution of errors was relatively even for simulating storm peaks, but there seemed to be a bias for undersimulating storm volumes.

Error Analysis

The types of error from the model calibration and testing can be classified as measurement errors or systematic errors. Measurement errors are introduced as a result of missing data and inaccurate rating tables of stage and discharge. Rainfall data are missing for several raingage stations during January through



Figure 11. Measured storm peaks and simulated storm peaks using calibrated parameters.

March 1993 and had to be estimated from daily observer data. The two streamgage stations with the highest simulation errors, Big Creek and Salt Creek (table 9), also had the least accurate rating tables which result in inaccurate discharge records. The spatial variability of rainfall might not be represented adequately with the existing raingage network of two to three points in each subbasin.

Systematic errors are associated with the inability of the simulation model to represent the physical processes of runoff. These errors are represented in the model parameters and model equations. The wide variation in measured rainfall compared to measured storm volume indicates the seasonal differences in runoff (fig. 13). A total of 4.20 in. of rainfall produced 1.56 in. of runoff in May on Walnut Creek, whereas 4.84 in. of rainfall produced only 0.36 in. of runoff in September. The model parameters and model equations need to account for these variations. On average, 11 storms were measured among 5 gaged subbasins during the 24month study period, ranging from 6 at Big Creek to 16 at Walnut Creek. Eleven is a very small number of storms when calibrating the model to account for a full range of storm volumes, seasons, and antecedent soilmoisture conditions. Another limitation of this study involved using flood-hydrograph streamgage stations with a continuous-simulation model, which provided no data to calibrate the parameters related to base flow. The pervious land segments used in this model might not represent adequately all the different hydrologic response units of the study area. Also, some uncertainty exists in the values of FTABLES for the reach volume and the corresponding discharge, which seems to be true for the Big Creek and Salt Creek subbasins.

The measurement and systematic errors account for some of the error and bias of the simulated storm peaks and storm volumes. However, from the results listed in tables 8–9 and shown in figures 10–12, the



Figure 12. Measured storm volumes and simulated storm volumes using calibrated parameters.

calibrated parameter set does not adequately simulate the storm peaks and storm volumes. There could be a seasonal bias for simulating storm volumes based on the monthly distribution of simulation errors presented in table 10. Also, the errors seem to increase from water year 1993 to water year 1994 (table 9). Additional information on the sensitivity of each parameter might provide some insight into which parameters might be adjusted to provide better calibration and testing results.

Sensitivity Analysis

A sensitivity analysis was done on selected parameters on the Walnut Creek subbasin to determine what effect a change in a parameter value has on the largest storm peak and the total storm volume for the 24 months. The simulation was done using data from 10 of the 16 storms for Walnut Creek. A maximum of 10 storms can be used for calibration by the expert system of HSPEXP (Lumb and others, 1994). The largest simulated peak discharge is 15,400 ft³/s, and the total simulated storm volume is 4.57 in., using the calibrated model parameters for the entire 24 months. The change in simulated peak discharge from 17,800 ft³/s listed in table 8, using the calibrated model parameters for water year 1993, to 15,400 ft³/s, using the calibrated model parameters for the 24 months, represents the sensitivity of storm peaks to the change in the initial conditions (AGWS, LZS, and UZS values) assumed for calibration (table 7) to the simulated values at the end of water year 1993.

Each parameter was modified to represent a reasonable change. The changes in model results relative to a change in the parameter value are listed in table 11 (at end of report). The parameters INFILT, LZSN, and UZSN have the most effect on both storm



Figure 13. Rainfall and measured storm volume.

peaks and storm volumes combined; the parameters DISCH, LSUR, NSUR, SLSUR, and VOL primarily affect the timing and magnitude of the storm peaks with relatively less effect on the storm volumes. None of these parameters affected the timing of the largest peak, but a few of the smaller storm peaks were affected. Also, the parameters AGWS, LZS, and UZS represent the initial conditions at the start of the simulation and did not have an effect on storm peaks that occurred more than 6 months into a simulation, but did have an effect on the storm peaks and storm volumes that occurred early in a simulation. Changing the 11 parameters noted above produced the largest change in model results.

Model Recalibration and Testing

A new process-related parameter set was developed from the recalibration of the Beans Creek and

Walnut Creek subbasins from October 1, 1992, to September 30, 1994, using information derived from the error analysis and the sensitivity analysis, and was tested on the data from Big Creek, Garrett Creek, and Salt Creek. The model was run for the entire period of record to provide additional storms for calibration. Some values of parameters INFILT, INTFW, LZETP, and LZSN were increased and some values of UZSN were decreased to decrease the magnitude of the storm peaks and storm volumes. Some values of parameters LSUR and NSUR were increased, whereas some values of SLSUR were decreased. The values of the parameter CEPSC were not modified. The values of parameters AGWRC and IRC were decreased to steepen the ground-water and interflow recession rates. The values of parameter BASETP were modified from the default value of zero (table 1) to 0.1 to allow available potential evaporation to be satisfied from base flow.

The parameters KVARY, INFEXP, INFILD, DEEPFR, and AGWETP retained the default values listed in table 1. The recalibrated annual parameters are listed in table 12 (at end of report); the 3 recalibrated monthly parameters are listed in table 13 (at end of report); and the 3 initial condition values are listed in table 14 (at end of report).

The FTABLE volumes from table 4 were increased by 25 percent for the Big Creek, Garrett Creek, and Salt Creek subbasins to provide more storage in the reaches for each subbasin to decrease the magnitude of the storm peaks. The initial values for AGWS, LZS, and UZS for water year 1994 (table 14) were from the simulation output from the end of water year 1993.

Data from the 8 storms for the Beans Creek subbasin and the 16 storms for the Walnut Creek subbasin during water years 1993 and 1994 were used to recalibrate HSPF. The remaining 31 storms for the other 3 subbasins were used to test the recalibrated parameters. The measured and simulated times of peak, storm peaks, storm volumes, and errors using recalibrated parameters for the five subbasins are listed in table 15 (at end of report).

The difference between the measured and simulated times of peak, using the recalibrated parameters, is within 3 hours for 23 of the 24 storms for the Beans Creek and Walnut Creek subbasins (table 15). The errors for the storm peaks range from -99.9 to 312 percent using the recalibrated parameters (table 15), compared to -98.6 to 416 percent using the calibrated parameters (table 8). The errors for the storm volumes using the recalibrated parameters range from -98.8 to 203 percent (table 15), compared to -87.8 to 258 percent using the calibrated parameters (table 8). Although the ranges of errors were decreased using the recalibrated parameters, the number of storms with absolute errors less than 50 percent did not improve. Nine storm peaks and 10 storm volumes of the 24 storms using the recalibrated parameters have errors less than 50 percent (table 15), compared to 9 storm peaks and 9 storm volumes of the 24 storms using the calibrated parameters (table 8). The improvement in the simulated hydrograph for Beans Creek on February 14-16, 1993, using the recalibrated parameters compared to the simulated hydrograph using the calibrated parameters, is shown in figure 14. The error of the storm peak improved from 82.1 percent (table 8) to -18.4 percent (table 15), and the error of the storm

volume improved from 55.0 percent (table 8) to -15.3 percent (table 15).

The recalibrated parameters were tested for 6 storms for Big Creek, 12 storms for Garrett Creek, and 13 storms for Salt Creek during water years 1993 and 1994. Twenty-two of the 31 simulated times of peak are within 3 hours. Most of the nine storms for which the simulated time of peak differs by more than 3 hours from the measured time of peak correspond to the storms for which little or no runoff was simulated. The errors for the storm peaks using the recalibrated parameters range from -99.9 to 582 percent (table 15), compared to -99.6 to 1,000 percent using the calibrated parameters (table 8). The errors for storm volumes using the recalibrated parameters range from -99.6 to 306 percent (table 15), compared to -98.7 to 786 percent using the calibrated parameters (table 8). Six storm peaks and 7 storm volumes for the 31 storms using the recalibrated parameters have absolute errors less than 50 percent (table 15), compared to 10 storm peaks and 11 storm volumes for the 31 storms using the calibrated parameters (table 8).

Seven of the 10 maximum simulated storm peaks and 5 of the 10 maximum storm volumes for each water year for the 5 gaged subbasins exceed the measured values (table 15). These results, using the recalibrated parameters, are the same as the results using the calibrated parameters (table 8).

The measured and simulated hydrographs for Big Creek near Chico during October 19–20, 1993, are shown in figure 15. There is relatively good agreement between simulated and measured storm peaks using the recalibrated parameters, but the storm volume is overestimated.

Measured times of peak and simulated times of peak using the recalibrated parameters for the 55 storms are shown in figure 16. The time of peak either remained unchanged or was delayed 1 hour from the times shown in figure 10 for most of the 55 storms. The storms with the largest differences generally were the storms for which little or no simulated runoff was generated.

Measured storm peaks and simulated storm peaks for the 55 storms using the recalibrated parameters are shown in figure 17. The distribution of data about the match line is very similar to the distribution in figure 11. The same 11 simulated storm peaks of less than about 70 ft^3 /s from figure 11 are less than the corresponding measured storm peaks.



Figure 14. Measured and simulated hydrographs for Beans Creek at Wizard Wells, Texas, February 14-16, 1993.

Measured storm volumes and simulated storm volumes for the 55 storms using the recalibrated parameters are shown in figure 18. The distribution of data about the match line is very similar to the distribution in figure 12. Ten of the 55 simulated storm volumes are less than 0.02 in. and are clustered apart from the rest of the data, which is similar to the simulated storm peaks (fig. 17) where increased infiltration, storage, and evaporation, as represented by the recalibrated parameters, generated little or no simulated runoff for some storms.

The mean absolute errors and biases of storm peaks and storm volumes for each water year using the

recalibrated parameters for the five gaged subbasins are listed in table 16 (at end of report). The mean absolute errors for each subbasin for storm peaks range from 47.1 to 297 percent using the recalibrated parameters compared to 48.0 to 470 percent using the calibrated parameters (table 9). The mean absolute errors for each subbasin for storm volumes range from 27.6 to 193 percent using the recalibrated parameters compared to 34.4 to 416 percent using the calibrated parameters (table 9). The biases for each subbasin for storm peaks range from -85.4 to 202 percent using the recalibrated parameters compared to -47.2 to 401 percent using the calibrated



Figure 15. Measured and simulated hydrographs for Big Creek near Chico, Texas, October 19-20, 1993.

parameters (table 9). The biases for each subbasin for storm volumes range from -75.6 to 114 percent using the recalibrated parameters compared to -52.4 to 416 percent using the calibrated parameters (table 9). The errors for the five gaged subbasins generally decreased using the recalibrated parameters.

The mean absolute errors and biases generally are less for the storm volumes than for the storm peaks using the recalibrated parameters (table 16), which is similar to the results of the initial calibration (table 9). In addition, the mean absolute error and bias using the recalibrated parameters (table 16) increases from water year 1993 to water year 1994 for fewer storm peaks and storm volumes than using the calibrated parameters (table 9).

The monthly distribution of errors for storm peaks and storm volumes for the 55 storms using the recalibrated parameters are listed in table 17 (at end of report). The monthly results are similar to the results using the calibrated parameters (table 10); however, the annual totals are somewhat reversed: the calibrated parameters tend more to undersimulate storm volumes, whereas the recalibrated parameters tend more to undersimulate the storm peaks. December is the only



Figure 16. Measured times of peak and simulated times of peak using recalibrated parameters.

month that indicates a bias to oversimulating storm peaks and storm volumes using the recalibrated parameters. However, more storm data are needed to clearly indicate a monthly bias for simulating storm peaks and storm volumes for the other 11 months.

Transfer of Model Parameters to Ungaged Subbasins

The model produced better results for simulation of the larger storm peaks and storm volumes than for simulation of the smaller storm peaks and storm volumes, especially after an extended period of no runoff. However, only selected data for 55 storms at 5 subbasins during 24 months were used to calibrate and test the HSPF model parameters. A continuous-simulation model does provide a means to account for some variations in runoff generation compared to event-based models. The recalibrated process-related parameter set can be transferred to the other ungaged subbasins with the same range in error expected. The basin-related parameters will need to be computed for each ungaged subbasin.

On the basis of the results of this study, additional collection of storm-runoff data, improvement of discharge rating curves, and identification of additional sources of rainfall data could result in model parameters that account for the wide variations in runoff and reduce the range of expected error in model simulation. A longer period of record would provide a better representation of the combinations of storm size, season, and antecedent moisture conditions. The smaller storm peaks that occur during spring and summer might be the result of more widely scattered rainfall with nonuniform intensity.



Figure 17. Measured storm peaks and simulated storm peaks using recalibrated parameters.

SUMMARY

The purpose of this study was to simulate storm peaks and storm volumes for selected subbasins of the West Fork Trinity River Basin upstream from Lake Worth northwest of Fort Worth, Texas. The simulated flows can be used as input to a channel-routing model that can be used to improve reservoir operation in the basin during floods.

A model parameter set for use with HSPF was developed to simulate storm peaks and storm volumes for ungaged subbasins from calibration and testing of five gaged subbasins. Rainfall and runoff data were collected from October 1, 1992, to September 30, 1994, with a total of 55 storms used in this study. Twelve different pervious land segments were defined for the study based on 2 groups of soil, 3 groups of land cover, and 2 groups of slope. Seventeen parameters were defined for each land segment. The model was calibrated with data from 10 storms and tested temporally and spatially with data from the remaining 45 storms. The mean absolute errors for storm peaks for the five subbasins range from 48.0 to 470 percent and for storm volumes range from 34.4 to 416 percent using the calibrated parameters. A sensitivity analysis was done on selected parameters to determine the effect of a change in a parameter value on time of peak, storm peak, and storm volume for one gaged subbasin.

The results of the parameter sensitivity and error analyses from the initial model calibration were used to recalibrate the parameters. The mean absolute errors for storm peaks for the five subbasins range from 47.1 to 297 percent and for storm volumes range from 27.6 to 193 percent using the recalibrated parameters.

The model produced better results for simulation of the larger storm peaks and storm volumes than for

Figure 18. Measured storm volumes and simulated storm volumes using recalibrated parameters.

simulation of the smaller storm peaks and storm volumes, especially after an extended period of no runoff. The parameters can be transferred to the 23 ungaged subbasins with the same range in error expected. However, additional data collection and model refinement could decrease the model errors. In the study area, a wide variation in runoff was produced from similar magnitudes of measured rainfall. Continuous discharge data, improved stage-discharge rating tables, and more storm data could improve model calibration.

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Table 1. Pre	ocess-related model	parameters for t	the Hydrologic	Simulation Program	m-FORTRAN
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[E1, evapotranspiration	1]
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Parameter	Description ¹	Default	Minimum	Maximum	Units
AGWS	Initial active ground-water storage	None	0	None	Inches
AGWETP	Available ET satisfied by active ground water	0	0	1.0	None
AGWRC	Active ground-water recession rate	None	.001	1.0	Per day
BASETP	Available ET satisfied by base flow	0	0	1.0	None
CEPSC	Interception storage capacity	0	0	1.0	Inches
DEEPFR	Fraction of inflow that enters inactive ground water	0	0	1.0	None
INFEXP	Infiltration equation exponent	2.0	0	10.0	None
INFILD	Ratio of maximum and mean infiltration capacities	2.0	1.0	2.0	None
INFILT	Index to infiltration capacity of soil	None	.0001	100.0	Inches per hour
INTFW	Interflow inflow	None	0	None	None
IRC	Interflow recession rate	None	0	1.0	Per day
KVARY	Nonexponential ground-water recession rate	0	0	None	Per inch
LSUR	Length of assumed overland flow plane	None	1.0	None	Feet
LZETP	Lower zone ET	0	0	1.0	None
LZS	Initial lower zone storage	None	0	None	Inches
LZSN	Lower zone nominal storage	None	10.	100.0	Inches
NSUR	Manning's n for assumed overland flow plane	.1	.001	1.0	None
SLSUR	Slope of assumed overland flow plane	None	.000001	10.0	Feet per foot
UZS	Initial upper zone storage	None	0	None	Inches
UZSN	Upper zone nominal storage	None	.01	10.0	Inches

¹ The users manual for Hydrologic Simulation Program—FORTRAN (U.S. Environmental Protection Agency, 1992) provides a more complete description of each parameter.

Table 2. Basin-related model parameters for the Hydrologic Simulation Program-FORTRAN

[PERLND, pervious land segment; IMPLND, impervious land segment; FTABLE, table of depth, surface area, volume, and discharge for each reach]

Parameter	Description	Units
AREA	Drainage area of each PERLND or IMPLND	Acres
LEN	Reach length	Miles
DEPTH	FTABLE depth	Feet
SAREA	FTABLE surface area	Acres
VOL	FTABLE volume	Acre-feet
DISCH	FTABLE discharge	Cubic feet per second

¹ The users manual for Hydrologic Simulation Program—FORTRAN (U.S. Environmental Protection Agency, 1992) provides a more complete description of each parameter.

24 Simulation of Storm Peaks and Storm Volumes for Selected Subbasins in the West Fork Trinity River Basin, Texas, Water Years 1993–94

U.S. Geological Survey station number	Station name	Drainage area (square miles)
08042900	Beans Creek at Wizard Wells	36.0
08042950	Big Creek near Chico	50.0
08044135	Garrett Creek near Paradise	52.7
08044140	Salt Creek near Paradise	53.0
08044800	Walnut Creek at Reno	62.6

Table 3. Gaged subbasins in the West Fork Trinity River Basin upstream from Lake Worth

Table 4. Basin-related parameters for each gaged subbasin

[Land segments are characterized and designated by a 3-digit descriptor. The first digit designates soil permeability: 1, low permeability (less than 2 inches/hour); 2, moderately low permeability (more than 2 inches/hour; X, all permeabilities (or permeability not characterized). The second digit designates land cover: C, crop; F, forest; P, pasture; X, all land covers (or land cover not characterized). The third digit designates watershed slope: F, flat, less than 3 percent; S, steep, more than 3 percent; X, all slopes (or slope not characterized). For example, a land segment characterized by soils of moderately low permeability, a forest cover, and a steep slope has the descriptor 2FS. A land segment for which only land cover is characterized (as crop) has the descriptor XCX.

AREA, land-segment drainage area; LEN, reach length; mi, miles; FTABLE, table of depth (DEPTH), surface area (SAREA), volume (VOL), and discharge (DISCH) for a reach; ft, feet; acre-ft, acre-feet; ft³/s, cubic feet per second; --, no value]

Parameter	Beans Creek	Big Creek	Garrett Creek	Salt Creek	Walnut Creek
		Rea	ach 1		
AREA (acres)					
ICF	1,307	844	0	104	175
1CS	128	131	0	80	[4]
1FF	2,037	4,622	0	133	469
1FS	381	808	0	129	329
1 PF	4,782	6,736	0	253	586
1PS	899	1,402	0	171	382
2CF	127	130	2,701	2,856	3,106
2CS	9	27	338	1,317	179
2FF	54	1,048	4,409	4,802	3,513
2FS	6	194	765	2,476	403
2PF	302	1,158	5,220	5,273	5,610
2PS	43	340	795	2,403	569
LEN (mi)	7.71	9.44	13.8	11.8	8.55
FTABLE					
DEPTH (ft)	0-20.0	020.0	0-20.0	0-20.0	016.0
SAREA (acres)	0-23.4	057.2	0-45.0	071.2	051.8
VOL (acre-ft)	0-421	0-687	0567	0-1,024	0-498
DISCH (ft ³ /s)	02,400	0-10,600	03,360	0-12,100	07,180
		Rea	ch 2		
AREA (acres)					
ICF	734	699	52	0	148
1CS	76	34	12	0	61
1FF	902	1,976	136	0	779
IFS	169	195	13	0	416

Parameter ¹	Beans Creek	Big Creek	Garrett Creek	Salt Creek	Walnut Creek
	ант. <u>и,</u> от	Reach 2—	-Continued		
1PF	1,900	3,088	77	0	690
1PS	347	201	12	0	341
2CF	288	250	2,189	3,456	2,309
2CS	39	36	334	489	89
2FF	314	568	4,540	3,449	2,514
2FS	76	144	738	898	122
2PF	647	861	6,159	4,671	4,043
2PS	98	209	864	948	190
LEN (mi)	6.74	6.49	13.3	4.4	2.70
FTABLE					
DEPTH (ft)	0-20.0	0-20.0	0–20.0	0-20.0	0-20.0
SAREA (acres)	0-19.2	0-39.3	0-43.5	0-56.5	0-36.0
VOL (acre-ft)	0-331	0-472	0–548	0-979	0-426
DISCH (ft ³ /s)	02,000	0-11,600	0-3,360	0–9,500	0-18,100
		Rea	ich 3		
AREA (acres)					
1CF	568	282	0		0
1CS	222	20	0		0
IFF	1,439	1,229	0		0
IFS	532	225	0		0
1PF	2,192	2,296	0		0
1PS	804	420	0		0
2CF	188	319	956		3,300
2CS	9	22	109		93
2FF	618	477	1,082		3,247
2FS	26	89	224		205
2PF	729	730	1,782		5,746
2PS	33	171	270		306
LEN (mi)	3.88	4.37	2.64		4.19
FTABLE					
DEPTH (ft)	020.0	0-20.0	0-20.0		0-20.0
SAREA (acres)	0–29.9	0-68.9	0-18.2		0-50.8
VOL (acre-ft)	0-543	0847	0-236.8		0-610
DISCH (ft ³ /s)	0-4,760	0-20.000	0-4.410		0-22.500

Table 4. Basin-related parameters for each gaged subbasin-Continued

¹ The users manual for Hydrologic Simulation Program—FORTRAN (U.S. Environmental Protection Agency, 1992) provides a more complete description of each parameter.

[Units liste	ed below pa	rameter name	except for I	parameters v	with no uni	ts: in/hr, ii	nch per hou	ır; ft, feet: i	in., inches	s; ft/ft, foo	ot per foot.			
Land segm 2, moderat crop: F, fo than 3 pero	tents are chi cely low per rest; P, pasti rent; X, all s thas the des	aracterized an meability (mc ure: X, all lan slopes (or slop scriptor 2FS	id designated ore than 2 in d covers (or pe not charad A land segre	d by a 3-dig ches/hour; 7 land cover 1 cterized). Fo nent for whi	it descripto X, all perm not characte or example ch only lan	or. The firs eabilities (erized). Th , a land se d cover is	t digit desi (or permeal he third dig gment char characteri	gnates soil bility not ch it designate acterized b zed (as crop	permeabi haracteriz es watersh y soils of y has the	lity: 1, lo ed). The (bed slope: moderate descripto	w permeab second dig F, flat, less tly low per r XCX.	iility (less it designat s than 3 pe meability,	than 2 inch tes land cov treent: S, st a forest co	es/hour); er: C, eep, more ver, and a
Parameter evapotrans maximum nonexpone overland f Protection	definitions: spiration sat and mean in ential ground low plane; S Agency, 19	AGWETP, a tisfied by base nfiltration cap d-water reces: SLSUR, slope 92) provides	vailable eva Pflow; DEEI pacities; INF sion rate; LS o f assumed a more com	potranspirat PFR, fractio ILT, index t SUR, length overland fl plete descrij	ion satisfie n of inflow o infiltratio of assumed ow plane. 7	d by activ that enter on capacity d overland The users ch parame	e ground w s inactive g y of soil; IN I flow plane manual for tter.]	ater, AGW ground wate UTFW, inte' : LZSN, lo Hydrologio	'RC, activ er; INFEX rflow infl wer zone c Simulat	/e ground (P, infiltra ow; IRC, nominal ion Progr	-water rece ttion equation interflow r storage; NS am—FOR	ssion rate ion expon ecession r SUR, Man TRAN (U	; BASETP ent; INFIL ate; KVAR ate; KVAR nning's n fo .S. Environ	available), ratio of Y, r assumed mental
Land segment	AGWETP	AGWRC ^{2.3} (per day)	BASETP ^I	DEEPFR	INFEXP	INFILD	INFILT ^{2.3} (in/hr)	INTFW ^{2,3}	IRC ^{2.3} (per day)	KVARY ¹ (per inch)	LSUR ^{2.3} (ft)	LZSN ^{2.3} (in.)	NSUR ^{3,4}	SLSUR ^{3.4} (ft/ft)
1CF	0	966.0	0	0	2.0	2.0	0.015	0.4	0.01	0	3,000	1.0	0.20	10.0
ICS	0	966	0	0	2.0	2.0	.015	4	10.	0	2,000	1.0	.20	.05
IFF	0	966.	0	0	2.0	2.0	.015	4	10 [.]	0	3.000	1.0	.35	10 [.]
IFS	0	966.	0	0	2.0	2.0	.015	4	.01	0	2,000	1.0	.35	.05
IPF	0	966.	0	0	2.0	2.0	.015	4	10 ⁻	0	3,000	1.0	.25	10 [.]
IPS	0	966.	0	0	2.0	2.0	.015	4	10.	0	2,000	1.0	.25	.05
2CF	0	966.	0	0	2.0	2.0	.045	×.	.01	0	3,000	3.0	.20	.01
2CS	0	966	0	0	2.0	2.0	.045	<u>%</u>	10 [.]	0	2,000	3.0	.20	.05
2FF	0	966.	0	0	2.0	2.0	.045	<u></u> 8	10.	0	3,000	3.0	.35	10.
2FS	0	966.	0	0	2.0	2.0	.045	ø.	10.	0	2,000	3.0	.35	.05
2PF	0	966.	0	0	2.0	2.0	.045	×.	.01	0	3.000	3.0	.25	10.
2PS	0	966.	0	0	2.0	2.0	.045	<u>%</u>	10.	0	2,000	3.0	.25	.05
¹ Def	àult value u	ised for this p	arameter.	,				- 						

Table 5. Calibrated annual parameters

³ Parameter was revised during calibration. ⁴ Initial estimates for this parameter taken from Chow and others (1988).

² Initial estimates for this parameter taken from Dinicola (1990).

Table 5 27

Table 6. Calibrated monthly parameters

[Units listed below parameter name except for parameters with no units; in., inches.

Land segments are characterized and designated by a 3-digit descriptor. The first digit designates soil permeability: 1, low permeability (less than 2 inches/hour); 2, moderately low permeability (more than 2 inches/hour; X, all permeabilities (or permeability not characterized). The second digit designates land cover: C, crop; F, forest; P, pasture; X, all land covers (or land cover not characterized). The third digit designates watershed slope: F, flat, less than 3 percent; S, steep, more than 3 percent; X, all slopes (or slope not characterized). For example, a land segment characterized by soils of moderately low permeability, a forest cover, and a steep slope has the descriptor 2FS. A land segment for which only land cover is characterized (as crop) has the descriptor XCX.

Parameter definitions: CEPSC, interception storage capacity; LZETP, lower zone evapotranspiration; UZSN, upper zone nominal storage. The users manual for Hydrologic Simulation Program—FORTRAN (U.S. Environmental Protection Agency, 1992) provides a more complete description of each parameter.

Land segment	Jan	Feb	Mar	Apr	Мау	June	July	Aug	Sept	Oct	Νον	Dec
						CEPSC ^{1.2} (in.)						
XCX	0.05	0.05	0.05	0.09	0.14	0.18	0.24	0.28	0.24	0.18	0.12	0.08
XFX	.11	.11	.16	.18	.24	.28	.32	.32	.28	.22	.18	.14
ХРХ	.08	.09	.10	.12	.18	.22	.26	.26	.22	.18	.12	.08
						LZETP ^{1,2}						
XCX	.2	.3	.5	.6	.6	.7	.8	.9	.8	.7	.5	.3
XFX	.3	.4	.6	.7	.7	.8	.9	.9	.9	.7	.6	.4
ХРХ	.2	.3	.5	.6	.6	.7	.8	.9	.8	.6	.5	.3
						UZSN ^{1,2} (in.)						
IXX	.1	.1	.2	.2	.3	.4	.5	.6	.5	.4	.3	.2
2XX	.2	.2	.4	.4	.5	.6	.7	.8	.7	.6	.5	.4

¹ Initial estimates for this parameter taken from Dinicola (1990).

² Parameter was revised during calibration.

Table 7. Initial condition values for model calibration

[Land segments are characterized and designated by a 3-digit descriptor. The first digit designates soil permeability: 1, low permeability (less than 2 inches/hour); 2, moderately low permeability (more than 2 inches/hour; X, all permeabilities (or permeability not characterized). The second digit designates land cover: C, crop; F, forest; P, pasture; X, all land covers (or land cover not characterized). The third digit designates watershed slope: F, flat, less than 3 percent; S, steep, more than 3 percent; X, all slopes (or slope not characterized). For example, a land segment characterized by soils of moderately low permeability, a forest cover, and a steep slope has the descriptor 2FS. A land segment for which only land cover is characterized (as crop) has the descriptor XCX.

AGWS, active ground-water storage; LZS, lower zone storage; UZS, upper zone storage. The users manual for Hydrologic Simulation Program-FORTRAN (U.S. Environmental Protection Agency, 1992) provides a more complete description of each parameter. in., inches]

Ctation	\A/		AGWS ^{1,2}	LZS ^{1,2}	UZS ^{1,2}
Station	water year	Land segment	(in.)	(in.)	(in.)
Beans Creek	1993	1XX	0.01	0.5	0.1
		2XX	.01	.5	.1
	1994	1XX	.2	2.0	.3
		2XX	.2	2.0	.3
Big Creek	1993	1XX	.01	.5	.1
		2XX	.01	.5	.1
	1994	1XX	.2	2.0	.3
		2XX	.2	2.0	.3
Garrett Creek	1993	1XX	.6	3.0	.4
		2XX	.6	3.0	.4
	1994	1XX	.6	3.0	.4
		2XX	.6	3.0	.4
Salt Creek	1993	1XX	.6	3.0	.4
		2XX	.6	3.0	.4
	1994	1XX	.2	2.0	.3
		2XX	.2	2.0	.3
Walnut Creek	1993	IXX	.6	3.0	.4
		2XX	.6	3.0	.4
	1994	IXX	.2	2.0	.3
		2XX	.2	2.0	.3

¹ Initial estimates for this parameter taken from Dinicola (1990).

² Parameter was revised during calibration.

calibrated parameters	lue; M. measured value]
n volumes using	4]; S, simulated vi
peaks and storn	r = 100 * [(S-M)/N
simulated storm	t per second; Erro
Measured and	es; ft ³ /s, cubic fee
Table 8.	[in., inch

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										Ē	ror
Water	Date of	Date of	Deinfall		Measured			Simulated		(per	cent)
year	storm	peak	(in.)	Time of peak	Storm peak (ft ³ /s)	Storm volume (in.)	Time of peak	Storm peak (ft ³ /s)	Storm volume (in.)	Storm peak	Storm volume
					Beans C	reek subbasin					
1993	02/14-16/93	02/15/93	1.86	1300	571	0.262	1200	1,040	0.406	82.1	55.0
1993	02/24-25/93	02/25/93	96.	0800	472	.183	0500	185	.061	-60.8	-66.7
1993	02/28-03/02/93	03/01/63	1.07	1400	378	.200	1200	374	.164	-1.1	-18.0
1994	10/18-20/93	10/19/93	4.06	2100	1,520	.929	1900	4,240	3.18	179	242
1994	12/03-04/93	12/03/93	1.86	1500	364	.179	1200	1,880	.641	416	258
1994	02/21-24/94	02/22/94	1.50	0090	110	.150	0200	70	.065	-36.4	-56.7
1994	05/11-14/94	05/13/94	5.09	0400	4,230	2.76	0200	2,460	2.33	-41.8	-15.6
1994	05/25-26/94	05/25/94	1.89	1200	703	.349	0060	61	.052	-91.3	-85.1
					Big Cr	eek subbasin					
1993	02/14-16/93	02/15/93	1.62	1500	809	.118	1100	1,150	.245	42.2	108
1993	02/28-03/02/93	03/01/63	1.16	1400	1,850	.436	1000	525	.128	-71.6	-70.6
1993	05/08-09/93	05/09/93	1.50	1600	1,560	.230	1200	9	.003	9.66-	-98.7
1993	06/08-09/93	06/08/93	4.85	2400	9,470	2.32	2100	3,810	.807	-59.8	-65.2
1994	10/19-20/93	10/19/93	3.31	0500	1,620	.193	0200	5,830	1.71	260	786
1994	02/21-24/94	02/22/94	1.63	0090	868	.142	0200	666	.207	11.2	45.7
					Garrett	Creek subbasin					
1993	12/13-14/92	12/14/92	2.77	0800	568	.278	0200	2,330	069.	310	148
1993	02-15-16/93	02/15/93	1.35	1600	660	.288	1100	683	.214	3.5	-25.7
1993	02/24-25/93	02/25/93	1.8.1	1200	947	.385	0090	1,060	.194	11.9	-49.6
1993	02/28-03/02-93	03/01/93	.87	2200	181	.131	1300	208	.100	14.9	-23.7

Simulation of Storm Peaks and Storm Volumes for Selected Subbasins in the West Fork Trinity River Basin, Texas, Water Years 1993–94

					Measured			Simulated		Eri (perc	ror cent)
water year	Uate of storm	Date of peak	Kainfall (in.)	Time of peak	Storm peak (ft ³ /s)	Storm volume (in.)	Time of peak	Storm peak (ft ³ /s)	Storm volume (in.)	Storm peak	Storm volume
				Ga	nrrett Creek	subbasin-Cont	inued				
1993	09/13-14/93	09/14/93	4.39	0100	337	0.099	0100	597	0.165	77.2	66.7
1994	10/12-14/93	10/13/93	1.73	0300	284	.057	1400	9	.003	6.76-	-94.7
1994	10/17-21/93	10/19/93	6.13	2400	2,090	1.600	2000	7,030	1.97	237	21.9
1994	12/03-04/93	12/03/93	1.58	1500	437	.112	1400	656	.056	50.1	-50.0
1994	02/2124/94	02/22/94	1.35	0090	186	.051	1300	28	.030	-84.9	-41.2
1994	05/11-13/94	05/12/94	4.09	0400	2,230	1.830	0200	2,890	1.03	29.6	-43.7
1994	09/08-09/94	09/09/94	1.34	1200	498	.085	1100	7	.010	-98.6	-88.2
1994	09/15-16/94	09/16/94	1.59	1400	237	.038	1100	11	.011	-95.4	-71.1
					Salt C	reek subbasin					
1993	10/15-16/92	10/16/92	2.31	0200	287	.116	0200	234	.070	-18.5	-39.7
1993	11/18-19/92	11/19/92	1.48	1100	122	.037	1200	5	900.	-95.9	-83.8
1993	12/13-15/92	12/13/92	3.50	2000	544	.375	2000	2,060	.941	279	151
1993	02/03-04/93	02/03/93	1.13	1800	181	.092	1800	512	.159	183	72.8
1993	02/14-16/93	02/15/93	1.47	1200	547	.307	1400	582	.236	6.4	-23.1
1993	02/24-25/93	02/25/93	2.98	0400	1,300	.438	0400	7,670	1.26	490	188
1993	02/28-03/02/93	03/01/93	06'	1200	261	.125	1200	345	.147	32.2	17.6
1993	04/03-05/93	04/04/93	1.41	0500	261	.138	1600	32	.041	-87.7	-70.3
1993	09/13-14/93	09/14/93	4.60	0500	491	.145	0090	1,760	.326	258	125
1994	10/12-14/93	10/13/93	1.97	0100	238	.021	0060	124	.049	-47.9	133
1994	10/18-19/93	10/19/93	4.28	1800	965	.581	1900	8,130	2.36	742	306
1994	05/11-14/94	05/13/94	5.42	0700	737	.503	0700	8,110	1.63	1,000	224
1994	09/15-16/94	09/16/94	1.99	1300	169	.014	1600	16	.013	-90.5	-7.1

Table 8. Measured and simulated storm peaks and storm volumes using calibrated parameters—Continued

					Ponto o M			L atoli imi O		Er	ror
ator.		Data of	Dainfall		measured			olliulateu		(per	cent)
/ater /ear	storm	peak	(in.)	Time of peak	Storm peak (ft ³ /s)	Storm volume (in.)	Time of peak	Storm peak (ft ³ /s)	Storm volume (in.)	Storm peak	Storm volume
					Walnut (Creek subbasin					
993	10/15-16/92	10/15/92	2.32	2400	764	0.097	2400	530	0.093	-30.6	-4.1
993	12/13-15/92	12/13/92	3.53	2100	3,180	890	2100	1,880	.866	-40.9	-2.7
993	02/03-04/93	02/03/93	1.53	1800	768	.201	1500	2,410	.424	214	111
993	02/14-16/93	02/15/93	1.23	1200	1,080	.257	1200	429	.148	-60.3	-42.4
993	02/24-25/93	02/25/93	2.02	0090	7,420	.750	0500	4,470	.556	-39.8	-25.9
993	02/28-03/02/93	03/01/93	88.	1300	106	.258	1200	388	.121	-56.9	-53.1
993	09/13-14/93	09/14/93	4.84	0500	3,070	.358	0200	3,070	.365	0	2.0
994	10/12-14/93	10/13/93	16.1	0200	211	.068	0700	440	.083	109	22.1
94	10/18-20/93	10/19/93	4.75	1800	7,760	1.41	1800	17,800	2.63	129	86.5
94	02/2124/94	02/23/94	1.46	0400	480	.093	0090	28	.029	-94.2	-68.8
94	04/29-30/94	04/30/94	1.45	0300	397	.066	0200	29	.013	-92.7	-80.3
94	05/11-14/94	05/13/94	4.20	0700	5,980	1.56	0100	7,670	1.24	28.3	-20.5
94	05/25-26/94	05/26/94	1.68	0800	250	.048	0200	228	.074	-8.8	54.2
94	08/20-21/94	08/20/94	1.28	2400	692	.082	2400	6	010	-98.6	-87.8
94	09/08-09/94	09/09/94	2.76	1000	2,950	.304	1100	540	.069	-81.7	-77.3
94	09/12-16/94	09/16/94	1.87	0060	2.240	.254	1300	291	.061	-87.0	-76.0

Simulation of Storm Peaks and Storm Volumes for Selected Subbasins in the West Fork Trinity River Basin, Texas, Water Years 1993–94

Table 8. Measured and simulated storm peaks and storm volumes using calibrated parameters-Continued

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Table 9. Mean absolute errors and biases of storm peaks and storm volumes using calibrated parameters

[Mean absolute error = $100 * [\Sigma(|S-M|)/M]/N$; bias = $100 * [\Sigma(S-M)/M]/N$; S, simulated value; M, measured value; N, number of values]

	Mater.	Storm peal	(S	Storm volun	nes
Station	water	Mean absolute error	Bias	Mean absolute error	Bias
	year	(percent)	(percent)	(percent)	(percent)
Beans Creek	1993	48.0	6.7	46.6	-9.9
	1994	153	85.2	132	68.6
Big Creek	1993	68.3	-47.2	85.5	-31.7
	1994	136	136	416	416
Garrett Creek	1993	83.5	83.5	62.8	23.2
	1994	99.0	-8.6	58.7	-52.4
Salt Creek	1993	161	116	85.6	9.7
	1994	470	401	168	164
Walnut Creek	1993	63.2	6.9	34.4	-2.2
	1994	81.0	-21.9	63.7	-8.0

Table 10. Monthly distribution of errors of storm peaks and storm volumes using calibrated parameters

Manih	Storm-pe	eak errors	Storm-vol	ume errors
MONTR	Positive	Negative	Positive	Negative
January				
February	8	7	6	9
March	3	2	1	4
April		2		2
May	3	4	2	5
June		1		I
July				
August		1		1
September	3	5	2	6
October	6	4	7	3
November		l		1
December	4	1	3	2
Total	27	28	21	34

Table 11. Sensitivity of selected parameters

[Parameter definitions: AGWETP, available evapotranspiration satisfied by active ground water; AGWRC, active groundwater recession rate; BASETP, available evapotranspiration satisfied by base flow; DEEPFR, fraction of inflow that enters inactive ground water; INFEXP, infiltration equation exponent; INFILD, ratio of maximum and mean infiltration capacities; INFILT, index to infiltration capacity of soil; INTFW, interflow inflow; IRC, interflow recession rate; KVARY, nonexponential ground-water recession rate; LSUR, length of assumed overland flow plane; LZSN, lower zone nominal storage; NSUR, Manning's n for assumed overland flow plane; SLSUR, slope of assumed overland flow plane; CEPSC, interception storage capacity; LZETP, lower zone evapotranspiration; UZSN, upper zone nominal storage; DISCH, FTABLE discharge; VOL, FTABLE volume; AGWS, initial active ground-water storage; LZS, initial lower zone storage; UZS, initial upper zone storage; FTABLE, table of depth, surface area, volume, and discharge for each reach. The users manual for Hydrologic Simulation Program—FORTRAN (U.S. Environmental Protection Agency, 1992) provides a more complete description of each parameter.

Parameter adjusted	Initial value	Adjusted value	Change in parameter (percent)	Time of peak	Storm peak (ft ³ /s)	Change in storm peak (percent)	Storm volume (in.)	Change in storm volume (percent)
<u> </u>	· · · · · · · ·	• • • • • • • • • • • • • • • • • • •	Annual process-re	lated paran	neters	1 0		
AGWETP	0.0	0.1		1800	15,800	2.6	4.69	2.6
AGWRC	.996	.906	-9.0	1800	15,500	.7	4.73	3.5
BASETP	.0	.1		1800	15,800	2.6	4.66	2.0
DEEPFR	.0	.1		1800	15,400	.0	4.56	2
INFEXP	2.0	1.5	-25.0	1800	15,900	3.2	4.80	5.0
INFILD	2.0	1.5	-25.0	1800	15,200	-1.3	4.57	.0
INFILT	.015, .045	.022, .068	50.0	1800	13,000	-15.6	3.80	-16.9
INTFW	.4, .8	.6, 1.2	50.0	1800	14,400	-6.5	4.85	6.1
IRC	.01	.1	900.0	1800	15,200	-1.3	4.43	-3.1
KVARY	.0	.2		1800	15,400	.0	4.57	.0
LSUR	2,000, 3,000	3,000, 4,000	50.0, 33.3	1800	13,300	-13.6	4.35	-4.8
LZSN	1.0, 3.0	1.5, 4.5	50.0	1800	13,300	-13.6	3.67	-19.7
NSUR	.20, .35, .25	.30, .45, .35	50.0, 28.6, 40.0	1800	13,000	-15.6	4.33	-5.3
SLSUR	.01, .05	.02, .07	100.0, 40.0	1800	18,300	18.8	4.80	5.0
			Monthly process-re	elated paran	neters			
CEPSC	.0532	.15–.42	200.0-31.2	1800	15,100	-1.9	4.37	-4.4
LZETP	.29	.3-1.0	50.0-11.1	1800	15,000	-2.6	4.68	2.4
UZSN	.18	.2–.9	50.0-2.5	1800	14,100	-8.4	4.03	-11.8
			Basin-related	parameters	i			
DISCH	0-22,500	0-11,250	-50.0	1800	13,400	-13.0	4.50	-1.5
VOL	0-610	0-1220	100.0	1800	12,100	-21.4	4.42	-3.3
			Initial process-re	lated condit	ions			
AGWS	.6	.3	-50.0	1800	15,400	.0	4.57	.0
LZS	3.0	2.0	-33.3	1800	15,400	.0	4.49	-1.8
UZS	.4	.2	-50.0	1800	15,400	.0	4.55	4
None			.0	1800	15,400	.0	4.57	.0

 ft^3/s , cubic feet per second; in., inches; change = 100 * [(A-1)/1]; A = adjusted value; 1 = initial value; --, undefined change]

34 Simulation of Storm Peaks and Storm Volumes for Selected Subbasins in the West Fork Trinity River Basin, Texas, Water Years 1993–94

Land segr 2, modera crop; F, fc than 3 per steep slop	nents are chi itely low peri rest; P, pastu cent; X, all s e has the des	aracterized at meability (m ure; X, all lan slopes (or slo scriptor 2FS.	nd designated ore than 2 inc id covers (or la pe not charact A land segme	by a 3-digit hes/hour; X. and cover no terized). For ent for which	descriptor. , all permes ot character example, a only land	The first of abilities (or ized). The alard segner is of a cover is of cover is of the cover is of a	ligit desig r permeab third digit nent chara haracterize	nates soil _F ility not ch t designate: ccterized by ed (as crop	bermeabill aracterize s watershe v soils of 1 has the 0	ity: 1, lov d). The s ed slope: moderate lescriptoi	v permeabi econd digii F, flat, less Iy low perr XCX.	lity (less t designat than 3 pe neability,	than 2 incl es land co rcent; S, st a forest cc	es/hour); ær: C, eep, more ver, and a
Parametel evapotran maximurr nonexpon overland 1 Protectior	r definitions: ispiration sat and mean in ential ground flow plane; S Agency, 19	AGWETP, a isfied by base infiltration car d-water reces iLSUR, slope 92) provides	tvailable evap e flow; DEEPI pacities; INFI ssion rate; LSU e of assumed (a more comp	otranspiratic FR, fraction LT, index to JR, length o overland flov lete descript	on satisfied of inflow th infiltration f assumed w plane. Th ion of each	by active g hat enters i capacity c overland fl ne users ma n parametel	ground wa inactive gr of soil; IN' low plane; anual for I r.]	ater; AGWJ cound wate TFW, inter LZSN, lov Hydrologic	RC, active r; INFEX flow inflo ver zone r Simulati	e ground- P, infiltra w; IRC, i nominal s on Progra	water recention equation the total of	ssion rate on expon ccession r UR, Man 'RAN (U	; BASETP ent; INFIL ate; KVAF ning's n fo S. Enviror	available O, ratio of Y, r assumed mental
Land segment	AGWETP	AGWRC ^{2.3} (per day)	BASETP ^{1.3}	DEEPFR	INFEXP	INFILD	INFILT ^{2.3} (in/hr)	INTFW ^{2.3}	IRC ^{2.3} (per day)	KVARY ¹ (per inch)	LSUR ^{2.3} (ft)	LZSN ^{2.3} (in.)	NSUR ^{2,3}	SLSUR ^{2,3} (ft/ft)
1CF	0	0.83	0.1	0	2.0	2.0	0.03	0.5	0.001	0	3,500	2.0	0.20	0.01
ICS	0	.83	г.	0	2.0	2.0	.03	S.	001	0	2.500	2.0	.20	.04
IFF	0	.83	Ι.	0	2.0	2.0	.03	.5	100.	0	3,500	2.0	.40	.01
IFS	0	.83	1.	0	2.0	2.0	.03	<i>i</i> v	.001	0	2,500	2.0	.40	.04
IPF	0	.83	Γ.	0	2.0	2.0	.03	s.	100.	0	3,500	2.0	.30	10.
IPS	0	.83	Т.	0	2.0	2.0	.03	.5	100.	0	2,500	2.0	.30	.04
2CF	0	.83		0	2.0	2.0	.08	1.0	.001	0	3,500	3.0	.20	10.
2CS	0	.83	Ι.	0	2.0	2.0	.08	1.0	.001	0	2,500	3.0	.20	.04
2FF	0	.83	Г.	0	2.0	2.0	80.	1.0	100.	0	3,500	3.0	.40	.01
2FS	0	.83	Π.	0	2.0	2.0	.08	1.0	.001	0	2,500	3.0	.40	.04
2PF	0	.83	Γ.	0	2.0	2.0	.08	1.0	100.	0	3,500	3.0	.30	10.
2PS	0	.83	l.	0	2.0	2.0	.08	1.0	.001	0	2,500	3.0	.30	.04
¹ De	fault value u tial estimates	s for this para	oarameter. ameter taken 1	from calibra	tion results									

Table 12. Recalibrated annual parameters

[Units listed below parameter name except for parameters with no units; in/hr, inch per hour; ft, feet; in., inches; ft/ft, foot per foot.

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Table 12

³ Parameter was revised during recalibration.

Table 13. Recalibrated monthly parameters

[Units listed below parameter name except for parameters with no units; in., inches.

Land segments are characterized and designated by a 3-digit descriptor. The first digit designates soil permeability: 1, low permeability (less than 2 inches/hour); 2, moderately low permeability (more than 2 inches/hour; X, all permeabilities (or permeability not characterized). The second digit designates land cover: C, crop; F, forest; P, pasture; X, all land covers (or land cover not characterized). The third digit designates watershed slope: F, flat, less than 3 percent; S, steep, more than 3 percent; X, all slopes (or slope not characterized). For example, a land segment characterized by soils of moderately low permeability, a forest cover, and a steep slope has the descriptor 2FS. A land segment for which only land cover is characterized (as crop) has the descriptor XCX.

Parameter definitions: CEPSC, interception storage capacity; LZETP, lower zone evapotranspiration; UZSN, upper zone nominal storage. The users manual for Hydrologic Simulation Program—FORTRAN (U.S. Environmental Protection Agency, 1992) provides a more complete description of each parameter]

Land segment	Jan	Feb	Mar	Apr	Мау	June	July	Aug	Sept	Oct	Νον	Dec
					<u> </u>	CEPSC ^{1,2} (in.)						
XCX	0.05	0.05	0.05	0.09	0.14	0.18	0.24	0.28	0.24	0.18	0.12	0.08
XFX	.11	.11	.16	.18	.24	.28	.32	.32	.28	.22	.18	.14
ХРХ	.08	.09	.10	.12	.18	.22	.26	.26	.22	.18	.12	.08
						LZETP ^{1,2}						
XCX	.2	.3	.4	.5	.6	.7	.8	.9	.7	.6	.5	.3
XFX	.3	.4	.5	.6	.7	.8	.9	.9	.8	.7	.6	.4
XPX	.2	.3	.4	.5	.6	.7	.8	.9	.7	.7	.5	.3
						UZSN ^{1,2} (in.)						
1XX	.1	.1	.2	.2	.2	.3	.4	.4	.3	.3	.3	.2
2XX	.2	.2	.3	.3	.4	.5	.6	.6	.5	.4	.4	.3

¹ Initial estimates for this parameter taken from calibration results.

² Parameter was revised during recalibration.

Table 14. Initial condition values for model recalibration

[Land segments are characterized and designated by a 3-digit descriptor. The first digit designates soil permeability: 1, low permeability (less than 2 inches/hour); 2, moderately low permeability (more than 2 inches/hour; X, all permeabilities (or permeability not characterized). The second digit designates land cover: C, crop; F, forest; P, pasture; X, all land covers (or land cover not characterized). The third digit designates watershed slope: F, flat, less than 3 percent; S, steep, more than 3 percent; X, all slopes (or slope not characterized). For example, a land segment characterized by soils of moderately low permeability, a forest cover, and a steep slope has the descriptor 2FS. A land segment for which only land cover is characterized (as crop) has the descriptor XCX.

AGWS, active ground-water storage; LZS, lower zone storage; UZS, upper zone storage. The users manual for Hydrologic Simulation Program-FORTRAN (U.S. Environmental Protection Agency, 1992) provides a more complete description of each parameter. in., inches]

Station	Water year	Land segment	AGWS ^{1.2} (in.)	LZS ^{1.2} (in.)	UZS ^{1,2} (in.)
Beans Creek	1993	1XX	0.02	0.5	0.11
		2XX	.02	.6	.12
	1994	1XX	.01	.5	.01
		2XX	.01	.7	.02
Big Creek	1993	1XX	.02	.5	.11
		2XX	.02	.6	.12
	1994	1XX	.01	.3	.01
		2XX	.01	.5	.01
Garrett Creek	1993	1XX	.12	.7	.11
		2XX	.12	1.1	.12
	1994	1XX	.01	.4	.01
		2XX	.02	1.0	.03
Salt Creek	1993	1XX	.02	.7	.05
		2XX	.02	1.7	.08
	1994	1XX	.03	.5	.03
		2XX	.05	1.3	.05
Walnut Creek	1993	1XX	.52	2.7	.21
		2XX	.62	3.1	.32
	1994	1XX	.02	.7	.01
		2XX	.02	1.1	.02

¹ Initial estimates for this parameter taken from calibration results.

² Parameter was revised during recalibration.

Table 15. Measured and simulated storm peaks and storm volumes using recalibrated parameters

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c feet per second; in., inches; Error = 100 * [(S-1
ubic feet per second: in., inches; Error = 100 * [(S-1
s, cubic feet per second; in., inches; Error = 100 * [(S-1
t^{3}/s , cubic feet per second; in., inches; Error = 100 * [(S-1)

								2010		Ē	or
Water	Date of	Date of	Dainfall		measured			Simulated		(per	cent)
year	storm	peak	(in.)	Time of peak	Storm peak (ft ³ /s)	Storm volume (in.)	Time of peak	Storm peak (ft ³ /s)	Storm volume (in.)	Storm peak	Storm volume
					Beans C	reek subbasin					
1993	02/14-16/93	02/15/93	1.86	1300	571	0.262	1300	466	0.222	-18.4	-15.3
1993	02/24-25/93	02/25/93	96.	0800	472	.183	0200	116	.044	-75.4	-76.0
1993	02/28-03/02/93	03/01/93	1.07	1400	378	.200	1200	198	.158	-47.6	-21.0
1994	10/18-20/93	10/19/93	4.06	2100	1,520	.929	1900	3,260	2.00	114	115
1994	12/03-04/93	12/03/93	1.86	1500	364	.179	1200	1,500	.543	312	203
1994	02/21-24/94	02/22/94	1.50	0600	110	.150	0400	46	.041	-58.4	-72.7
1994	05/11-14/94	05/13/94	5.09	0400	4,230	2.76	0200	1,990	1.97	-53.0	-28.6
1994	05/25-26/94	05/25/94	1.89	1200	703	.349	0060	43	.063	-93.8	-81.9
					Big Cr	eek subbasin					
1993	02/14-16/93	02/15/93	1.62	1500	808	.118	1200	264	.075	-67.4	-36.4
1993	02/28-03/02/93	03/01/93	1.16	1400	1,850	.436	1100	171	.074	-90.8	-83.0
1993	05/08-09/93	05/09/93	1.50	1600	1,560	.230	1200	_	100	-99.9	9.66-
1993	06/08-09/93	06/08/93	4.85	2400	9,470	2.32	2300	1,560	.397	-83.5	-82.9
1994	10/19-20/93	10/19/93	3.31	0200	1,620	.193	0500	1,530	.784	-5.6	306
1994	02/21-24/94	02/22/94	1.63	0090	868	.142	0300	31	.030	-96.6	-78.9
					Garrett	Creek subbasin					
1993	12/13-14/92	12/14/92	2.77	0800	568	.278	0090	1,680	.682	196	145
1993	02-15-16/93	02/15/93	1.35	1600	660	.288	1300	716	.321	8.5	11.5
1993	02/24-25/93	02/25/93	1.81	1200	947	.385	0100	1,210	.410	27.8	6.5
1993	02/28-03/02-93	03/01/93	.87	2200	181	.131	1400	273	.264	50.8	102

Simulation of Storm Peaks and Storm Volumes for Selected Subbasins in the West Fork Trinity River Basin, Texas, Water Years 1993–94

Weter	je otoľ		Definition		Measured			Simulated		Eri (perc	or ent)
year	storm	peak	(in.)	Time of peak	Storm peak (ft ³ /s)	Storm volume (in.)	Time of peak	Storm peak (ft³/s)	Storm volume (in.)	Storm peak	Storm volume
				Ű	arrett Creek	subbasin-Con	ıtinued				
1993	09/13-14/93	09/14/93	4.39	0100	337	0.099	0800	839	0.267	149	170
1994	10/12-14/93	10/13/93	1.73	0300	284	.057	0090	-	100	9.66-	-98.2
1994	10/17-21/93	10/19/93	6.13	2400	2,090	1.600	2000	5,250	2.32	151	45.0
1994	12/03-04/93	12/03/93	1.58	1500	437	.112	1500	643	.284	47.1	154
1994	02/21-24/94	02/22/94	1.35	0090	186	.051	1300	65	.067	-64.8	31.4
1994	05/11-13/94	05/12/94	4.09	0400	2,230	1.830	0400	1,710	1.44	-23.3	-21.3
1994	09/08-09/94	09/09/94	1.34	1200	498	.085	1100	Ι	.001	8.00-	-98.8
1994	09/15-16/94	09/16/94	1.59	1400	237	.038	1200	13	.003	-94.6	-92.1
					Salt C	reek subbasin					
1993	10/15-16/92	10/16/92	2.31	0200	287	.116	0090	5	.002	-98.3	-98.3
1993	11/18-19/92	11/19/92	1.48	1100	122	.037	1200	-	100.	1.99.1	-97.3
1993	12/13-15/92	12/13/92	3.50	2000	544	.375	2200	1,090	.844	100	125
1993	02/03-04/93	02/03/93	1.13	1800	181	.092	2000	386	.180	113	95.7
1993	02/14-16/93	02/15/93	1.47	1200	547	.307	1400	740	.352	35.3	14.7
1993	02/24-25/93	02/25/93	2.98	0400	1,300	.438	0200	5,300	1.31	308	661
1993	02/28-03/02/93	03/01/93	<u>.</u> 90	1200	261	.125	1200	473	.369	81.2	195
1993	04/03-05/93	04/04/93	1.41	0500	261	.138	1800	12	.055	-72.7	-60.1
1993	09/13-14/93	09/14/93	4.60	0200	491	.145	0200	1,850	.456	277	214
1994	10/12-14/93	10/13/93	1.97	0100	238	.021	0060	13	.010	-94.7	-52.4
1994	10/18-19/93	10/19/93	4.28	1800	965	.581	2000	4,970	2.22	415	282
1994	05/11-14/94	05/13/94	5.42	0700	737	.503	0800	5,030	1.68	582	234
1994	09/15-16/94	09/16/94	1.99	1300	169	.014	1800	×	.003	-95.2	-78.6

Table 15. Measured and simulated storm peaks and storm volumes using recalibrated parameters—Continued

Motor			Deinfell		Measured			Simulated		(perc	or ent)
year	storm	peak	(in.)	Time of peak	Storm peak (ft ³ /s)	Storm volume (in.)	Time of peak	Storm peak (ft ³ /s)	Storm volume (in.)	Storm peak	Storm volume
					Walnut	Creek subbasin					
1993	10/15-16/92	10/15/92	2.32	2400	764	0.097	2400	478	0.095	-37.4	-2.1
1993	12/13-15/92	12/13/92	3.53	2100	3,180	.890	2200	1,680	.907	-47.2	1.9
1993	02/03-04/93	02/03/93	1.53	1800	768	.201	1600	1,910	.449	149	123
1993	02/14-16/93	02/15/93	1.23	1200	1,080	.257	1400	572	.236	-47.0	-8.2
1993	02/24-25/93	02/25/93	2.02	0090	7,420	.750	0200	4,530	.651	-63.8	-13.2
1993	02/28-03/02/93	03/01/93	88.	1300	106	.258	1100	633	.296	-29.7	14.7
1993	09/1314/93	09/14/93	4.84	0200	3,070	.358	0200	3,570	.464	16.2	29.6
1994	10/12-14/93	10/13/93	16.1	0700	211	.068	0100	8	.007	-96.2	-89.7
1994	10/18-20/93	10/19/93	4.75	1800	7,760	1.41	1800	12,000	2.20	54.6	56.0
1994	02/21-24/94	02/23/94	1.46	0400	480	.093	0090	26	.043	-94.6	-53.8
1994	04/29-30/94	04/30/94	1.45	0300	397	.066	0200	2	100.	-99.5	-98.5
1994	05/11-14/94	05/13/94	4.20	0700	5,980	1.56	0100	5,900	1.33	-1.3	-14.7
1994	05/25-26/94	05/26/94	1.68	0800	250	.048	0100	325	.108	30.0	125
1994	08/20-21/94	08/20/94	1.28	2400	692	.082	2400	-	100.	-99.9	-98.8
1994	09/08-09/94	09/09/94	2.76	1000	2,950	.304	1100	539	.069	-81.7	-77.3
1994	09/15-16/94	09/16/94	1.87	0060	2,240	.254	1300	273	.057	-87.8	-77.6

Table 15. Measured and simulated storm peaks and storm volumes using recalibrated parameters-Continued

40 Simulation of Storm Peaks and Storm Volumes for Selected Subbasins in the West Fork Trinity River Basin, Texas, Water Years 1993–94

Table 16. Mean absolute errors and biases of storm peaks and storm volumes using recalibrated parameters

[Mean absolute error = $100 * [\Sigma(|S-M|)/M]/N$; bias = $100 * [\Sigma(S-M)/M]/N$; S, simulated value; M, measured value; N, number of values]

		Storm peak	(S	Storm volum	nes
Station	Water year	Mean absolute error (percent)	Bias (percent)	Mean absolute error (percent)	Bias (percent)
Beans Creek	1993	47.1	-47 1	37.4	-37.4
	1994	126	44.2	100	27.1
Big Creek	1993	85.4	-85.4	75.6	-75.6
	1994	51.1	-51.1	193	114
Garrett Creek	1993	86.4	86.4	86.9	86.9
	1994	82.9	-26.3	77.4	-11.7
Salt Creek	1993	132	71.6	122	65.4
	1994	297	202	162	96.3
Walnut Creek	1993	55.7	-8.6	27.6	20.9
	1994	71.7	-52.9	77.0	-36.7

Table 17. Monthly distribution of errors of storm peaks and storm volumes using recalibrated parameters

[Error = 100 * [(S-M)/S]; S, sin	ulated value; M, measured v	value;, no storms]
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Month	Storm-pe	eak errors	Storm-vol	ume errors
wonth	Positive	Negative	Positive	Negative
January				
February	6	9	7	8
March	2	3	3	2
April		2		2
May	2	5	2	5
June		1		1
July				
August		I		1
September	3	5	3	5
October	4	6	5	5
November		1		1
December	4	1	5	
Total	21	34	25	30

Table 17 41