

In cooperation with the U.S. Army Corps of Engineers, Fort Worth District; City of Corpus Christi; Guadalupe-Blanco River Authority; San Antonio River Authority; and San Antonio Water System

Simulation of Streamflow and Suspended-Sediment Concentrations and Loads in the Lower Nueces River Watershed, Downstream from Lake Corpus Christi to the Nueces Estuary, South Texas, 1958–2008

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By Darwin J. Ockerman and Franklin T. Heitmuller

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Conversion Factors, Datum, and Water-Quality Abbreviation

Inch/Pound to SI

Multiply	Ву	To obtain
	Length	
inch (in.)	25.4	millimeter (mm)
foot (ft)	0.3048	meter (m)
mile (mi)	1.609	kilometer (km)
	Area	
acre	0.4047	hectare (ha)
square mile (mi ²)	2.590	square kilometer (km²)
	Volume	
ounce, fluid (fl. oz)	0.02957	liter (L)
acre-foot (acre-ft)	0.001233	cubic hectometer (hm³)
	Flow rate	
cubic foot per second (ft ³ /s)	0.02832	cubic meter per second (m³/s)
	Mass	
pound, avoirdupois (lb)	0.4536	kilogram (kg)
ton, short (2,000 lb)	0.9072	megagram (Mg)
	Pressure	
pound per square foot (lb/ft²)	0.04788	kilopascal (kPa)
	Hydraulic gradient	
foot per mile (ft/mi)	0.1894	meter per kilometer (m/km)
	Yield	
pound per acre (lb/acre)	1.121	kilogram per hectare kg/ha)

Temperature in degrees Fahrenheit (°F) may be converted to degrees Celsius (°C) as follows:

°C=(°F-32)/1.8

SI to Inch/Pound

Multiply	Ву	To obtain
	Length	
millimeter (mm)	0.03937	inch (in.)
meter (m)	3.281	foot (ft)
	Volume	
milliliter (mL)	0.03382	ounce, fluid (fl. oz)
	Mass	
kilogram (kg)	2.205	pound avoirdupois (lb)

Datum

Horizontal coordinate information is referenced to the North American Datum of 1983 (NAD 83).

Water-Quality Abbreviation

mg/L, milligrams per liter

Simulation of Streamflow and Suspended-Sediment Concentrations and Loads in the Lower Nueces River Watershed, Downstream from Lake Corpus Christi to the Nueces Estuary, South Texas, 1958–2008

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Abstract

The U.S. Geological Survey (USGS), in cooperation with the U.S. Army Corps of Engineers–Fort Worth District, City of Corpus Christi, Guadalupe-Blanco River Authority, San Antonio River Authority, and San Antonio Water System, developed, calibrated, and tested a Hydrological Simulation Program—FORTRAN (HSPF) watershed model to simulate streamflow and suspended-sediment concentrations and loads during 1958-2008 in the lower Nueces River watershed, downstream from Lake Corpus Christi to the Nueces Estuary in South Texas. Data available to simulate suspended-sediment concentrations and loads consisted of historical sediment data collected during 1942-82 in the study area and suspendedsediment concentration data collected periodically by the USGS during 2006-07 at three USGS streamflow-gaging stations, Nueces River near Mathis, Nueces River at Bluntzer, and Nueces River at Calallen. The Nueces River near Mathis station is downstream from Wesley E. Seale Dam, completed in 1958 to impound Lake Corpus Christi. Suspended-sediment data collected before and after completion of Wesley E. Seale Dam provide insights to the effects of the dam and reservoir on suspended-sediment loads transported by the lower Nueces River from downstream of the dam to the Nueces Estuary. Annual suspended-sediment loads at a site near the Nueces River at Mathis station were considerably lower, for a given annual mean discharge, after the dam was completed than before the dam was completed.

Most of the suspended sediment transported by the Nueces River downstream from Wesley E. Seale Dam occurred during high-flow releases from the dam or during floods. During October 1964–September 1971, about 532,000 tons of suspended sediment were transported by the Nueces River near Mathis. Of this amount, about 473,000 tons, or about 89 percent, were transported by large runoff events (mean streamflow exceeding 1,000 cubic feet per second).

To develop the watershed model to simulate suspendedsediment concentrations and loads in the lower Nueces River watershed during 1958–2008, streamflow simulations were calibrated and tested with available data for 2001–08 from the Nueces River at Bluntzer and Nueces River at Calallen stations. Streamflow data from the Nueces River near Mathis station were used as input to the model at the upstream boundary of the model. Simulated streamflow volumes for the Bluntzer and Calallen stations showed good agreement (within 6 percent) with measured streamflow volumes.

The HSPF model was calibrated to simulate suspended sediment using suspended-sediment data collected at the Mathis, Bluntzer, and Calallen stations during 2006-07. The calibrated watershed model was used to estimate streamflow and suspended-sediment loads for 1958-2008, including loads transported to the Nueces Estuary. During 1958–2008, on average, an estimated 307 tons per day of suspended sediment were delivered to the lower Nueces River; an estimated 297 tons per day were delivered to the estuary. The annual suspended-sediment load was highly variable, depending on the occurrence of storm events and high streamflows. During 1958–2008, the annual total sediment loads to the estuary varied from an estimated 3.8 to 2,490 tons per day. On average, 117 tons per day, or about 38 percent of the estimated annual suspended-sediment contribution, originated from cropland in the study watershed. Releases from Lake Corpus Christi delivered an estimated 98 tons per day of suspended sediment or about 32 percent of the 307 tons per day estimated to have been delivered to the lower Nueces River. Erosion of streamchannel bed and banks accounted for 55 tons per day or about 18 percent of the estimated total suspended-sediment load. All other land categories, except cropland, accounted for an estimated 37 tons per day, or about 12 percent of the total. An estimated 9.6 tons per day of suspended sediment or about 3 percent of the suspended-sediment load delivered to the lower Nueces River were removed by water withdrawals before reaching the Nueces Estuary.

Introduction

The Nueces River extends approximately 315 miles from its headwaters in the southern Edwards Plateau in South Texas to Nueces Bay near Corpus Christi, Tex., and has a drainage

area of approximately 16,700 square miles (fig. 1). The river exits the Edwards Plateau near Uvalde, Tex., and enters the South Texas Plains, also referred to as the South Texas Brush Country (Texas Parks and Wildlife Department, 2007), where the majority of its length and drainage area are located. The Frio River, a major tributary, joins the Nueces River near Three Rivers, Tex. Major impoundments in the Nueces River watershed in the South Texas Plains include Upper Nueces Reservoir, formed in 1948 by Upper Nueces Dam; Choke Canyon Reservoir, formed in 1982 by Choke Canyon Dam (on the Frio River); and Lake Corpus Christi, impounded by Mathis Dam in 1935 (surface area 5,493 acres, storage volume 43,800 acre-feet) (Texas Water Development Board, 2002) and impounded since 1958 by Wesley E. Seale Dam (surface area 19,251 acres, storage volume 257,260 acre-feet) (City of Corpus Christi, 2010). Downstream from Wesley E. Seale Dam (fig. 2), the Nueces River flows about 50 miles through an alluvial valley to the Nueces Bay.

The Nueces Estuary (fig. 2) consists of two areas of nearly equal size, Nueces Bay and the Nueces River delta. Nueces Bay is a shallow, 27-square-mile secondary bay of Corpus Christi Bay. Nueces Bay has a mean depth of about 2.5 feet and a volume of about 40,000 acre-feet. Bottom sediments in Nueces Bay are deposited mostly by the Nueces River (Yeager and others, 2006). The Nueces River delta in southern San Patricio County is a 28-square-mile area of vegetated salt and brackish marshes, land subject to inundation from river or tidal flooding, and open water formed where the Nueces River flows into Nueces Bay (fig. 2). Currently (2010), the Nueces River channel is located along the southern margins of the delta. The distributary network includes a man-made overflow channel in the northwestern part of the delta that connects the Nueces River to Rincon Bayou (U.S. Bureau of Reclamation, 2000; Ockerman, 2001).

In November 2005, during a resource agency meeting in San Antonio, Tex., the U.S. Army Corps of Engineers (USACE) highlighted 12 ecological problems in the Nueces River watershed. One of these was a "loss of sediment loading and nutrient loads to estuaries" (Marcia Hackett, U.S. Army Corps of Engineers-Fort Worth District, written commun., 2005). A number of the other ecological problems were directly related, including a reduction of overbank flows downstream from reservoirs and decrease in freshwater inflows to the Nueces Estuary. The reduction in sediment loads to the Nueces Estuary is the result of sedimentation in large impoundments, notably Lake Corpus Christi (Leibbrand, 1987). Downstream from the reservoirs, ecological problems caused by sedimentation impoundment are expected to include river channel incision (Williams and Wolman, 1984; Salant and others, 2006), channel bed armoring (Williams and Wolman, 1984; Vericat and others, 2006), and deltaic and shoreline erosion (Jaffe and others, 1998; Fan and others, 2006; Yang and others, 2006). Reductions in the extent of marshland and vegetated areas in the Nueces River delta occurred following the initial impoundment of Lake Corpus Christi (Morton and Paine 1984; White and Calnan,

1991). The decreased sediment loads of the Nueces River, combined with relative sea level rise and subsidence are responsible for deltaic erosion and the conversion of wetland habitat to open water and shallow flats (Day and others, 1995; White and others, 2002; Yeager and others, 2006).

The USACE–Fort Worth District began a study in 2002 to identify opportunities for flood-damage reduction, ecosystem restoration, and implementation of multipurpose projects in the Nueces River Basin (U.S. Army Corps of Engineers, 2009). The purpose of the USACE study was to participate with other (Federal and non-Federal) sponsor agencies to identify and conduct detailed studies of water-resource problems in the Nueces River Basin (fig. 1), including documenting existing hydrologic, engineering, and environmental conditions of the study area. One of the specific feasibility investigations outlined in the USACE study was related to defining the existing conditions and opportunities for ecosystem restoration in the Nueces Estuary, namely an investigation of the current conditions of suspended-sediment concentrations and loads delivered by the Nueces River to the estuary. As part of this feasibility study, the U.S. Geological Survey (USGS), in cooperation with the USACE-Fort Worth District, City of Corpus Christi, Guadalupe-Blanco River Authority, San Antonio River Authority, and San Antonio Water System, developed, calibrated, and tested a watershed model of the lower Nueces River watershed to simulate existing hydrologic conditions and suspended-sediment concentrations and loads to the Nueces Estuary.

Purpose and Scope

The purpose of this report is to estimate suspended-sediment concentrations and loads in the lower Nueces River watershed downstream from Lake Corpus Christi to the Nueces Estuary during 1958–2008. To accomplish this, (1) previous suspended-sediment data and studies for the study area were reviewed; (2) historic estimates of suspended-sediment loads were compiled; and (3) a watershed model to simulate streamflow and suspended-sediment concentrations and loads in the lower Nueces River watershed was developed and calibrated. Using the watershed model, estimates of suspended-sediment loads to the Nueces Estuary for 1958–2008 were prepared. Limitations of model-simulated estimates of sediment loads are described.

Description of the Lower Nueces River Watershed

The lower Nueces River study area comprises about 216 square miles of the nontidal part of the Nueces River watershed, from the outlet of Wesley E. Seale Dam near Mathis (fig. 2) to the tidal reach of the river that flows into the Nueces Estuary. The study area encompasses parts of Bee, Jim Wells, Live Oak, Nueces, and San Patricio Counties in South Texas.

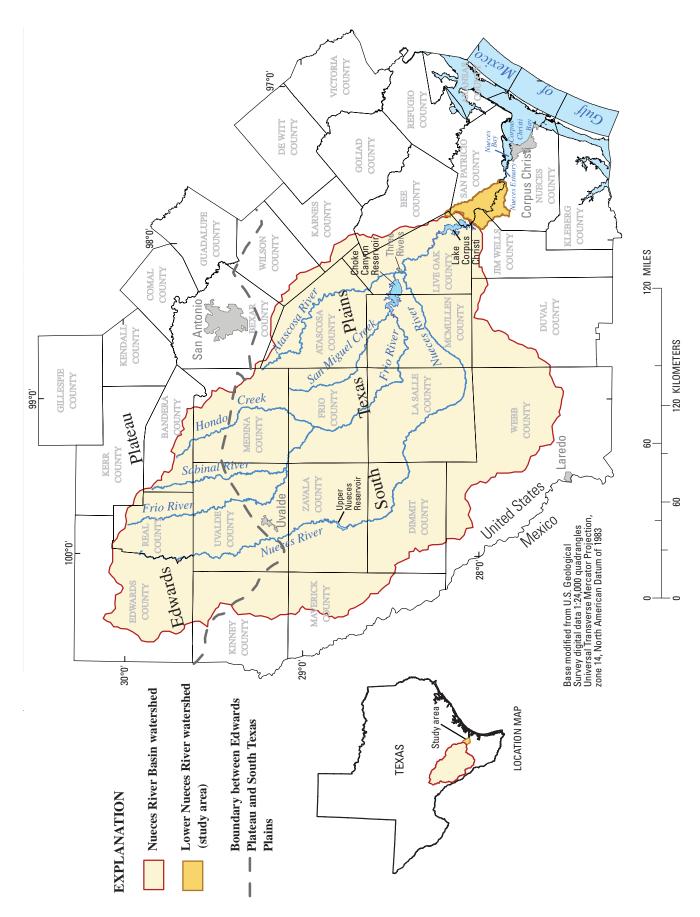


Figure 1. Location of Nueces River Basin, including lower Nueces River study area, South Texas.

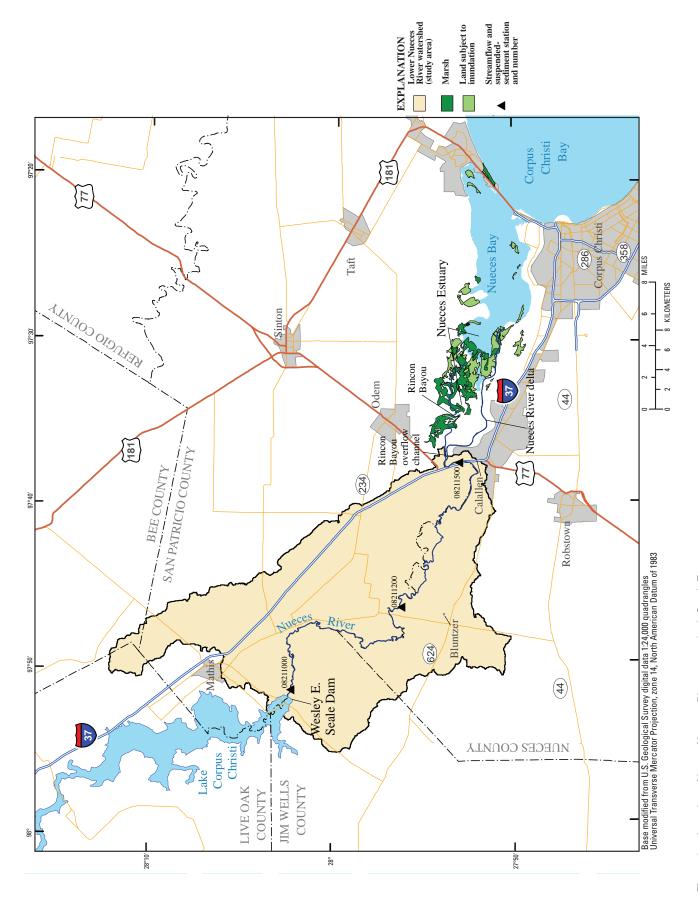


Figure 2. Location of lower Nueces River watershed, South Texas.

The largest town in the study area is Mathis, Tex., which had a population of about 5,000 in 2000 (Handbook of Texas Online, 2009).

The study area has a subtropical, subhumid climate characterized by hot summers and mild, dry winters (Larkin and Bomar, 1983). The average monthly low temperatures range from 46.2 degrees Fahrenheit (°F) in January to 74.5 °F in August; average daily high temperatures range from 66.0 °F in January to 93.4 °F in August (National Climatic Data Center, 2009). The average annual rainfall (1971–2000) in the study area is about 32.6 inches (32.2 inches at Mathis National Weather Service [NWS] station COOP ID 415661 and 32.9 inches at Robstown NWS station COOP ID 417677). Daily rainfall greater than 0.01 inch occurs, on average, 72 days per year (National Climatic Data Center, 2009); daily rainfall equal to or greater than 1.0 inch occurs, on average, every 39 days (Asquith and Roussel, 2003). Although most rainfall occurs in spring, early summer, and fall, amounts greater than about 1.0 inch can occur anytime during the year (Larkin and Bomar, 1983).

Most land use is rangeland and cropland. Elevation in the lower Nueces River watershed ranges from about 3 to 230 feet above sea level (U.S. Geological Survey, 2001). Land slopes are generally low, mostly less than 5 percent. Overall, the stream-channel slope of the lower Nueces River is about 1 foot per mile over the 38 miles from the outlet of Wesley E. Seale Dam to the river crossing at Interstate Highway 37, north of Corpus Christi (fig. 2).

Suspended-Sediment Concentrations and Loads of the Lower Nueces River

Characterizing suspended-sediment concentrations and loads in the lower Nueces River included the following major steps: (1) A review of the available suspended-sediment concentration and load data and studies, including a compilation of historical estimates of suspended-sediment loads; (2) development and calibration of a watershed model to simulate suspended-sediment concentrations and loads in the lower Nueces River watershed; and (3) estimation of suspended-sediment loads to the Nueces Estuary for 1958–2008.

Review of Historical Suspended-Sediment Data and Studies in the Lower Nueces River

Various studies have investigated the problems associated with the reduction in sediment loads to the Nueces Estuary and the associated loss of wetland habitats in the Nueces River delta. Morton and Paine (1984) used aerial photographs to show that the propagation of deltaic wetlands in the Nueces River delta ceased sometime between 1930 and 1959. These findings were substantiated by White and Calnan (1990), who also showed an additional decrease in net vegetated areas by 1979. As further support to the importance of sediment

transport in maintaining the Nueces River delta, Leibbrand (1987) showed that almost all sediment entering Lake Corpus Christi between 1972 and 1985 was trapped. Recent studies showed that loss of wetlands in the Nueces Estuary not only was the result of upstream impoundments but also was influenced by gradual subsidence of the delta (White and others, 2002). As a consequence of these two factors, White and others (2002) also predicted that wetland areas would continue to decrease in the Nueces River delta. Finally, Yeager and others (2006) showed that sediment in Nueces Bay is supplied mostly by the river rather than by marine sources (transport from the Gulf of Mexico by wind or tidal currents), supporting claims that a reduction of suspended-sediment inflow from the Nueces River will contribute to loss of Nueces Estuary wetland habitat.

In 1924, the Texas Board of Water Engineers (whose name and functions were subsequently changed several times by the Texas Legislature and whose functions unrelated to water rights were transferred to the Texas Water Development Board [TWDB] in 1965) initiated a program to evaluate the economic life of reservoirs, which included monitoring suspended-sediment loads in selected Texas rivers (Stout and others, 1961; Mirabal, 1974). During the program, the USGS (in 1942) established one site in the lower Nueces River study area (fig. 2), streamflow-gaging station 08210000 Nueces River near Mathis, Tex. (hereinafter, Mathis gage). Suspended-sediment concentration samples were collected on a daily basis at this site by TWDB and its predecessor agencies; monthly and annual suspended-sediment concentration data and sediment loads (using streamflow data from the Mathis gage) were computed until 1982 and published by various authors, including Stout and others (1961), Adey and Cook (1964), Cook (1967, 1970), Mirabal (1974), Dougherty (1979), and Quincy (1988).

Records for June 1958–June 1961 were invalid (Adey and Cook, 1964) and were not used for interpretation in this report. Suspended-sediment samples were collected during 1942–82 using an 8-ounce (236.3-milliliter) narrow-neck bottle which was held in a 10-pound (4.54-kilogram) torpedo-shaped sampler frame positioned approximately 1 foot (0.3 meter) below the water surface. Samples were collected either daily or throughout the day if the stage changed considerably. Samples were collected at one-sixth, one-half, and five-sixths of the water-surface width using a sampling device called the "Texas sampler" (Stout and others, 1961; Welborn, 1967). To account for increasing suspended-sediment concentrations with depth, the measured percentage of suspended sediment by weight was multiplied by a correction factor of 1.102 to obtain the mean percentage of suspended sediment in the vertical profile (Farris, 1933). Suspended-sediment loads were computed with the assumptions that 1 acre-foot of streamflow weighed 1,361.25 tons and 1 acre-foot of sediment weighed 1,524.60 tons (Stout and others, 1961). Since 1982, most sediment data have been collected as part of relatively short-term studies for selected river reaches (for example, Phillips and others, 2005; Yeager and others, 2005).

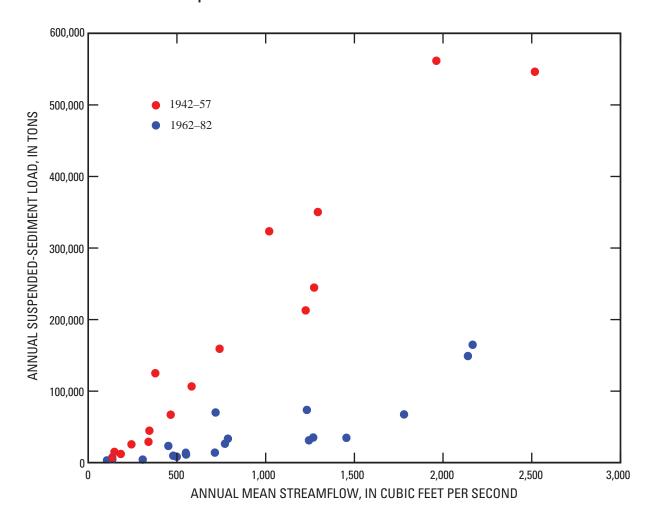


Figure 3. Annual suspended-sediment loads for the Nueces River near Mathis, Texas, 1942-57 and 1962-82.

To verify the accuracy of the suspended-sediment sampling method described in the previous paragraph, referred to hereinafter as the "Texas sampler method," Welborn (1967) compared the results of samples collected from various Texas streams by the Texas sampler method with results of concurrent samples collected by a depth-integrated method (Edwards and Glysson, 1999). Correlations between the two methods for sand-bed rivers in East Texas were poor but were very good (differing by 15 percent or less) for mixed-bed and gravel-bed rivers. The lower Nueces River is a mixed sand- and gravel-bed channel; therefore, suspended-sediment data collected using the Texas sampler method were deemed reasonably accurate for computation of historical loads.

Suspended-sediment data collected before and after completion of Wesley E. Seale Dam provide insights to the effects of the dam and reservoir on suspended-sediment loads transported by the lower Nueces River from downstream of the dam to the Nueces Bay. Suspended-sediment loads (fig. 3; table 1) measured by TWDB and its predecessor agencies at the Mathis gage decreased appreciably in the years after completion of Wesley E. Seale Dam in 1958. During 9 of the

16 years before completion of the dam, annual suspendedsediment loads exceeded 100,000 tons. The maximum annual suspended-sediment load for the period of record was 561,739 tons in 1957, an amount that likely was larger compared with loads for all previous years because of the erosion of sediment during construction of Wesley E. Seale Dam immediately upstream from the suspended-sediment sampling site. In the 21 years following completion of the dam, for which reliable suspended-sediment data were available (1962-82), more than 100,000 tons of suspended sediment were transported in only 2 years—1967 and 1971. These 2 years were characterized by relatively high annual mean streamflows at the Mathis gage— 2,167 and 2,140 cubic feet per second in 1967 and 1971, respectively. During 1962-82, annual suspended-sediment loads were considerably lower, for a given annual mean discharge, than in the years before the dam was completed (fig. 3; table 1). The relation between annual suspended-sediment loads and annual mean streamflow before and after completion of Wesley E. Seale Dam is shown on figure 3. As an indication of the sediment-retention capacity of the dam, for comparable annual mean discharges, annual suspended-sediment loads

Table 1. Annual suspended-sediment loads for the Nueces River near Mathis, Texas, 1942–58 and 1961–82 (from Stout and others, 1961; Adey and Cook, 1964; Cook, 1967, 1970; Mirabal, 1974).

Hydrologic year ^{1,2}	Number of days measured during hydrologic year	Annual suspended- sediment load (tons)	Annual mean suspended- sediment load (tons per day)	Annual mean streamflow (cubic feet per second)
1942	241	546,504	2,268	2,517
1943	365	44,790	123	345
1944	366	323,550	884	1,020
1945	365	125,070	343	378
1946	365	350,430	960	1,294
1947	365	244,730	670	1,273
1948	366	15,170	41	148
1949	365	212,770	583	1,226
1950	365	29,160	80	340
1951	365	106,740	292	583
1952	366	25,670	70	244
1953	365	159,200	436	741
1954	365	67,020	184	465
1955	365	7,269	20	135
1956	366	12,165	33	184
1957	365	561,739	1,539	1,962
1958	243	395,791	1,629	2,179
1961	92	2,088	23	373
1962	365	2,845	8	111
1963	365	2,769	8	110
1964	366	3,445	9	104
1965	365	33,642	92	787
1966	365	23,400	64	452
1967	365	177,600	48	2,167
1968	366	73,640	201	1,232
1969	365	4,810	13	136
1970	365	70,290	193	718
1971	365	149,100	408	2,140
1972	366	67,530	185	1,780
1973	365	14,030	38	714
1974	365	31,370	86	1,244
1975	365	11,450	31	552
1976	366	12,460	34	550
1977	365	34,830	95	1,455
1978	365	4,280	12	307
1979	365	8,210	22	499
1980	366	26,560	73	771
1981	365	35,190	96	1,268
1982	365	9,790	27	479

 $^{^{1}}$ A hydrologic year begins October 1 of previous calendar year and ends September 30 of reported year.

 $^{^{\}rm 2}$ Data for June 1958–June 1961 are invalid (Adey and Cook, 1964) and are not included.

after completion of the dam have been consistently lower compared with annual suspended-sediment loads before completion of the dam (fig. 3).

Most of the suspended sediment transported by the Nueces River downstream from Wesley E. Seale Dam occurred during high-flow releases from the dam or during floods. During October 1964–September 1971, about 532,000 tons of suspended sediment were transported by the Nueces River near Mathis (table 1). Of this amount, about 473,000 tons (table 2), or about 89 percent, were transported by large runoff events (mean streamflow exceeding 1,000 cubic feet per second). Furthermore, the suspended-sediment transport rate increases with higher magnitude flows (fig. 3). The largest flow event listed in table 2 (September 21–October 11, 1967) transported a mean of 8,872 tons of suspended sediment per day.

Recent (2006–07) Suspended-Sediment Sampling

In addition to historical suspended-sediment data collected daily by TWDB and its predecessor agencies at the Mathis gage during 1942–82, the USGS periodically collected suspended-sediment samples during 2006–07 at the Mathis

Table 2. Streamflow and suspended-sediment loads for selected flow releases from Wesley E. Seale Dam on the Nueces River near Mathis, Texas, October 1964—September 1971 (from Cook, 1970; Mirabal, 1974).

Event	Mean stream- flow (cubic feet per second)	Suspended- sediment load (tons)	Mean suspended- sediment load (tons per day)
October 5–16, 1964	7,425	7,506	682
February 24-March 11, 1965	2,727	3,939	246
May 20-June 11, 1965	4,465	15,541	676
May 2–June 10, 1966	2,980	18,653	466
September 21–October 11, 1967	35,129	186,302	8,872
October 16-30, 1967	1,545	4,318	288
January 21-February 11, 1968	5,492	14,802	673
May 8-June 5, 1968	4,970	15,806	545
July 12–16, 1968	2,544	4,909	982
October 19-November 19, 1969	1,968	5,438	170
May 25–June 14, 1970	5,543	50,447	2,402
July 8–18, 1971	9,894	17,480	1,589
August 8–September 6, 1971	11,006	74,537	2,485
September 10-30, 1971	14,432	53,578	2,551

gage, station 08211200 Nueces River at Bluntzer, Tex. (hereinafter, Bluntzer gage), and station 08211500 Nueces River at Calallen, Tex. (hereinafter, Calallen gage) (fig. 2). Whereas the historical suspended-sediment data-collection program typically involved daily sample collection to compute long-term sediment loads, 2006–07 suspended-sediment samples represent data only for selected streamflow and sediment-load conditions. Eight suspended-sediment samples were collected at the Mathis gage, 11 at the Bluntzer gage, and 10 at the Calallen gage.

Using standard USGS protocols and quality-control (QC) procedures (U.S. Geological Survey, 2006), an isokinetic sampler was used to collect suspended-sediment samples in 2006 and 2007. Ockerman and Fernandez (2010, p. 7) noted other investigators' findings that "an isokinetic sampler collects a water-sediment sample from the stream at a rate such that the velocity of the intake nozzle is equal to the incident stream velocity at the nozzle entrance. The water-sediment sample collected is thus representative of the suspended-sediment load throughout the channel cross section and is appropriate for use in estimating sediment load carried by the stream (Davis, 2005)."

Samples were collected at five or more equally spaced intervals across the stream channel and were depth-integrated by lowering and raising the sampler through the water at a constant rate. The samples from each equal-width segment were then combined into a single composite sample for analysis. A composite water-sediment sample is horizontally and vertically averaged throughout the stream cross section and is assumed to represent the average streamflow-weighted suspended-sediment concentration (Edwards and Glysson, 1999; U.S. Geological Survey, 2006).

Suspended-sediment samples were collected by wading when streamflow conditions permitted. During higher flow conditions, when wading was not possible, samples were collected from a bridge or by boat. Suspended-sediment samples collected by the USGS were analyzed by the USGS sediment laboratory in Iowa City, Iowa. Samples were analyzed for suspended-sediment concentration and sand-separation analysis. Sand-separation analysis gives the percentage of sediment, by weight, that is finer and coarser than 0.0625 millimeter. Particle sizes smaller than 0.0625 millimeter are defined as silt and clay; particle sizes 0.0625 millimeter or larger are defined as sand (Guy, 1969).

Each suspended-sediment sample collected by the USGS was associated with a streamflow value. Streamflow data were usually obtained from the stage-discharge rating curve at the streamflow-gaging station where suspended-sediment samples were collected (Rantz and others, 1982). For some high-flow conditions, streamflow measurements were made when the suspended-sediment samples were collected. The availability of streamflow data and suspended-sediment concentration data allowed computation of suspended-sediment discharge, or load, according to equation 1:

$$L_s = Q \times C_s \times k_s, \tag{1}$$

where

- L_s is the instantaneous suspended-sediment load, in tons per day;
- Q is the streamflow, in cubic feet per second;
- C_s is the suspended-sediment concentration, in milligrams per liter; and
- $k_{_{\rm S}}$ is a conversion factor of 0.0027, which results in a sediment load in tons per day, given streamflow in cubic feet per second and suspended-sediment concentration in milligrams per liter (Porterfield, 1972, p. 46–47).

Simulation of Streamflow and Suspended-Sediment Concentrations and Loads of the Lower Nueces River

Although historical suspended-sediment loads during 1942–82 are available for the Nueces River near Mathis (at the Mathis gage), suspended-sediment concentrations and loads downstream from the Mathis gage have not been documented, and suspended-sediment concentrations and loads at the Mathis gage have possibly changed since they were last documented. To better understand suspended-sediment conditions in the lower Nueces River and to estimate the amount of suspended sediment transported to the Nueces Estuary, a watershed model was developed to simulate streamflow and suspended-sediment loads in the lower Nueces River.

Streamflow and suspended-sediment concentrations and loads were simulated with the Hydrological Simulation Program—FORTRAN (HSPF) (Bicknell and others, 2001). HSPF was selected for the study watershed because it is one of the more comprehensive watershed models available, can simulate a variety of stream and watershed conditions with reasonable accuracy, and enables flexibility in adjusting the model to simulate alternative conditions or scenarios (Donigian and others, 1995). To simulate the hydrologic and sediment processes that occur in a watershed, different data sources are used as input to the HSPF model including rainfall data, potential evapotranspiration, and other meteorological parameters; land cover and land use; and soil characteristics. The outputs of an HSPF model are simulated time series of suspended concentrations, sediment loads, or both, as well as streamflow; the time series are for a user-specified interval, or time step. A 1-hour time step was used for this study. HSPF also can simulate other water-quality constituents, including nutrients, metals, and organic compounds. Simulations for this study were limited to streamflow and suspended sediment.

Continuous (hourly) models enable simulation of important watershed processes for a full range of streamflows. Ockerman and Roussel (2009, p. 4) noted other investigators' findings that "the relative importance of various processes and factors influencing water quality can vary considerably with the magnitude of streamflow. For example, processes that

appreciably affect water-quality conditions during low flows might have relatively minor effects on water-quality conditions during high flows. For assessment of peak-streamflow characteristics, continuous simulation models can provide a more realistic evaluation of antecedent soil-moisture conditions than is generally possible with event-based models (Martin and others, 2001, p. 66)."

Functional Description of Hydrological Simulation Program—FORTRAN¹

HSPF, a continuous, semi-lumped parameter model (Singh, 1995), provides continuous water and mass balance by tracking rainfall and water-quality constituents through the conceptual pathways of the hydrologic cycle. In HSPF, a watershed is represented by a group of hydrologically similar areas referred to as hydrologic response units (HRUs) that drain to a stream segment, lake, or reservoir referred to as a RCHRES (ReaCH REServoir). HRUs are areas in a subwatershed that have similar hydrologic and water-quality characteristics that are determined on the basis of land use, surficial geology, soil characteristics, and other factors that are deemed to produce similar hydrologic responses to rainfall and potential evapotranspiration. HRUs are categorized as either pervious or impervious land segments, termed PERLND (PERvious LaND) or IMPLND (IMPervious LaND), respectively. A PERLND is represented conceptually within HSPF by three interconnected water-storage zones—an upper zone, a lower zone, and a groundwater zone. An IMPLND is represented by surface storage, evaporation, and runoff processes. Each RCHRES is associated with a particular subwatershed and receives the runoff, sediment, and chemical loads from the PERLNDs and IMPLNDs in the subwatershed. The hydraulics of a RCHRES are simulated by a storage routing method (Donigian and others, 1995).

HSPF is composed of a series of computational routines that separately simulate processes of the hydrologic cycle. Specifically, HSPF simulates the hydrologic cycle as an interconnected series of storage (and processing) segments with water fluxes (volume per unit area per unit time) and constituent fluxes (mass [weight] per unit area per unit time) moving between the various storages. Figure 4 shows a flowchart of HSPF hydrologic processes for IMPLNDs and PERLNDs. Figure 5 shows a flowchart of HSPF sediment processes for IMPLNDs, PERLNDs, and RCHRESs. The movement of water and suspended sediment from IMPLNDs and PERLNDs and between storage zones is controlled by various process-related parameters. Although some parameters are directly measurable, most are estimated during model calibration (Martin and others, 2001).

The HSPF model of the lower Nueces River watershed was developed by (1) compiling and processing required input data, (2) configuring the model to represent the watershed, and (3) calibrating the model to improve simulation accuracy.

¹ This section modified from Ockerman and Roussel (2009, p. 4–7).

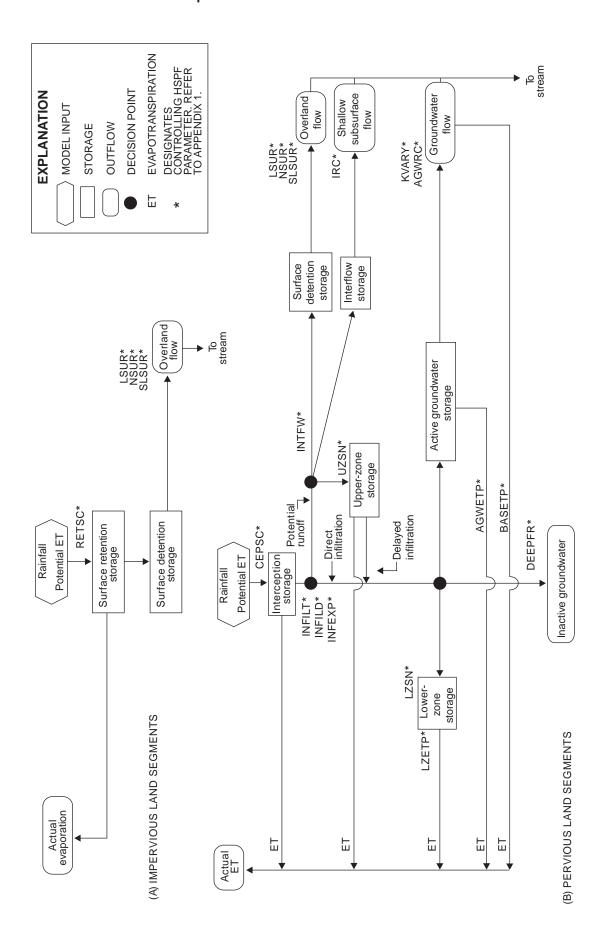


Figure 4. Hydrological Simulation Program—FORTRAN (HSPF) flowchart for hydrologic processes on (A) impervious and (B) pervious land segments (modified from Wicklein and Schiffer, 2002, fig. 3).

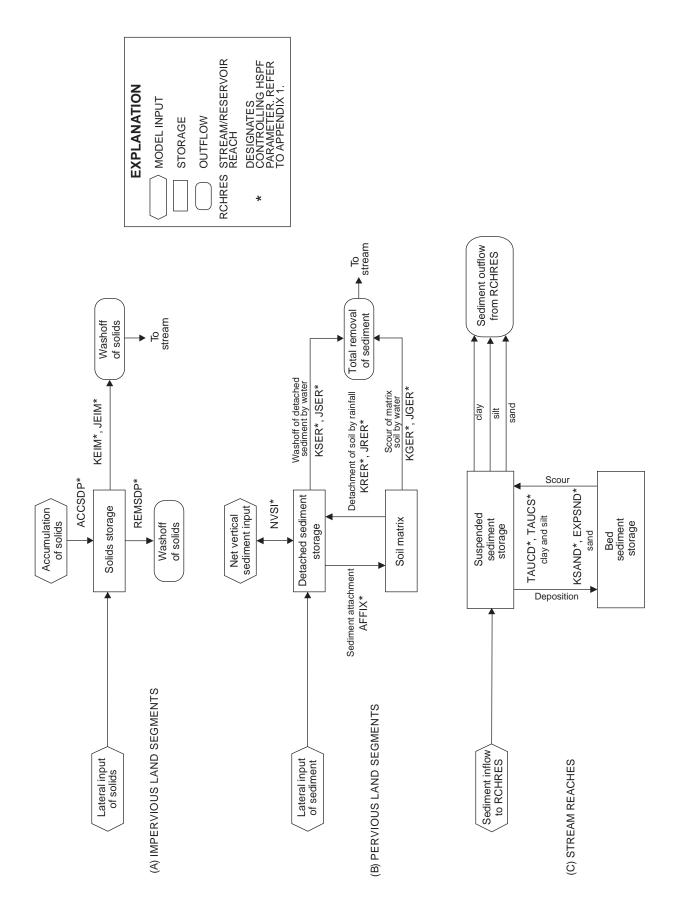


Figure 5. Hydrological Simulation Program—FORTRAN (HSPF) flowchart for sediment processes on (A) impervious land segments, (B) pervious land segments, and (C) stream reaches (modified from Bicknell and others, 2001).

The definitions of the HSPF model process parameters used in the lower Nueces River watershed model are listed in appendix 1. A complete description of the computational processes and required input model parameters is provided in the HSPF users' manual (Bicknell and others, 2001).

Input Data for the Lower Nueces River Watershed Model

Input data for the lower Nueces River watershed model included spatial data (land cover, geology, soils, topography, and drainage characteristics such as subwatershed boundaries and stream-reach length and cross-section data) and times-series data, including meteorological data (rainfall and potential evapotranspiration), streamflow data, and suspended-sediment concentrations. Spatial data were used to create the model HRUs and RCHRESs. Of all the input data, meteorological data had the most pronounced effect on the results of the time-series simulations. Streamflow and suspended-sediment data were used to calibrate the model.

Land-cover data for the study area were obtained from the U.S. Environmental Protection Agency's Multi-Resolution Land Characteristics Consortium (Homer and others, 2004). In the lower Nueces River watershed, there were 15 land covers and land uses as classified by the 2001 National Land Cover Database (NLCD) (Multi-Resolution Land Characteristics Consortium, 2008). To simplify the model configuration, the 15 classes were reclassified into 7 land-cover categories shown on figure 6. Areas classified as open water were modeled as part of a RCHRES. Developed land use was principally classified as low-intensity development, of which about 15 percent was simulated as impervious area. Barren land was grouped with developed open space and the resulting acreage also was considered to be 15-percent impervious.

Surficial geology data (fig. 7) were obtained from areal geologic maps published in the "Geologic Atlas of Texas" (GAT) by the University of Texas Bureau of Economic Geology for the following areas within the lower Nueces River study area: Beeville-Bay City sheet (Aronow and others, 1975) and Corpus Christi sheet (Aronow and Barnes, 1975). The most upstream parts of the study area include the Pleistoceneage Lissie and Deweyville Formations. Most of the rest of the study area is covered by the Beaumont Formation (Pleistocene age) or alluvium (recent age). In the Lissie and Deweyville Formations, the sediments, in order of dominance, consist mostly of sand, silt, and clay. In the Beaumont Formation and alluvium, the sediments, in order of dominance, consist mostly of clay, silt, and sand (Minzenmayer, 1979). The data obtained from the Beeville-Bay City sheet (northern part of the study area) provide a more detailed breakdown of the areas of the Beaumont Formation that are dominated by clay or sand, compared with the data available from the Corpus Christi sheet (southern part of the study area).

Soils data for the study area were compiled from the U.S. Department of Agriculture, Natural Resources Conservation

Service (2009) "Soil Survey Geographic (SSURGO) Database." These data were used to provide initial estimates for selected HSPF process-related parameter values, primarily soil infiltration rate (INFILT). Figure 8 shows the relative soil infiltration rates for the study area.

Topography (slope) data for the study area were obtained from USGS 7.5-minute digital elevation models (U.S. Geological Survey, 2001). The digital elevation models also were used to delineate subwatersheds as part of the HSPF model development. The study area was subdivided into 64 subwatersheds as shown on figure 9. The average size of each subwatershed is 2,000 acres. Also shown on figure 9 is the stream network used in the model configuration. The stream segment (RCHRES) that is associated with each subwatershed is identified with the same identification number as the subwatershed identification number shown in figure 9. Spatial data for streams (location and reach length) were obtained from the "National Hydrography Dataset" (U.S. Geological Survey, 2009). Stream-channel cross-section data were obtained from streamflow discharge measurements made at USGS streamflow-gaging stations in the study area (U.S. Geological Survey, 2010).

Streamflow data from the Mathis gage were used as a boundary condition input to the model. Location information, type of data collected, and period of record for sites that provided streamflow data for the lower Nueces River watershed are listed in table 3. Locations of the streamflow-gaging stations are shown on figure 10.

Water is withdrawn from the Nueces River for municipal and industrial uses in RCHRES 82 and 84 (fig. 9). These withdrawals averaged 64,500 acre-feet per year or the flow volume equivalent to a continuous flow of about 89 cubic feet per second during 2000–2008 (Nueces River Authority, 2009); withdrawal data are available as monthly totals. Within HSPF, the monthly total withdrawals were disaggregated to hourly values and input to the model.

Rainfall and air temperature data were obtained from NWS stations Mathis, Mathis 4 SSW, Robstown, Robstown Airport (rainfall only), Corpus Christi International Airport, and Corpus Christi Maus Field (sites 1-6, respectively, fig. 10; table 3). The NWS has operated rainfall stations in Mathis and Robstown since 1957 and 1947, respectively. The Mathis 4 SSW station replaced the Mathis station in 1977. The Robstown station has been in operation since 1947 and the Robstown Airport station since 2003. The Mathis and Mathis 4 SSW stations (sites 1 and 2, respectively, fig. 10; table 3) and the Robstown and Robstown Airport stations (sites 3 and 4, respectively, fig. 10; table 3) were the primary sources of rainfall data. For modeling purposes, the data from the two Mathis stations were combined and the location of the rainfall data time series was considered to be the same as the newer Mathis 4 SSW station. Similarly, data from the two Robstown stations were combined into a single time series that was considered to be located at the newer Robstown Airport station. These time series and associated locations were then used for the entire simulation period. The spatial

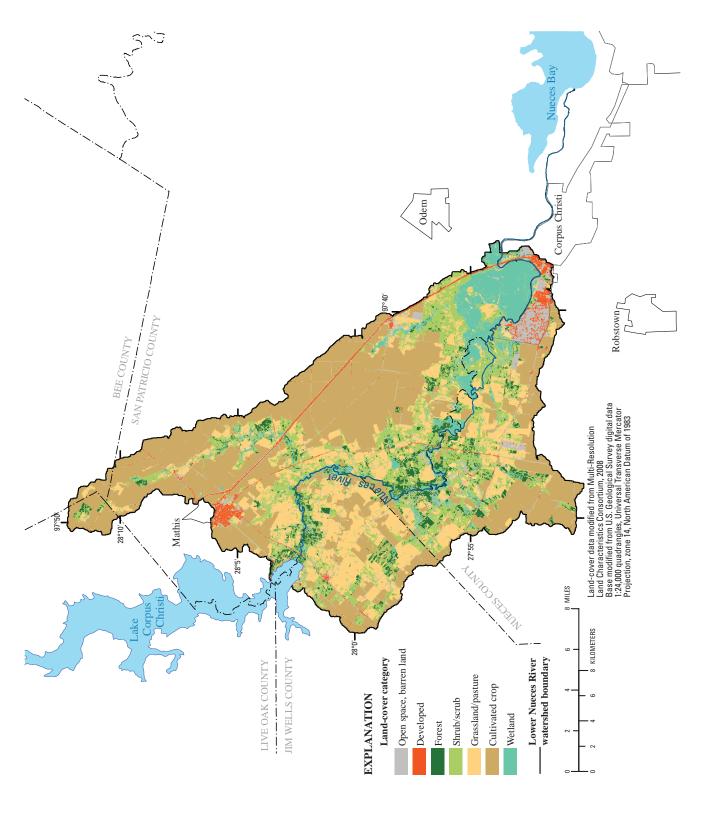


Figure 6. Land-cover categories in the lower Nueces River watershed, South Texas.

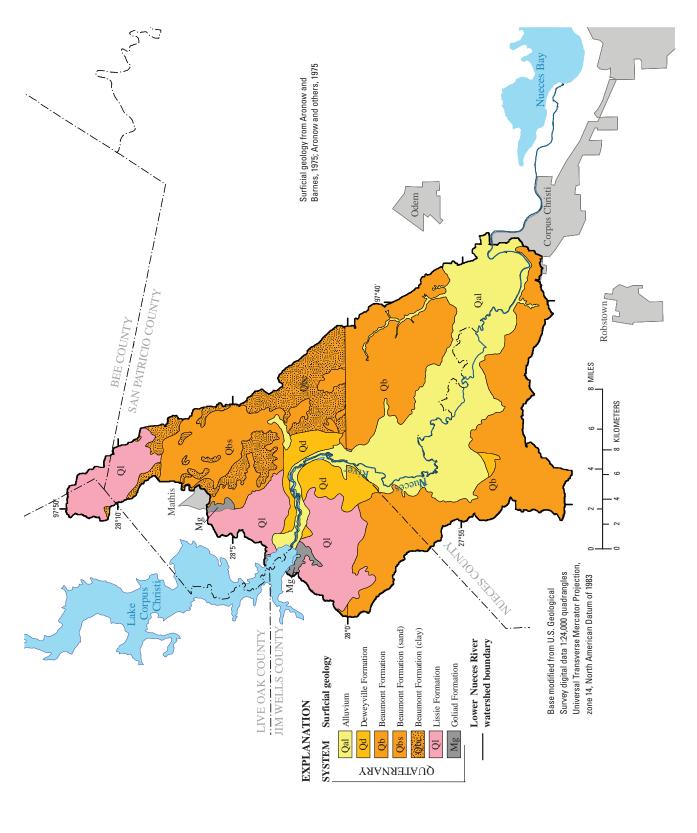


Figure 7. Surficial geology of the lower Nueces River watershed, South Texas.

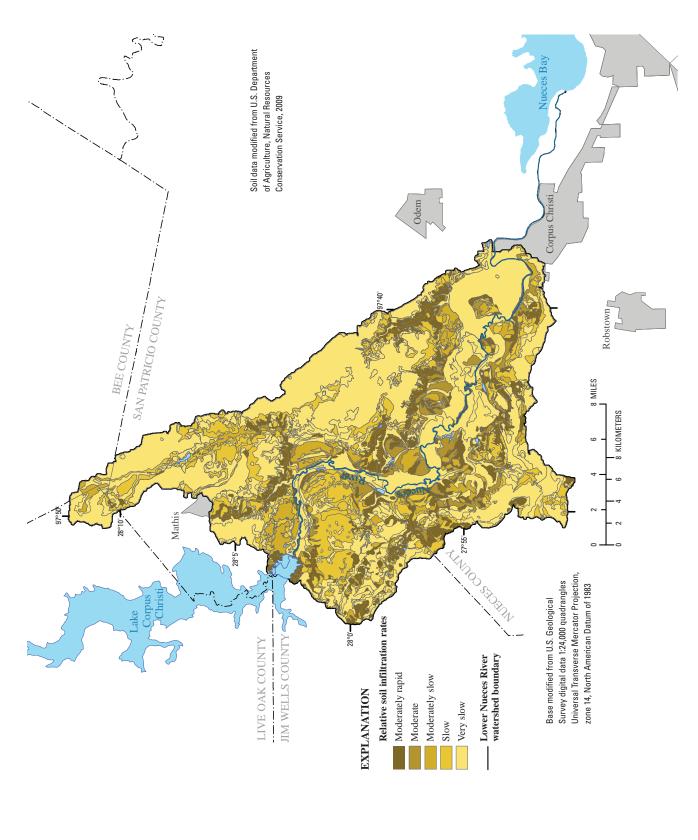


Figure 8. Relative soil infiltration rates in the lower Nueces River watershed, South Texas.

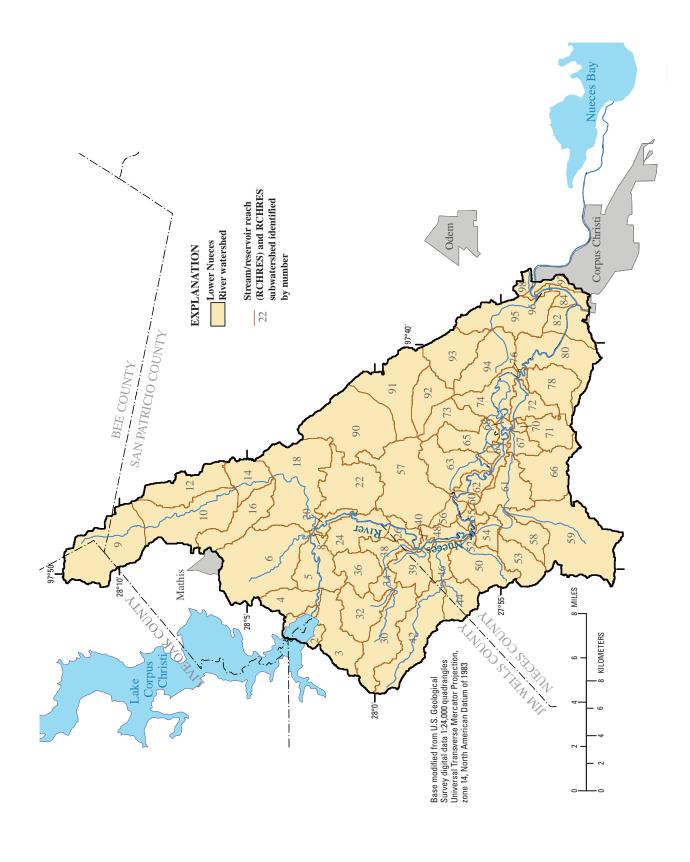


Figure 9. Subwatershed delineation for the lower Nueces River watershed model, South Texas.

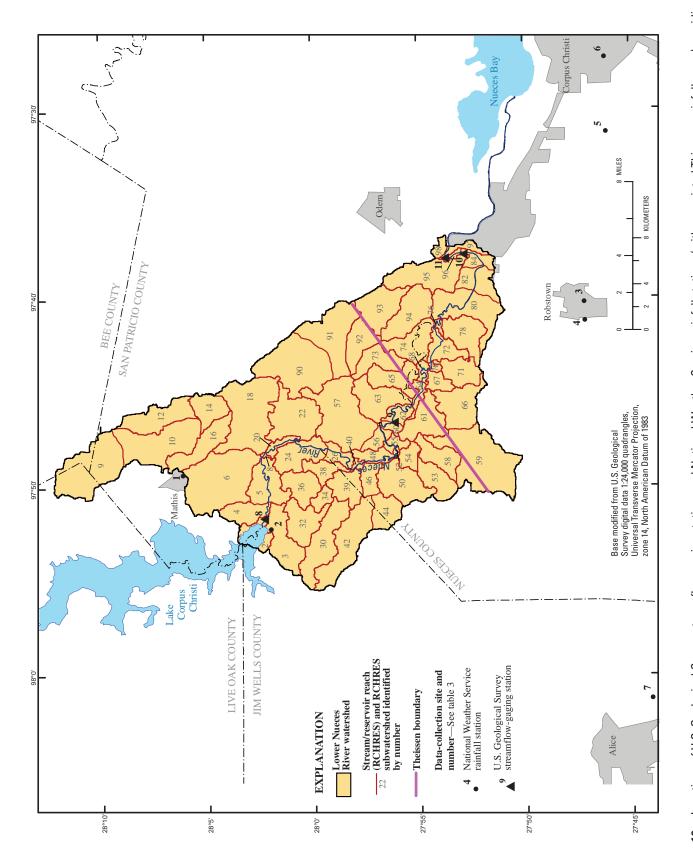


Figure 10. Locations of U.S. Geological Survey streamflow-gaging stations and National Weather Service rainfall stations (with associated Thiessen rainfall areas) providing data for the lower Nueces River watershed model, South Texas.

Table 3. Data-collection sites providing data for the lower Nueces River watershed model, South Texas.

[ddmmss, degrees minutes seconds; NWS, National Weather Service; ID, identifier; --, not available; max, maximum; min, minimum; temp, temperature; USGS, U.S. Geological Survey]

Site number (fig. 10)	Station number and name	Latitude (ddmmss)	Longitude (ddmmss)	Type of data (period of record available)
1	NWS station Coop ID 415661, Mathis, Jim Wells County, Tex.	28°06'"	97°49'"	Daily rainfall; daily max and min air temp (1957–77)
2	NWS station Coop ID 415661, Mathis 4 SSW, Jim Wells County, Tex.	28°02'14"	97°52'21"	Daily rainfall; daily max and min air temp (1977–2008)
3	NWS station Coop ID 417677, Robstown, Nueces County, Tex.	27°47'22"	97°39'43"	Daily rainfall; daily max and min air temp (1947–2008)
4	NWS station WBAN ID 12984, Robstown Airport, Nucces County, Tex.	27°46'43"	97°41'26"	Hourly rainfall (2003–08)
5	NWS station Coop ID 412015, Corpus Christi International Airport, Nueces County, Tex.	27°46'27"	97°30'44"	Daily rainfall; daily max and min air temp (1960–2008)
6	NWS station Coop ID 412015, Corpus Christi Maus Field, Nueces County, Tex.	27°46'"	97°27'"	Daily rainfall; daily max and min air temp (1934–60)
7	NWS station Coop ID 410144, Alice, Jim Wells County, Tex.	27°43'42"	98°04'04"	Daily max and min air temp (1942–2008)
8	USGS streamflow-gaging station 08211000 Nueces River near Mathis, Tex.	28°02'17"	97°51'36"	Streamflow (1952–2008); suspended sediment (1942–82, 2006–08)
9	USGS streamflow-gaging station 08211200 Nueces River at Bluntzer, Tex. (partial-record station)	27°56'15"	97°46'32"	Streamflow (2005–08); suspended sediment (2006–08)
10	USGS streamflow-gaging station 08211500 Nueces River at Calallen, Tex.	27°52'58"	97°37'30"	Streamflow (2001–08); suspended sediment (2006–08)
11	USGS streamflow-gaging station 08211502 Nueces River near Odem, Tex. (partial-record station)	27°53'42"	97°37'43"	Streamflow (flood discharge measurements, 2001–08)

application of rainfall time-series data to the model is based on a Theissen polygon distribution (Linsley and others, 1982). Two rainfall areas were defined by the Theissen analysis as shown on figure 10. Rainfall to the upstream area of the watershed was simulated using the Mathis station data, and rainfall to the downstream area of the watershed was simulated using the Robstown station data. Rainfall data from the Corpus Christi International Airport, Corpus Christi Maus Field, and Alice stations (sites 5, 6, and 7, respectively, fig. 10; table 3) were used to fill periods of missing data when data from the Mathis or Robstown stations were not available.

HSPF uses time series of potential evapotranspiration (PEVT) data to set the upper limit of actual evapotranspiration (ET) that can be simulated for any of the HRUs. PEVT is the observed ET if there is an unlimited supply of water to satisfy the potential ET rate. PEVT was computed from maximum and minimum daily air temperature (from NWS station data) using the Hamon method (Bidlake, 2002). Similar to the sources of rainfall data, the primary sources of air temperature data were the Mathis and Robstown NWS stations. PEVT time-series data were applied to delineated subwatersheds according to the Theissen boundary shown on figure 10.

Model Development

To develop the model, the stream network of the lower Nueces River watershed was segmented into subwatersheds (fig. 9), generally on the basis of (1) similar streamflow travel times that approximated the model time step (1 hour); (2) homogeneous channel properties such as slope and conveyance; and (3) outlets of subwatersheds at points of interest such as streamflow-gaging stations, major tributary confluences, and points of water withdrawals.

In each subwatershed, unique pervious and impervious HRUs were defined according to three factors: (1) land cover and land use; (2) surficial geology and soil characteristics; and (3) the location of the nearest rainfall station (to spatially distribute meteorological input data [rainfall and PEVT]). Spatial information describing these three factors were compiled and analyzed using the Geographic Information System software ArcGIS (ESRI, 2009) to compute the acreage of each HRU within a given subwatershed.

Model Calibration

A primary goal of model calibration is to adjust the process-related parameter values to minimize the differences

between measured and simulated flows at a streamflow-gaging station. The model was calibrated in accordance with guidelines by Donigian and others (1984) and Lumb and others (1994). The calibration of the model proceeded in two steps. First, parameters related to hydrologic processes were calibrated. Second, parameters related to suspended-sediment processes were calibrated. Calibration of hydrologic processes included adjusting appropriate model parameters to minimize differences between measured and simulated streamflow at streamflow-gaging stations during 2001–08 over a wide range of hydrologic conditions. Model parameters that control land-surface erosion and washoff processes and instream sediment transport processes were adjusted to minimize differences between measured and simulated suspended-sediment concentrations and loads.

Hydrology

To evaluate the goodness-of-fit between measured and simulated streamflows, criteria such as error in total streamflow volume for the calibration period and low-flow and high-flow distribution were used. Simulation errors were evaluated by comparing total streamflow volume, 50-percent lowest daily flows, and 10-percent highest daily flows. Donigian and others (1984) presented general guidelines for characterizing the goodness-of-fit of HSPF calibrations. For annual and monthly streamflow volumes, model calibration is considered very good when the error is less than 10 percent, good when the error is 10–15 percent, and fair when the error is 15–25 percent.

Additionally, model-fit statistics generated by the software program GenScn (GENeration and analysis of model simulation SCeNarios for watersheds) (Kittle and others, 1998) were used to examine the quality of the model fit on an annual, monthly, daily, and hourly basis for the (1) coefficient of determination (R-squared [R²]) of the linear regression between measured and simulated streamflow (Ott and Longnecker, 2001); (2) Nash-Sutcliffe coefficient of modelfit efficiency (NSE) (Nash and Sutcliffe, 1970); (3) mean absolute error (MAE) (StatSoft, Inc., 2010); and (4) root mean square error (RMSE) (StatSoft, Inc., 2010). The R² and NSE are similar because each provides a measure of the variation in the measured values that is explained by the simulated values. The NSE, however, provides a generally preferable evaluation of the fit quality compared with the R² because the NSE measures the magnitude of the differences between measured and simulated values, whereas the R² measures the difference between mean values (Zarriello and Ries, 2000, p. 44). MAE and RMSE express the difference between measured and simulated streamflow in original units (cubic feet per second) (StatSoft, Inc., 2010).

At selected calibration sites, depending on the availability of streamflow data, the calibration process included a separate, post-calibration test of the model fit. For sites where the testing process was performed, some of the observed streamflow-gaging data were withheld from the

calibration. After calibration, the withheld data were then used for testing the model fit between measured and simulated streamflow.

Most of the streamflow in the study area originated as releases from Lake Corpus Christi (fig. 1). The Mathis gage is located immediately downstream from the lake and monitors stream discharge from the lake. Daily discharges for this station were available beginning in 1939 and were input to the model at the inlet of RCHRES 4 (fig. 9) as a boundary condition. Because actual gaged streamflow data were used as input to the model at this site, no model calibration was necessary at this station.

The Bluntzer gage (outlet of RCHRES 60, fig. 9) is operated as a partial-record station. Streamflows of more than 2,750 cubic feet per second are not measured because streamflow-gaging conditions are not favorable at higher flows. Because streamflow data from the Bluntzer gage were not suitable for calibration of the entire range of streamflow conditions, calibration of hydrologic parameters upstream from the Bluntzer gage was not based on data from this gage. During 2005–06, streamflow at the Bluntzer gage did not exceed 2,750 cubic feet per second and the 2005-06 data were used as a test of the calibrated model. Streamflow testing results for the Bluntzer gage during 2005–06 (fig. 11; table 4) indicate very good agreement between measured and simulated streamflow volumes; the error in total simulated streamflow volume compared with measured streamflow volume during 2005–06 at this gage was less than 2 percent.

The Calallen gage was operated as a partial-record station during 1989-2000; daily streamflows were not reported when instantaneous streamflow exceeded 2,750 cubic feet per second because of difficulty measuring higher streamflows at this gage. A complete record of streamflow for the Calallen gage is available for 2001-08. In June 2000, the Calallen gage was moved about 0.4 mile downstream to its present location and converted to a continuous streamflow-gaging station capable of measuring the full range of streamflow. Streamflow data measured during 2005–08 at the Calallen gage were used to calibrate the model and streamflow data from 2001-04 were used to test the calibrated model. Streamflow calibration and testing results for the Calallen gage (fig. 12; table 4) indicate very good agreement between measured and simulated streamflow volumes. The error in total simulated streamflow volume at the Calallen gage compared with measured streamflow volume during the calibration period (2005–08) is less than 3 percent; for the testing period (2001–04), the error in total simulated streamflow volume compared with measured streamflow volume is less than 6 percent.

Using evaluation criteria by Donigian and others (1984), calibration and testing results for streamflow volumes at both the Bluntzer (testing only) and Calallen gages (table 4) were considered very good. The R² and NSE values were considered acceptable for annual, monthly, daily, and hourly statistics. The NSE for hourly streamflows ranged from .90 to .96 for the calibration and testing periods at the Bluntzer and Calallen gages. The minimum NSE values for daily, monthly,

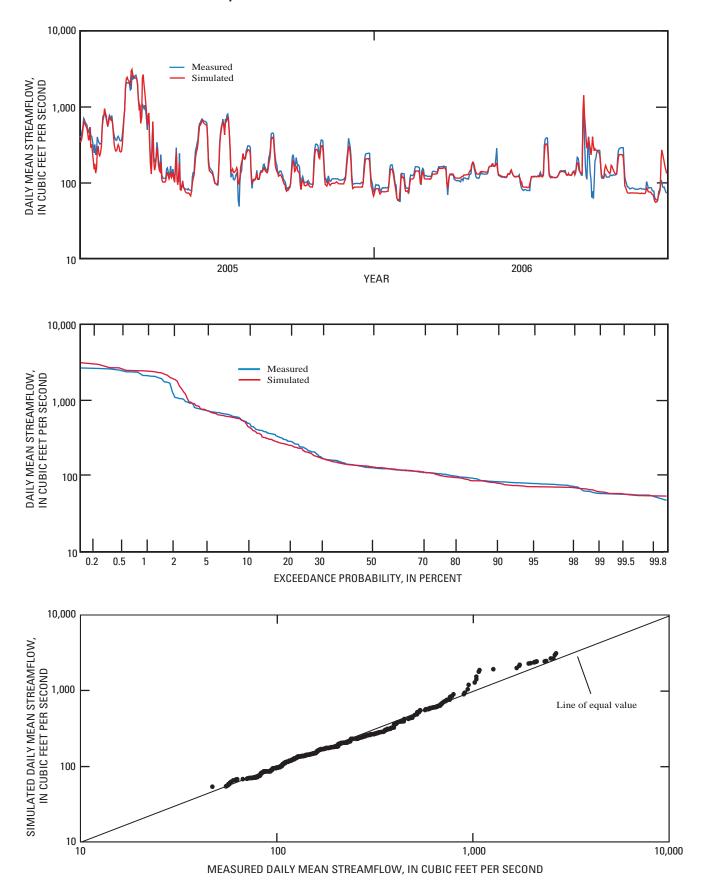


Figure 11. Measured and simulated daily mean streamflow at U.S. Geological Survey streamflow-gaging station 08211200 Nueces River at Bluntzer, Texas, 2005–06.

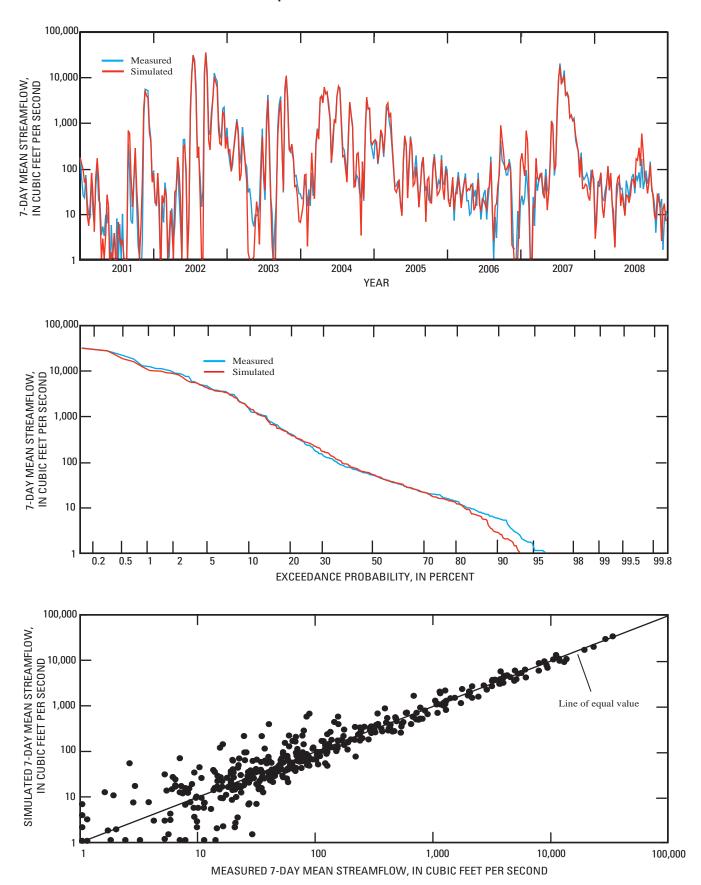


Figure 12. Measured and simulated 7-day mean streamflow at U.S. Geological Survey streamflow-gaging station 08211500 Nueces River at Calallen, Texas, 2001–08.

[acre-ft, acre-feet; ft3s, cubic feet per second]

08211200 Nueces River at Bluntzer, Texas

Testing period 2005–06

Streamflow volumes and peaks	Measured	Simulated	Error (percent)¹	Criteria (percent) ²
Total flow volume (acre-ft)	353,000	357,000	1.1	10
Mean flow rate (ft ³ /s)	244	247	1.1	10
Highest 10-percent daily flows (acre-ft)	151,000	166,000	9.9	10
Lowest 50-percent daily flows (acre-ft)	72,800	71,600	-1.6	10
Model-fit statistics 2005–06	Annual	Monthly	Daily	Hourly
Number of years, months, days, or hours	2	24	730	17,520
Coefficient of determination (R ²)	1.00	.98	.92	.91
Nash-Sutcliffe coefficient of model-fit efficiency (NSE)	1.00	.97	.91	.90
Mean absolute error (ft³/s)	4.5	30.7	38.4	38.4
Root mean square error (ft ³ /s)	5.3	57.0	117	124

08211500 Nueces River at Calallen, Texas

Calibration period 2005-08

Streamflow volumes and peaks	Measured	Simulated	Error (percent) ¹	Criteria (percent)²
Total flow volume (acre-ft)	1,400,000	1,440,000	2.9	10
Mean flow rate (ft ³ /s)	483	497	2.9	10
Highest 10-percent daily flows (acre-ft)	1,220,000	1,215,000	4	10
Lowest 50-percent daily flows (acre-ft)	25,400	25,100	-1.2	10
Model-fit statistics 2005–08	Annual	Monthly	Daily	Hourly
Number of years, months, days, or hours	4	48	1,461	35,064
Coefficient of determination (R ²)	1.00	1.00	.95	.95
Nash-Sutcliffe coefficient of model-fit efficiency (NSE)	1.00	.99	.93	.92
Mean absolute error (ft³/s)	37	66	127	130
Root mean square error (ft ³ /s)	39	152	510	523

and annual simulations for both stations were .91, .97, and .98, respectively (table 4).

Suspended Sediment

Suspended-sediment concentrations and loads were simulated using the appropriate HSPF modules: SEDMNT for PERLND simulation, SOLIDS for IMPLND simulation, and SEDTRN for RCHRES simulation. For each PERLND, the processes of detachment of sediment from the soil matrix and washoff of this sediment were simulated on the basis of

rainfall intensity, surface runoff, and model parameters that control the accumulation, detachment, and transport of soils. For each IMPLND, the processes of accumulation and washoff of sediment were simulated in the SOLIDS module. In each RCHRES, the sediment-transport processes in the stream channel were simulated by the SEDTRN module. Transport processes in RCHRES included deposition and scour, which are functions of sediment size, settling velocity, density, erodibility, bed depth, and critical shear stress. RCHRES sediment transport is computed separately for each sand, silt, and clay fraction of sediment size.

Table 4. Streamflow calibration and testing results for the lower Nueces River watershed model, South Texas—Continued.

08211500 Nueces River at Calallen, Texas

Testing period 2001-04

Streamflow volumes and peaks	Measured	Simulated	Error (percent)¹	Criteria (percent)²
Total flow volume (acre-ft)	4,225,000	4,007,000	-5.2	10
Mean flow rate (ft ³ /s)	1,455	1,380	-5.2	10
Highest 10-percent daily flows (acre-ft)	3,167,000	3,066,000	-3.2	10
Lowest 50-percent daily flows (acre-ft)	21,500	23,500	9.3	10
Model-fit statistics 2001–04	Annual	Monthly	Daily	Hourly
Number of years, months, days, or hours	4	48	1,461	35,064
Coefficient of determination (R ²)	1.00	1.00	.96	.96
Nash-Sutcliffe coefficient of model-fit efficiency (NSE)	.98	.99	.96	.96
Mean absolute error (ft ³ /s)	83	130	307	314
Root mean square error (ft ³ /s)	133	265	838	862

08211500 Nueces River at Calallen, Texas

Simulation period 2001-08

Streamflow volumes and peaks	Measured	Simulated	Error (percent)¹	Criteria (percent)²
Total flow volume (acre-ft)	5,613,000	5,422,000	-3.4	10
Mean flow rate (ft ³ /s)	970	937	-3.4	10
Highest 10-percent daily flows (acre-ft)	4,720,000	4,507,000	-4.5	10
Lowest 50-percent daily flows (acre-ft)	46,000	46,200	0.4	10
Model-fit statistics 2001–08	Annual	Monthly	Daily	Hourly
Number of years, months, days, or hours	8	96	2,922	70,128
Coefficient of determination (R ²)	1.00	1.00	.96	.96
Nash-Sutcliffe coefficient of model-fit efficiency (NSE)	.99	.99	.96	.96
Mean absolute error (ft ³ /s)	60	98	217	222
Root mean square error (ft ³ /s)	98	216	694	713

¹ Error = [(simulated-measured)/measured] x100.

Selection of initial values and calibration of sediment-related process parameters of the SEDMNT, SOLIDS, and SEDTRN modules (appendix 1) were based on published guidelines (Donigian and Love, 2003; U.S. Environmental Protection Agency, 2006). Calibration of sediment-related parameters involved the following steps: (1) Estimation of suspended-sediment loads carried in flows released from Lake Corpus Christi; (2) identification of reasonable sediment yields from the various land types in the watershed; (3) estimation of the soil-erosion and sediment-washoff parameters used to generate sediment washoff from PERLNDs

and IMPLNDs, respectively; and (4) calibration of sediment-transport (RCHRES) parameters by comparison of measured and simulated suspended-sediment concentrations and loads at the streamflow-gaging stations.

Estimation of Suspended-Sediment Loads from Lake Corpus Christi

Estimation of suspended-sediment loads in releases from Lake Corpus Christi were based on 14 reported suspended-sediment loads during 1964–71 (table 2) and

² Default error criteria from HSPEXP (Lumb and others, 1994).

8 suspended-sediment samples collected by the USGS during 2006-07 (table 5). All suspended-sediment data (historical and recent) were collected at the Mathis gage downstream from Lake Corpus Christi. These data represented streamflow ranging from 72 to 35,129 cubic feet per second and suspendedsediment loading rates ranging from 4.3 to 8,872 tons per day. Data collected during 1964-71 represented mean streamflow and suspended-sediment loads during relatively large releases that lasted days or weeks. Data collected during 2006-07 represented instantaneous conditions (a typical suspendedsediment sample was collected during a period of about 1 hour). A least-squares regression equation (fig. 13) was developed relating daily mean discharge at the Mathis gage and daily sediment load. Discharge and sediment load were log-transformed before performing the regression to improve the quality of the regression fit, then retransformed to original units (Helsel and Hirsch, 2002). The resulting equation was

$$L = 0.026 \times Q^{1.195},$$
 (2)

where

L = suspended-sediment load, in tons per day; and Q = daily mean discharge, in cubic feet per second.

The R² for the regression equation was .94; the standard error of the residuals (RSE), a measure of the dispersion of the data around the regression line (Helsel and Hirsch, 2002, p. 244) was 121 pounds per day. The residual plot on figure 13 shows the regression residuals (as a percentage of the measured suspended-sediment load) plotted as a function of the measured suspended-sediment load. The distribution of the error residuals was relatively uniform, indicating a reasonable regression model.

A comparison of model-simulated input of suspendedsediment loads with suspended-sediment loads computed from eight samples collected at the Mathis gage during 2006– 07 is listed in table 5. These results are not considered a calibration but rather a test of the ability of the model (equation 2) to reasonably simulate instantaneous suspended-sediment concentrations and loads entering the study area through releases from Lake Corpus Christi. In general, measured and simulated suspended-sediment loads compare favorably. Regression-equation derived sediment loads were within 30 percent of measured loads for all samples except that on November 20, 2006, for which the simulated load was 116 percent of the measured load (table 5). The November 20, 2006, sample was collected during a period of low flow and represented a relatively small sediment load, compared with the relatively high flows that were mostly sampled at this site. Therefore, the loads computed by the regression equation were considered reasonable estimates of the total suspendedsediment loads that were discharged from Lake Corpus Christi.

The time series of sediment loads from Lake Corpus Christi were apportioned between clay and silt particle sizes and input to the model in RCHRES 4 (as was done with flows from Lake Corpus Christi). An apportionment of 40 percent silt and 60 percent clay was done on the basis of literature synthesis work by White and Calnan (1990).

Sediment Yields for Selected Land-Use Types and Land Covers

Several studies of sediment yields (pounds per acre or tons per acre) from specific land-use types and land covers have been made for the South Texas area (Baird and others, 1996; Ockerman and Petri, 2001; Ockerman, 2002; Ockerman and Fernandez, 2010). These studies provided data for sediment yields for cropland, rangeland (pasture/grassland), and developed land. Data and analyses from these studies were compared with simulated sediment yields from similar land type PERLNDs. Sediment-related parameters for PERLNDs were adjusted so that simulated sediment yields compared reasonably with results from field data.

Ockerman and Petri (2001, p. 19), in a study of five cropland watersheds in Nueces and Kleberg Counties, reported an average sediment yield of 610 pounds per acre during 1996-98. Ockerman and Fernandez (2010, p. 20), in a study of two primarily cropland watersheds in the Oso Creek watershed in Nueces County, reported average sediment runoff yields of 139 and 522 pounds per acre during 2005-08. In a study of sediment runoff from two rangeland study sites in San Patricio County, Ockerman (2002) reported an average sediment yield of 28 pounds per year during 2000–2001.

Estimates of suspended-solids concentrations in runoff from developed land uses and land covers were compiled for the South Texas area in a study by Baird and others (1996). Median event-mean concentrations for residential, commercial, industrial, and transportation land uses were compiled from a literature review. These event-mean concentrations were used to develop estimates of sediment loads in the Oso Creek watershed for 1989-93. The average sediment yield for developed land (including residential, commercial, industrial, and transportation land uses) was 52 pounds per acre per year (Baird and others, 1996, p. 199-218). A summary of sediment yields for selected land-use/land-cover categories is listed in table 6. The yields from the literature review were used as calibration target values for model simulations. For comparison, table 6 also lists simulated sediment yields for selected land uses and land covers for 1989-2008 in the lower Nueces River watershed. HSPF parameter values related to sediment production (sediment detachment and washoff process) were iteratively adjusted until simulated yields approximated the target yields determined from the literature review.

Annual sediment yields simulated for cropland varied from 200 to 530 pounds per acre per year depending on crop, soil type, and rainfall. Annual sediment yields simulated for rangeland were 39 to 55 pounds per acre per year; for developed land were 38 to 75 pounds per acre per year; and for open/undeveloped land were 120 to 340 pounds per acre per year. In the part of the study area where sediment yields were

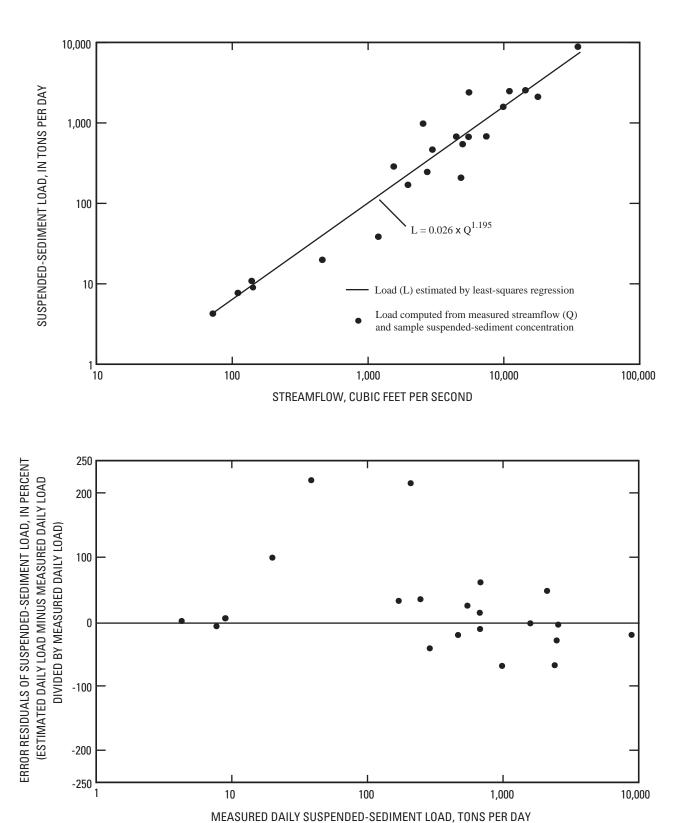


Figure 13. Relation between streamflow and suspended-sediment load and plot of regression error residuals based on 22 streamflow-load data pairs from 08211000 Nueces River near Mathis, Texas, 1964–71 and 2006–07.

Table 5. Measured and simulated streamflows, measured suspended-sediment particle size, and measured and simulated suspendedsediment concentrations and loads for selected samples collected at 08211000 Nueces River near Mathis, Texas, 2006-07.

[ft³/s, cubic feet per second; mm, millimeters; mg/L, milligrams per liter]

Sample date	Measured streamflow (ft³/s)	Simulated streamflow (ft³/s)	Measured sediment particle diameter less than 0.0625 mm (percent) ¹	Measured suspended- sediment concentration (mg/L)	Simulated suspended- sediment concentration (mg/L)	Measured suspended- sediment load (tons per day)	Simulated suspended- sediment load (tons per day)	Error in suspended- sediment load (percent) ²
May 17, 2006	139	137	98	24	28	9.0	10	11
July 20, 2006	110	117	88	26	31	7.7	10	30
Sept. 5, 2006	138	136	99	24	28	8.9	10	12
Nov. 20, 2006	72	71	94	22	48	4.3	9.3	116
May 26, 2007	461	511	99	16	12	20	17	-15
May 29, 2007	1,190	1,270	98	12	11	39	35	-10
July 1, 2007	4,830	5,120	98	16	18	208	235	13
July 11, 2007	17,800	17,600	86	44	45	2,110	2,100	5

¹ Percent by weight of sediment sample that passes through 0.0625-mm sieve.

simulated using NWS rainfall data from Robstown, sediment yields were larger for all land types compared with sediment yields for the part of the study area where sediment yields were simulated using NWS rainfall data from Mathis. The main reason for differences in sediment yields were differences in the amount of rainfall measured during 2000-2008 at the NWS stations for the two parts of the study area; average annual rainfall amounts recorded by the NWS during 2000-2008 were 34.1 inches at Robstown and 26.8 inches at Mathis.

Model Calibration

For parameters related to sediment erosion from various land types, model calibration was based on available studies

and data collected during 1989-2008. Therefore, calibration of PERLND parameters was based on simulations from the same period, 1989-2008. Model calibration of RCHRES suspendedsediment processes was based primarily on data collected at the Bluntzer and Calallen gages during 2006-07. These data were used to develop estimates of suspended-sediment loads for 2001–08, which were then compared with model simulation results.

Downstream from Lake Corpus Christi, at the Bluntzer and Calallen gages, suspended-sediment concentrations and loads in streams were calibrated to available suspended-sediment data collected by the USGS during 2006-07. RCHRES parameters related to sediment transport were adjusted (calibrated) to minimize the differences between measured and

Table 6. Comparison of literature estimates and simulation results for sediment yields from selected land covers and land uses in the lower Nueces River watershed and South Texas area.

[--, not applicable]

Land-cover/land-use type	Sediment yield estimate (pounds per acre per year) from literature and reference	Average 1989–2008 simulated sediment yield (pounds per acre per year) 200–530	
Cropland (cultivated crop)	610 (Ockerman and Petri, 2001); 139 and 522 (Ockerman and Fernandez, 2010)		
Rangeland (pasture, shrub, grassland)	28 (Ockerman, 2002)	39–55	
Developed (commercial, transportation, residential, industrial)	52 (Baird and others, 1996)	38–75	
Open/undeveloped		120-340	

² Error = [(simulated-measured)/measured] x 100.

simulated sediment concentrations and computed (from measured concentrations and measured streamflows) and simulated loads. Instream transport and concentrations of sand are simulated by power functions that relate sand concentration to average RCHRES stream velocity. The power functions are calibrated by user-selected coefficients and exponents for each RCHRES. Proceeding in downstream order, the calibration process to determine suspended-sediment concentrations and loads was repeated for each RCHRES in the intervening areas between sediment-sampling stations. Suspended-sediment calibration results at the Bluntzer gage are listed in table 7. Table 7 also lists measured and simulated suspended-sediment concentrations and measured (computed) and simulated loads from 11 samples collected during 2006-07. Errors in simulated suspended-sediment loads at the Bluntzer gage (table 7) were generally larger compared with those at the Mathis gage (table 5), mostly because of differences in the amount of error associated with the simulation of streamflow. Overall, the amount of error associated with the simulation of streamflow at the Bluntzer gage was larger than the amount of error associated with the simulation of streamflow at the Mathis gage.

Although simulation errors for instantaneous or daily suspended-sediment loads were at times relatively large (139 percent for the January 25, 2007, sample; table 7), simulation errors for longer periods (for example, monthly or yearly) were deemed acceptable. The sum of simulated sediment loads for all 11 samples (6,420 tons per day) was within 1 percent of the sum of the measured (computed) sediment loads for all 11 samples (6,360 tons per day).

Suspended-sediment data were available for 10 suspended-sediment samples collected at the Calallen gage during 2006–07. Using these data, suspended-sediment loads were computed from daily mean streamflows and measured suspended-sediment concentrations. Similar to the approach used to estimate sediment loads from Lake Corpus Christi (fig. 13), the computed loads at Calallen were used to develop a regression equation to relate daily suspended-sediment loads to daily mean streamflow (fig. 14). The resulting equation was

$$L = 0.119 \times Q^{1.061}, \tag{3}$$

where

L = suspended-sediment load, in tons per day; and

Q = daily mean discharge, in cubic feet per second.

R² for the regression equation was .87; RSE was 427

rounds per day. The residual plot on figure 14 shows regre

pounds per day. The residual plot on figure 14 shows regression residuals (as a percentage of the measured suspended-sediment load) plotted as a function of the measured suspended-sediment load. The distribution of the error residuals was relatively uniform, indicating a reasonable regression model.

The regression equation was then used to estimate suspended-sediment loads during 2001–08. The estimated daily suspended-sediment load computed using measured streamflow and sediment data was used as a calibration target to compare with simulated suspended-sediment loads at Calallen.

Model sediment-transport parameter values for RCHRESs downstream from the Calallen gage were adjusted on the basis of this comparison.

Calibration results for the Calallen gage (table 8) include a comparison of the average monthly streamflow volumes and suspended-sediment loads simulated using the HSPF model with the average monthly streamflow volumes measured at the gage and average monthly suspended-sediment loads estimated using equation 3. Table 8 also lists the annual, monthly, and daily model-fit statistics for 2001–08. A graphical comparison (fig. 15) of estimated and simulated monthly suspended-sediment loads shows reasonable agreement over the range of suspended-sediment conditions during 2001–08. Suspended-sediment calibration results for individual samples collected at the Calallen gage are listed in table 9. Table 9 compares simulated suspended-sediment concentrations and loads with estimated suspended-sediment concentrations and loads computed from measured streamflow and suspendedsediment concentrations obtained from 10 samples collected during 2006-07.

During 2001–08, average monthly suspended-sediment loads at the Calallen gage were undersimulated by 4.2 percent (table 8), compared with the loads estimated by regression (equation 3). Model-fit statistics indicated the model simulates daily, monthly, and yearly sediment loads reasonably well. The NSE was .80 for daily simulations, increasing to .95 and .97, respectively, for simulation of monthly and yearly suspended-sediment loads (table 8).

Donigian and others (1984) present general guidelines for characterizing HSPF sediment calibrations, similar to the guidelines for streamflow calibration. For annual and monthly sediment loads, model calibration is considered very good when the error is less than 15 percent, good when the error is 15–25 percent, and fair when the error is 25–35 percent. By these guidelines, calibration results for suspended-sediment loads at the Calallen gage (table 8) are considered very good. The R² and NSE values were considered acceptable for annual, monthly, and daily statistics.

Parameter Calibration Values

Calibration and testing of the HSPF model resulted in a final set of model parameter values for simulation of streamflow and suspended-sediment loads for the study area. Calibrated values (or ranges of values) for selected parameters related to hydrology simulation are listed in table 10, and calibrated values (or ranges of values) for selected sediment-related parameters used in the HSPF model are listed in table 11.

Sensitivity Analysis

Calibrated values of selected HSPF process-related parameters were further evaluated by doing a sensitivity analysis to determine the effects that changes in the selected

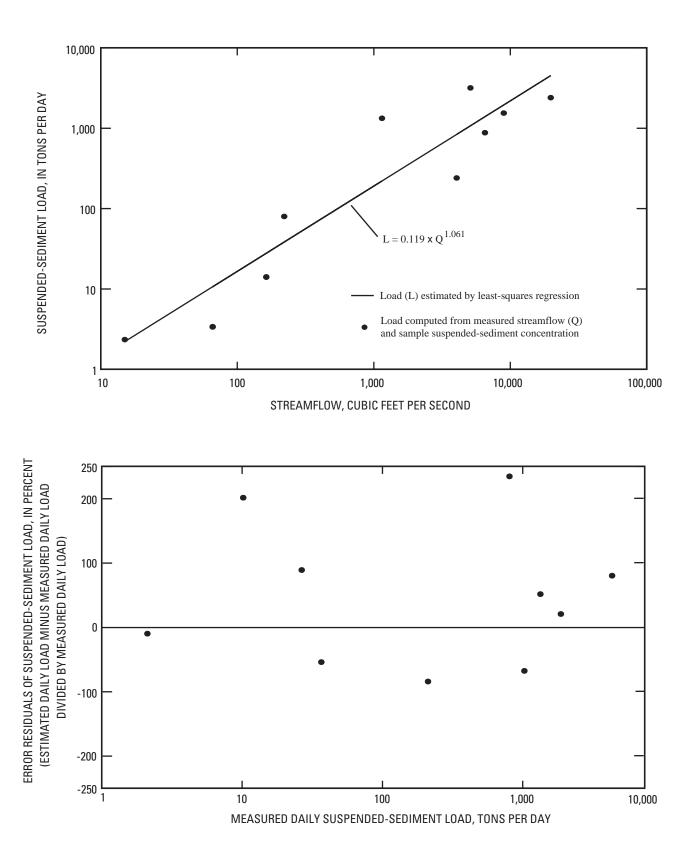
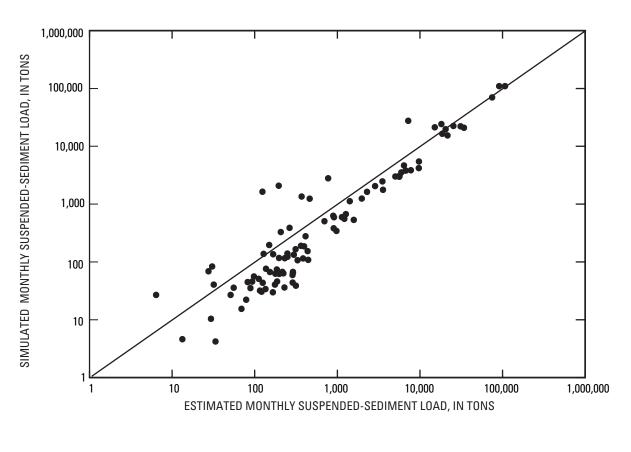


Figure 14. Relation between streamflow and suspended-sediment load and plot of regression error residuals based on 10 streamflow-load data pairs from 08211500 Nueces River at Calallen, Texas, 2006–07.



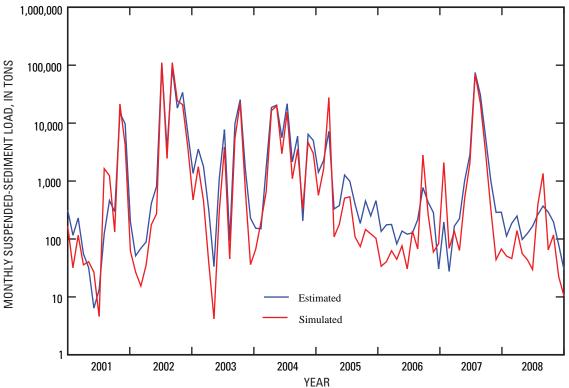


Figure 15. Estimated (by regression) and simulated monthly suspended-sediment loads for 08211500 Nueces River at Calallen, Texas, 2001–08.

Table 7. Measured and simulated streamflows, measured suspended-sediment particle size, and measured and simulated suspendedsediment concentrations and loads for selected samples collected at 08211200 Nueces River at Bluntzer, Texas, 2006-07.

[ft³/s, cubic feet per second; mm, millimeters; mg/L, milligrams per liter]

Sample date	Measured streamflow (ft³/s)	Simulated streamflow (ft³/s)	Measured sediment particle diam- eter less than 0.0625 mm (percent) ¹	Measured suspended- sediment concentration (mg/L)	Simulated suspended- sediment concentration (mg/L)	Measured suspended- sediment load (tons per day)	Simulated suspended- sediment load (tons per day)	Error in suspended- sediment load (percent) ²
May 17, 2006	121	136	97	48	22	16	8.3	-48
July 20, 2006	116	118	88	51	25	16	7.9	-51
Sept. 5, 2006	136	129	97	52	23	19	8.1	-57
Sep. 19, 2006	1,050	1,370	97	1,070	468	3,030	1,730	-43
Nov. 20, 2006	82	71	89	25	35	5.5	6.8	24
Jan. 4, 2007	153	344	81	293	344	121	238	97
Jan. 25, 2007	243	970	99	760	455	498	1,190	139
May 26, 2007	471	405	100	94	38	119	41	-66
May 29, 2007	1,330	1,170	97	54	44	194	140	-28
July 1, 2007	5,980	6,290	92	45	33	726	553	-24
July 11, 2007	15,000	14,400	79	40	64	1,620	2,500	54

¹ Percent by weight of sediment sample that passes through 0.0625-mm sieve.

Table 8. Suspended-sediment calibration results for the Hydrological Simulation Program—FORTRAN model of the lower Nueces River watershed, South Texas, 2001-08.

08211500 Nueces River at Calallen, Texas

Average monthly streamflow volumes and suspended-sediment loads	Measured/ estimated ¹	Simulated	Error (percent) ²
Flow volume (acre-feet)	58,500	56,400	-3.6
Suspended-sediment load (tons)	5,910	5,660	-4.2

Model-fit statistics for suspended-sediment loads 2001–08	Annual	Monthly	Daily
Coefficient of determination (R ²)	0.98	0.96	0.82
Nash-Sutcliffe coefficient of model-fit efficiency (NSE)	.97	.95	.80
Mean absolute error (tons)	12,200	1,640	84.4
Mean absolute error (percent)	17.1	27.8	43.4
Root mean square error (tons)	15,500	4,270	444

¹ Streamflow volumes at 08211500 are measured values; suspended-sediment loads are estimated by regression of measured streamflow and suspended-sediment loads calculated from streamflow and measured suspended-sediment concentrations for 10 sampling events during 2006-07.

² Error = [(simulated-measured)/measured] x 100.

² Error = [(simulated-measured/measured)] x100.

Table 9. Measured and simulated streamflows, measured suspended-sediment particle size, and measured and simulated suspended-sediment concentrations and loads for selected samples collected at 08211500 Nueces River at Calallen, Texas, 2006–07.

[ft³/s, cubic feet per second; mm, millimeters; mg/L, milligrams per liter]

Sample date	Measured streamflow (ft³/s)	Simulated streamflow (ft³/s)	Measured sedi- ment particle diameter less than 0.0625 mm (percent) ¹	Measured suspended- sediment concentration (mg/L)	Simulated suspended- sediment concentration (mg/L)	Measured suspended- sediment load (tons per day)	Simulated suspended- sediment load (tons per day)	Error in suspended- sediment load (percent) ²
May 16, 2006	15	22	82	58	27	2.3	1.4	-39
Sept. 19, 2006	1,150	1,340	100	429	316	1,330	992	-25
Jan. 5, 2007	66	299	98	19	129	3.4	104	2,960
Jan. 25, 2007	163	455	89	32	98	14	161	1,050
Mar. 15, 2007	221	170	32	134	74	80	40	- 50
July 2, 2007	5,100	7,360	94	231	44	3,180	871	- 73
July 4, 2007	6,530	7,070	93	50	43	882	814	- 7.8
July 5, 2007	8,960	10,200	84	64	61	1,550	1,670	7.7
July 11, 2007	19,800	17,400	49	45	80	2,410	3,750	56
Aug. 16, 2007	4,050	4,610	94	22	24	241	296	23

¹ Percent by weight of sediment sample that passes through 0.0625-mm sieve.

parameters would have on simulated streamflow and suspended-sediment loads. Each sensitivity simulation was made by adjusting a single parameter of the model by relatively large amounts (increased by 40 percent) while keeping other model parameters unchanged. The sensitivity analysis model runs were performed for 2001–08. The resulting changes in streamflow and suspended-sediment loads were evaluated at the inflow to RCHRES 97, which is considered the inflow to the Nueces Estuary. The changes in streamflow and suspended-sediment loads resulting from adjustments of selected parameters are listed in table 12.

Simulated streamflow was relatively insensitive to adjustments of any of the selected parameters. One reason is that most of the streamflow, which is simulated in the model as an input boundary condition, originates as inflow from Lake Corpus Christi releases. In the sensitivity analysis, the parameters that had the largest effects on simulated suspendedsediment loads were the index to infiltration capacity of soil (INFILT), lower-zone nominal storage (LZSN), and coefficient of detached-sediment washoff equation (KSER). Overall, changes in suspended-sediment loads when these parameters were adjusted were comparatively small compared with the changes applied to the parameter values. The INFILT parameter had the largest effect on simulated suspended-sediment loads; a 40-percent increase in the INFILT parameter resulted in a 7.1-percent decrease in the simulated suspended-sediment load. For the other parameters examined for sensitivity, increases in parameter values of 40 percent resulted in changes

to the simulated suspended-sediment load ranging from -3.2 to 4.9 percent (table 12).

Model Limitations

Errors in model calibration can be classified as systematic or measurement errors (Raines, 1996). Systematic errors are those that arise because of the model's failure to adequately represent the hydrologic and water-quality processes of the study watershed. As a result, there are limits to how well model parameters and equations can replicate the complex physical properties of streamflow and water-quality processes.

Measurement errors are those that are introduced as a result of inaccurate or missing data. The degree to which available rainfall data represent actual rainfall is potentially the most serious source of measurement error associated with this model; limitations in the amount of available rainfall data are often a serious source of measurement error for a watershed modeling study (Ockerman and Roussel, 2009). In the lower Nueces River watershed model, rainfall was applied to the study area using data from two locations—Mathis and Robstown NWS stations. Long-term rainfall totals from these two stations are likely representative of long-term study-area conditions. However, the lack of additional rainfall gages severely limits spatial resolution of rainfall input to the 218-square-mile study area, especially for relatively large storm events. Typically, measurements of extreme values of rainfall at a single location, when used to simulate average

² Error = [(simulated-measured)/measured] x 100.

Table 10. Summary of calibrated values for selected hydrologic parameters for the Hydrological Simulation Program—FORTRAN model of the lower Nueces River watershed, South Texas.

[PERLND, pervious land surface; --, none; IMPLND, impervious land surface]

Parameter	Land surface	Description	Value	Unit
AGWETP	PERLND	Fraction of available evapotranspiration from active groundwater	0	
AGWRC	PERLND	Base groundwater recession rate	.9092	1/day
BASETP	PERLND	Fraction of available evapotranspiration from baseflow	.01	
CEPSC	PERLND	Interception storage capacity	.1030	inch
DEEPFR	PERLND	Fraction of groundwater inflow to deep recharge	.20	
INFEXP	PERLND	Infiltration equation exponent	2.0	
INFILD	PERLND	Ratio of maximum to mean infiltration rate	2.0	
INFILT	PERLND	Index to infiltration capacity of soil	.2030	inch/hour
INTFW	PERLND	Index to interflow	2.0	
IRC	PERLND	Interflow recession coefficient	.50	1/day
KVARY	PERLND	Groundwater outflow modifier	2.0	1/inch
LSUR	PERLND or IMPLND	Average length of assumed overland-flow plane	250	foot
LZETP	PERLND	Lower-zone evapotranspiration	.280	
LZSN	PERLND	Lower-zone nominal storage	3.2-3.4	inch
NSUR	PERLND or IMPLND	Manning's n for assumed overland-flow plane	.2	
RETSC	IMPLND	Retention storage capacity of impervious areas	.10	inch
SLSUR	PERLND or IMPLND	Average slope of assumed overland-flow plane	.0104	
UZSN	PERLND	Upper-zone nominal storage	.2530	inch

rainfall for larger areas (such as the lower Nueces River watershed) tend to overestimate the average rainfall of the watershed (Asquith, 1999). Also, the overestimation increases as the size of the watershed increases; the fewer rain gages available for a study area, the greater the probability that rainfall in the study area is overestimated during large storms.

Another limitation of the rainfall data used for the model was that all data from the Mathis NWS station and much of the data from the Robstown NWS station were only available as daily totals. These daily totals were disaggregated to hourly values based on theoretical temporal distributions. As a result, some loss of accuracy in the timing and intensity of rainfall might have been incorporated into the model results.

Overall, streamflow was simulated with reasonable accuracy at the streamflow-gaging stations in the study area. Most of the streamflow originating in the study area was from Lake Corpus Christi releases and was measured by the Mathis gage. The Bluntzer gage was a partial-record station and did not record streamflows greater than 2,750 cubic feet per second. Similar to streamflow at the Mathis gage, streamflow at the Bluntzer gage was largely controlled by releases from Lake Corpus Christi. The accuracy of simulated streamflow at the Bluntzer gage might have been diminished during periods

when additional runoff downstream from Lake Corpus Christi was occurring. Streamflow simulation results at the Calallen gage were generally very good. Low flows at the Calallen gage (streamflows of less than about 10 cubic feet per second) exhibited greater error compared with higher flows (fig. 12, middle graph). The errors in low flows at the Calallen gage were possibly a result of withdrawals upstream from the gage, which were only available as monthly totals. Because only monthly totals were available to represent the withdrawals upstream, hourly and daily streamflows at the Calallen gage during low-flow periods were subject to greater uncertainty compared with monthly or annual flows.

Suspended-sediment simulations also were affected by uncertainties in parameters associated with rainfall and streamflow. For example, overestimation of rainfall parameters (in particular, rainfall intensity) would increase soil particle detachment and washoff of sediment from PERLNDs, resulting in possible oversimulation of runoff and suspendedsediment concentrations and loads.

How well the model represents basin-wide suspendedsediment yields for various land types is somewhat uncertain. Calibration of basin-wide suspended-sediment yields depends on reasonable estimates of sediment yields from various

Table 11. Summary of calibrated values for selected sediment-related parameters for the Hydrological Simulation Program—FORTRAN model of the lower Nueces River watershed, South Texas.

[PERLND, pervious land surface; IMPLND, impervious land surface; RCHRES, stream/reservoir reach]

Parameter	Model unit	Parameter description	Value	Unit
KRER	PERLND	Coefficient of soil-detachment equation	0.0745	complex
JRER	PERLND	Exponent of soil-detachment equation	2.0	complex
KSER	PERLND	Coefficient of detached-sediment washoff equation	.0565	complex
JSER	PERLND	Exponent of detached-sediment washoff equation	2.0	complex
AFFX	PERLND	Fraction by which detached sediment decreases daily through soil compaction	.05	1/day
COVER	PERLND	Fraction of land surface shielded from rainfall erosion	.10–.85	none
NVSI	PERLND	Rate at which sediment enters detached-sediment storage from atmosphere	.3	pound/acre-day
KEIM	IMPLND	Coefficient of solids washoff equation	.40	complex
JEIM	IMPLND	Exponent of solids washoff equation	2.0	complex
ACCSDM	IMPLND	Solids accumulation rate	.015	ton/acre-day
RHO	RCHRES	Density of sediment particle	2.0-2.6	gram/cubic centimeter
M (silt)	RCHRES	Erodibility coefficient of sediment	.0712	pound/square foot-day
M (clay)	RCHRES	Erodibility coefficient of sediment	.0914	pound/square foot-day
W (silt and clay)	RCHRES	Settling velocity of sediment particle in still water	.000010005	inch/second
TAUCD (silt)	RCHRES	Critical bed shear stress for sediment deposition	.0308	pound/square foot
TAUCS (silt)	RCHRES	Critical bed shear stress for sediment scour	.10–.22	pound/square foot
TAUCD (clay)	RCHRES	Critical bed shear stress for sediment deposition	.015–.04	pound/square foot
TAUCS (clay)	RCHRES	Critical bed shear stress for sediment scour	.09–.14	pound/square foot

land types. Several studies of suspended-sediment yields for specific land types provided guidance for establishing target yields for calibration. The target yields were reasonably simulated by the model—albeit the sediment-yield studies used to establish the targets were limited in scope compared with the size of the study watershed and variety of land uses and land covers in the lower Nueces River watershed.

Overall, suspended-sediment calibration results were very good for monthly and annual suspended-sediment loads. Relatively large errors were associated with simulated daily suspended-sediment loads (table 8) and with suspended-sediment load estimated for individual samples (table 9).

Estimated Suspended-Sediment Loads to the Nueces Estuary, 1958–2008

The calibrated model of the lower Nueces River watershed was used to estimate suspended-sediment loads to the Nueces Estuary for 1958–2008, as well as sediment loading

rates (mass per unit area) and sediment loads simulated for the different land types and within different geographic areas. The model configuration and parameter values that were used for the 2001–08 calibration period were retained for the 1958–2008 simulation period. The rainfall and potential evapotranspiration data used for modeling 1958–2008 were obtained from the same NWS stations used for calibration.

The location chosen for modeling delivery of the suspended-sediment load to the Nueces Estuary was USGS streamflow-gaging station 08211502 Nueces River near Odem, Tex. (hereinafter the Odem gage) (site 11, fig. 10; table 3). The Odem gage is located in the tidal reach of the Nueces River at Interstate Highway 37, about 1.1 miles downstream from the Calallen gage and 0.2 mile downstream from the confluence of Hondo Creek and the Nueces River (near the confluence of RCHRESs 95 and 96, or the inflow to RCHRES 97; fig. 9).

Estimated annual suspended-sediment loads, in tons per day, transported during 1958–2008 to the Nueces Estuary are shown on figure 16 and listed in table 13. The annual mean

Table 12. Sensitivity of simulated streamflow volumes and suspended-sediment loads to changes in selected process-related parameters for the lower Nueces River watershed model, South Texas.

[DEEPFR, fraction of groundwater inflow to deep recharge; INFILT, index to infiltration capacity of soil; LZSN, lower-zone nominal storage; KRER, coefficient of soil-detachment equation; KSER, coefficient of detached-sediment washoff equation; M (silt), erodibility coefficient of silt sediment; M (clay), erodibility coefficient of clay sediment]

Parameter	Initial value	Adjusted value	Change in parameter value (percent)¹	Change in streamflow volume (percent) ¹	Change in suspended- sediment load (percent)¹
DEEPFR	0.20	0.28	40	-0.6	-0.5
INFILT	.2030	.2842	40	2	-7.1
LZSN	3.20-3.40	4.48–4.76	40	-1.0	-3.2
KRER	.07–.45	.10–.63	40	0	0
KSER	.05–.65	.07–.91	40	0	4.9
M (silt)	.0712	.10–.17	40	0	2.1
M (clay)	.09–.14	.13–.20	40	0	1.1

¹ Simulation period for sensitivity analyses, 2001–08.

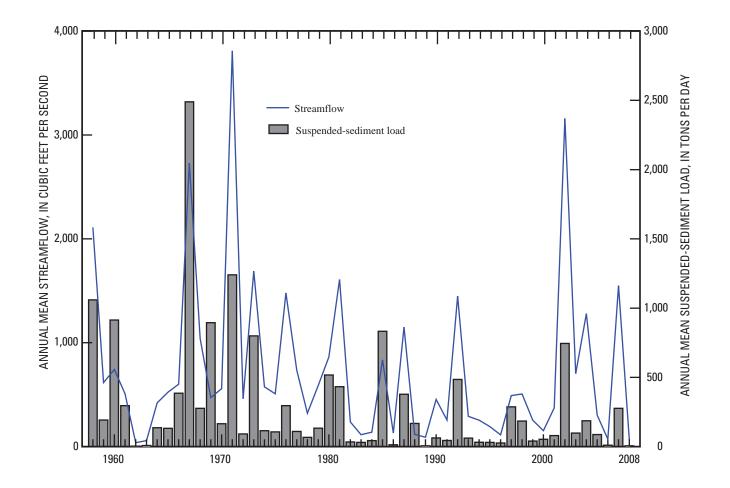


Figure 16. Estimated annual streamflow and suspended-sediment loads to the Nueces Estuary, South Texas, 1958–2008.

 Table 13.
 Estimated annual streamflows and suspended-sediment loads to the Nueces Estuary, South Texas, 1958–2008.

Year	Annual mean streamflow (cubic feet per second)	Annual suspended-sediment load (tons)	Annual mean suspended-sediment load (tons per day)
1958	2,030	387,160	1,060
1959	591	70,100	192
1960	816	334,000	914
1961	431	108,000	296
1962	38	1,390	3.8
1963	58	2,820	7.7
1964	421	50,000	137
1965	529	48,400	133
			386
1966	598	141,000	
1967	2,740	909,500	2,490
1968	1,040	101,000	277
1969	486	327,000	895
1970	544	60,400	165
1971	3,820	452,910	1,240
1972	452	33,400	91
1973	1,690	292,000	799
1974	572	42,100	115
1975	507	39,000	107
1976	1,500	108,000	296
1977	716	40,000	110
1978	324	24,500	67
1979	586	48,600	133
	862		517
1980		189,000	
1981	1,620	158,000	433
1982	236	12,500	34
1983	123	11,100	30
1984	144	15,800	43
1985	822	304,000	832
1986	144	4,880	13
1987	1,140	138,000	378
1988	117	61,300	168
1989	91	2,220	6.1
1990	453	22,700	62
1991	276	16,300	45
1992	1,430	177,000	485
1993	290	22,300	61
1994	259	11,600	32
1995	190	11,100	30
1996	111	9,720	27
1997	491	105,000	287
1998	510	67,400	185
1999	252	14,600	40
2000	156	19,500	53
2001	371	29,000	79
2002	3,170	272,000	745
2003	692	35,700	98
2004	1,290	68,200	187
2005	292	31,700	87
2006	79	3,700	10
2007	1,550	101,000	277
2007	64	2,320	6.4
958–2008	04	2,320	0.4
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suspended-sediment load was highly variable, ranging from an estimated 3.8 tons per day in 1962 to 2,490 tons per day in 1967. The daily mean suspended-sediment load during 1958–2008 was an estimated 297 tons per day. The median suspended-sediment load was an estimated 133 tons per day.

Estimated annual mean sediment loads at the Nueces Estuary (Odem gage) were compared with those estimated at the Mathis gage (outflow of Lake Corpus Christi) and at the Bluntzer and Calallen gages (table 14). These comparisons give an indication of sources of sediment delivered to the Nueces River and Nueces Estuary.

Estimated annual suspended-sediment loads in the lower Nueces River simulated by the model for each major source of sediment in the study area are listed by category in table 15. On average, an estimated 307 tons per day of suspended sediment were delivered to the lower Nueces River; an estimated 297 tons per day were delivered to the Nueces Estuary. Releases from Lake Corpus Christi delivered an estimated 98 tons per day of suspended sediment or 32 percent of the 307 tons per day estimated to have been delivered to the lower Nueces River. This indicated that, on average, about 209 tons per day of sediment, or 68 percent of the estimated 307 tons per day total were generated from erosion of land surfaces and stream-channel bed and banks. The largest source of sediment originating from within the study area was generated from cropland, about 117 tons per day, or about 38 percent of the total estimated for all sources each year, on average. Erosion of stream-channel bed and banks accounted for, on average, 55 tons per day, or 18 percent of the estimated total suspendedsediment load. All other land categories, except cropland, accounted for an estimated 37 tons per day, or 12 percent of the total. An estimated 9.6 tons per day of suspended sediment or 3 percent of the suspended-sediment load delivered to the lower Nueces River were removed by water withdrawals before reaching the Nueces Estuary.

Model results indicate most of the sediment load in the Nueces River consists of silt and clay, defined as particle sizes less than 0.0625 millimeter. At the Bluntzer gage, simulated silt and clay loads composed about 99 percent of the total suspended-sediment load. At the Calallen gage, simulated silt and clay loads composed about 98 percent of the total suspended-sediment load.

Annual streamflows and suspended-sediment loads to the Nueces River and Nueces Estuary varied depending on rainfall and streamflow conditions. Annual suspendedsediment loads delivered to the estuary ranged from 3.8 tons per day in 1962 to 2,490 tons per day in 1967. Large rainfall and runoff events contributed most of the streamflow and suspended-sediment loads to the Nueces River. Whether the runoff occurred upstream or downstream from Lake Corpus Christi affected the amount of annual suspended-sediment loads that were contributed by the each of the main sources shown in table 15. During some years with relatively low annual average streamflow (1984, 1988, 1989, and 1995) net stream-channel erosion was negative, indicating net annual sediment deposition to the stream channel. Simulation results indicate that the largest sediment loads transported to the Nueces Estuary as a result of stream-channel bed and bank erosion occurred during years with relatively large annual mean streamflows (for example, 1958, 1967, and 1971). During low-flow years, relatively large percentages of the total suspended sediment transported to the Nueces River were removed by water withdrawals. For example, 2008 was a lowflow year; the annual mean streamflow of the Nueces River to the Nueces Estuary was 64 cubic feet per second in 2008 compared with annual median streamflow of 507 cubic feet per second measured during 1958–2008, and about 50 percent of the total suspended-sediment load was removed in 2008 by water withdrawals.

 Table 14.
 Estimated annual suspended-sediment loads at selected stations, lower Nueces River watershed, South Texas, 1958–2008.

	Suspended-sediment load (tons per day)							
Year	08211000 Nueces River near Mathis, Texas	08211200 Nueces River at Bluntzer, Texas	08211500 Nueces River at Calallen, Texas	08211502 Nueces River near Odem, Texas				
1958	304	906	1,070	1,060				
1959	70	180	200	192				
1960	70	630	789	914				
1961	45	236	277	296				
1962	5.9	8.3	8.1	3.8				
1963	6.3	13	13	7.7				
1964	46	133	143	137				
1965	64	126	141	133				
1966	50	315	359	386				
1967	616	1,650	2,170	2,490				
1968	134	224	279	277				
1969	33	602	756	895				
1970	61	131	171	165				
1971	646	805	1,190	1,240				
1972	45	85	102	91				
1973	203	545	726	799				
1974	61	108	118	115				
1975	59	100	120	107				
1975	164	261	301	296				
1977								
1977	93 30	102 62	121 68	110				
				67				
1979	57	106	133	133				
1980	111	350	460	517				
1981	202	353	416	433				
1982	28	28	43	34				
1983	9.0	30	31	30				
1984	7.6	28	41	43				
1985	72	567	709	832				
1986	13	18	18	13				
1987	142	290	350	378				
1988	10	129	145	168				
1989	10	8.8	10	6.1				
1990	50	63	67	62				
1991	18	33	47	45				
1992	156	372	444	485				
1993	23	58	62	61				
1994	19	34	35	32				
1995	15	26	33	30				
1996	10	27	28	27				
1997	33	214	256	287				
1998	43	134	166	185				
1999	24	38	44	40				
2000	10	26	52	53				
2001	39	65	80	79				
2002	578	569	736	745				
2003	90	90	115	98				
2004	154	171	192	187				
2005	26	74	82	87				
2006	6.7	11	13	10				
2007	221	221	282	277				
2008	5.9	6.8	11.1	6.4				
58–2008 average								
average	98	223	279	297				

Table 15. Estimated annual streamflows and suspended-sediment loads, by sediment source, simulated by the Hydrological Simulation Program—FORTRAN model of the lower Nueces River watershed, South Texas, 1958–2008.

	Annual mean streamflow,	w, (tons per day)							
Year	Nueces River to Nueces Estuary (cubic feet per second) ¹	Inflow asso- ciated with Lake Corpus Christi releases	Erosion and washoff from cropland	Erosion and washoff from other land-cover categories	Erosion from stream- channel bed and banks ²	Total transported to lower Nueces River³	Removed by water withdrawals	Total transported to Nueces Estuary ⁴	
1958	2,110	304	26	18	729	1,080	16	1,060	
1959	617	70	31	17	87	205	12	193	
1960	742	70	677	114	74	935	19	916	
1961	508	45	140	44	79	308	12	296	
1962	38	5.9	0	4.7	5.1	16	12	3.7	
1963	58	6.3	.1	7.1	.9	14	6.7	7.7	
1964	421	46	15	9.0	77	147	10	137	
1965	524	64	6.2	5.0	67	142	10	132	
1966	601	50	171	57	122	400	14	386	
1967	2,730	616	1,240	416	232	2,500	11	2,490	
1968	1,040	135	39	15	99	288	10	278	
1969	472	33	668	201	7.3	909	14	895	
1970	559	61	39	16	61	177	11	166	
1971	3,810	647	240	74	296	1,260	16	1,240	
1972	462	45	6.4	6.6	43	101	9.3	92	
1973	1,690	203	384	132	95	814	14	800	
1974	575	61	22	7.5	31	122	6.8	115	
1975	509	59	37	11	12.9	120	13	107	
1976	1,480	164	38	13	92	307	11	296	
1977	737	93	4.1	3.2	18	118	8.8	109	
1978	321	30	18	10	16	74	7.1	67	
1979	589	58	37	15	33	143	9.3	133	
1980	863	112	304	88	28	532	14	518	
1981	1,610	202	124	44	73	443	11	432	
1982	238	28	4.3	4.4	4.7	41	7.7	34	
1983	115	9.0	19	5.2	4.5	38	7.0	31	
1984	139	7.6	32	11	-1.3	49	5.5	44	
1985	835	72	567	189	24	852	19	833	
1986	131	13	.1	2.5	2.7	18	5.1	13	
1987	1,150	142	147	52	47	388	11	377	
1988	118	10	155	19	-6	178	10	168	
1989	91	10	.2	2.9	-3.4	10	4.0	6.0	
1990	454	50	12	4.7	2.3	69	6.6	62	
1991	256	18	12	7.9	12.8	51	6.0	45	
1992	1,450	156	215	75	50	496	11	485	
1993	291	23	24	11	11.4	69	9.0	60	

Table 15. Estimated annual streamflows and suspended-sediment loads, by sediment source, simulated by the Hydrological Simulation Program—FORTRAN model of the lower Nueces River watershed, South Texas, 1958–2008—Continued.

	Annual mean streamflow,								
Year	Nueces River to Nueces Estuary (cubic feet per second) ¹	Inflow asso- ciated with Lake Corpus Christi releases	Erosion and washoff from cropland	Erosion and washoff from other land-cover categories	Erosion from stream- channel bed and banks ²	Total transported to lower Nueces River³	Removed by water withdrawals	Total transported to Nueces Estuary ⁴	
1994	254	19	6.4	4.4	7.6	37	5.2	32	
1995	193	15	17	7.1	-2.1	37	6.3	31	
1996	113	10	14	6.5	1.2	32	5.4	27	
1997	491	33	181	49	34	297	10	287	
1998	507	43	108	31	9.5	192	6.8	185	
1999	255	24	8.4	5.5	7.8	46	5.8	40	
2000	154	10	29	10	9.6	59	6.0	53	
2001	373	39	20	8.4	19	86	6.6	79	
2002	3,160	578	63	16	96	753	8.2	745	
2003	703	90	.7	3.9	17.0	112	13	98	
2004	1,280	154	14	8.5	20	197	10	187	
2005	303	26	44	17	10.0	97	10	87	
2006	73	6.7	1.6	3.9	2.2	14	4.4	10	
2007	1,550	221	14	7.4	41	283	6.8	276	
2008	64	5.9	.7	3.6	2.3	13	6.1	6.4	
1958–2008 average annual	741	98	117	37	55	307	9.6	297	

¹ Simulated streamflow at Nueces River at Interstate Highway 37, near Corpus Christi, Texas.

Summary

The U.S. Geological Survey (USGS), in cooperation with the U.S. Army Corps of Engineers—Fort Worth District, City of Corpus Christi, Guadalupe-Blanco River Authority, San Antonio River Authority, and San Antonio Water System, developed, calibrated, and tested a Hydrological Simulation Program—FORTRAN (HSPF) watershed model to simulate streamflow and suspended-sediment concentrations and loads during 1958–2008 in the lower Nueces River watershed, downstream from Lake Corpus Christi to the Nueces Estuary in South Texas. The loss of sediment loading is an important ecological problem in the Nueces River watershed in South Texas. The reduction in sediment loads to the Nueces Estuary is the result of sedimentation in large impoundments,

notably Lake Corpus Christi, a reservoir whose storage volume was greatly enlarged in 1958 compared to its original (1935) impoundment capacity; construction of the 1958 dam expanded the existing storage volume of Lake Corpus Christi from 43,800 to 257,260 acre-feet.

Data available to simulate suspended-sediment concentrations and loads consisted of historical sediment data collected during 1942–82 by the Texas Water Development Board (TDWB) and its predecessor agencies at sites in the study area, and suspended-sediment concentration data collected periodically by the USGS during 2006–07 at three sites—USGS streamflow-gaging station 08211000 Nueces River near Mathis, Tex. (Mathis gage), station 08211500 Nueces River at Bluntzer, Tex. (Bluntzer gage), and station 08211500 Nueces River at Calallen, Tex. (Calallen gage).

² Negative number indicates net suspended-sediment deposition to stream reach.

³ Sum of suspended-sediment loads for inflow associated with Lake Corpus Christi releases, erosion and washoff from cropland, erosion and washoff from other land-cover categories, and erosion from stream-channel bed and banks. In some cases rounding might result in discrepancies from the computed sum of these categories.

⁴ Total suspended-sediment loads transported to lower Nueces River minus loads removed by water withdrawals. In some cases rounding might result in discrepancies from the computed difference.

During 1942-82, TWDB and its predecessor agencies monitored suspended-sediment loads of the Nueces River at a site on the Nueces River at the Mathis gage. The Mathis gage is downstream from Wesley E. Seale Dam, completed in 1958 to impound Lake Corpus Christi. Suspended-sediment data collected before and after completion of Wesley E. Seale Dam provide insights to the effects of the dam and reservoir on suspended-sediment loads transported by the lower Nueces River from downstream of the dam to the Nueces Bay. Suspendedsediment loads measured at the Mathis gage decreased after completion of the dam in 1958. Annual suspended-sediment loads exceeded 100,000 tons during 9 of the 16 years before completion of the dam but during only 2 years after completion of the dam. In the 21 years following completion of the dam, for which reliable suspended-sediment data were available (1962-82), annual suspended-sediment loads were considerably lower, for a given annual mean discharge, than before the dam was completed.

Most of the suspended sediment transported by the Nueces River downstream from Wesley E. Seale Dam occurred during high-flow releases from the dam or during floods. During October 1964-September 1971, about 532,000 tons of suspended sediment were transported by the Nueces River near Mathis. Of this amount, about 473,000 tons, or about 89 percent, were transported by large runoff events (mean streamflow exceeding 1,000 cubic feet per second). Furthermore, the suspended-sediment transport rate increases with higher magnitude flows.

To develop the watershed model to simulate suspendedsediment concentrations and loads in the lower Nueces River watershed, streamflow simulations were calibrated and tested with available data for 2001-08. Streamflow data measured during 2005-08 at the Calallen gage were used to calibrate the model and streamflow data for 2001-04 were used to test the calibrated model. Streamflow data for 2005-06 from the Bluntzer gage were used to test streamflow simulations at that site. Streamflow data from the Mathis gage were used as input to the model at the upstream boundary of the model. Simulated streamflow volumes for the Bluntzer and Calallen gages showed good agreement (within 6 percent) with measured streamflow volumes. Annual, monthly, and daily coefficients of determination of the linear regression between measured and simulated streamflow and Nash-Sutcliffe coefficients of model-fit efficiency are considered acceptable for both gages.

The HSPF model was calibrated to simulate suspended sediment using suspended-sediment data collected at the Mathis, Bluntzer, and Calallen gages during 2006–07. Soil erosion and sediment washoff from various land types simulated by the model were calibrated by comparing simulated suspended-sediment loads from the various land types to estimates of suspended-sediment runoff yields determined from other studies. Parameters related to sediment transport were calibrated primarily by comparing measured suspendedsediment concentrations and loads at the three gages with simulated suspended-sediment concentrations and loads.

The calibrated watershed model was used to estimate streamflow and suspended-sediment loads for 1958–2008, including suspended-sediment loads transported to the Nueces Estuary. During 1958–2008, on average, an estimated 307 tons per day of suspended sediment were delivered to the lower Nueces River; an estimated 297 tons per day were delivered to the estuary. The annual suspended-sediment load was highly variable, depending on the occurrence of storm events and high streamflows. During 1958-2008, the annual total sediment loads to the estuary varied from an estimated 3.8 to 2,490 tons per day. On average, 117 tons per day, or about 38 percent of the estimated annual suspended-sediment contribution, originated from cropland in the study watershed. Releases from Lake Corpus Christi delivered an estimated 98 tons per day of suspended sediment or about 32 percent of the 307 tons per day estimated to have been delivered to the lower Nueces River. Erosion of stream-channel bed and banks accounted for 55 tons per day or about 18 percent of the estimated total suspended-sediment load. All other land categories, except cropland, accounted for an estimated 37 tons per day, or about 12 percent of the total. An estimated 9.6 tons per day of suspended sediment or about 3 percent of the suspended-sediment load delivered to the lower Nueces River were removed by water withdrawals before reaching the Nueces Estuary.

Annual streamflows and suspended-sediment loads to the Nueces River and Nueces Estuary varied depending on rainfall and streamflow conditions. Large rainfall and runoff events contributed most of the streamflow and suspendedsediment loads to the Nueces River. Whether the runoff occurred upstream or downstream from Lake Corpus Christi affected the amount of annual suspended-sediment loads that were contributed by inflows or erosion processes. Simulation results indicate that the largest sediment loads transported to the Nueces Estuary as a result of stream-channel bed and bank erosion occurred during high-flow years and that during low-flow years, large percentages of total suspended sediment transported to the Nueces River were removed by water withdrawals.

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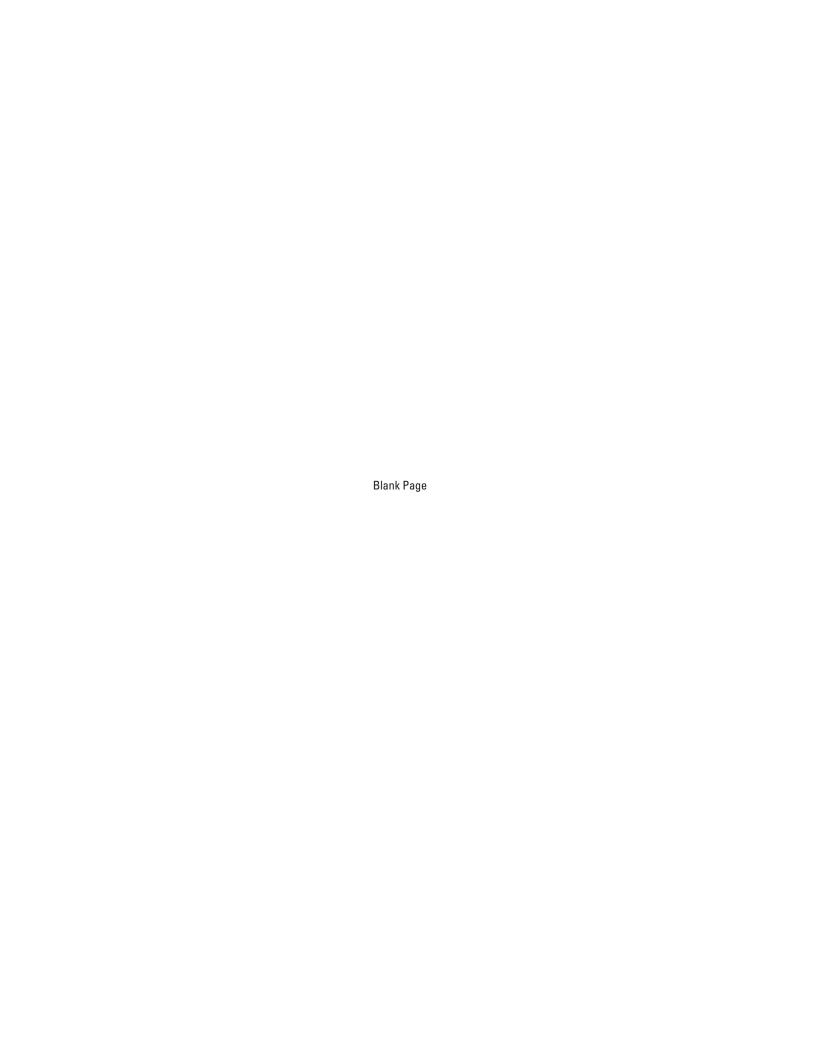
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Appendix 1—Hydrological Simulation Program—FORTRAN (HSPF) Parameters Used to Simulate Hydrologic and Sediment Processes in the Lower Nueces River Watershed, South Texas



Appendix 1. Hydrological Simulation Program—FORTRAN parameters used to simulate the hydrologic and sediment processes in the lower Nueces River watershed, South Texas.

[--, none; HSPF, Hydrological Simulation Program—FORTRAN]

Pervious Land (PERLND)

Secondary module	Parameter	Unit	Description
			Water balance
Interception	storage		
PWATER	CEPSC	inch	Interception storage capacity
	CEPS	inch	Initial interception storage
Surface and s	subsurface sto	orages	
	UZSN	inch	Upper-zone nominal storage; an index to amount of depression and surface-layer storage of a pervious area
	LZSN	inch	Lower-zone nominal storage; an index to soil-moisture-holding capacity
	SURS	inch	Initial surface storage
	IFWS	inch	Initial interflow storage
	UZS	inch	Initial upper-zone storage
	LZS	inch	Initial lower-zone storage
	AGWS	inch	Initial active-groundwater storage
Evapotranspi	ration		
	LZETP		Lower-zone evapotranspiration; an index to density of deep-rooted vegetation on a pervious area
	AGWETP		Fraction of available potential evapotranspiration demand from active groundwater
	BASETP		Fraction of available potential evapotranspiration demand from baseflow (groundwater outflow)
Recession ra	tes		
	KVARY	1/inch	Groundwater outflow modifier; an index of how much effect recent recharge has on groundwater outflow
	AGWRC	1/day	Basic groundwater recession rate if KVARY is zero and there is no inflow to groundwater
	IRC	1/day	Interflow recession coefficient
	GWVS	inch	Index to groundwater slope
Infiltration			
	INFILT	inch/hour	Index to infiltration capacity of soil
	INFILD		Ratio of maximum to mean infiltration rate of a pervious area
	INFEXP		Infiltration equation exponent
	INTFW		Index to amount of water that infiltrates and flows as interflow (shallow subsurface runoff)
	DEEPFR		Fraction of groundwater inflow to deep recharge

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Appendix 1. Hydrological Simulation Program—FORTRAN parameters used to simulate the hydrologic and sediment processes in the lower Nueces River watershed, South Texas—Continued.

Pervious Land (PERLND)—Continued

Secondary module	Parameter	Unit	Description
			Water balance—Continued
Overland flo	Overland flow		
	LSUR	foot	Average length of overland-flow plane
	SLSUR		Average slope of overland-flow plane
	NSUR		Average roughness coefficient of overland-flow plane
Soil erosion			
SEDMNT	SMPF		Management factor to account for use of erosion-control factors
	KRER	complex	Coefficient of soil-detachment equation
	JRER	complex	Exponent of soil-detachment equation
	AFFIX	1/day	Fraction by which detached sediment decreases daily through soil compaction
	COVER		Fraction of land surface shielded by vegetation or mulch from erosion by direct rainfall impact
	NVSI	pound/acre/ day	Rate at which sediment enters detached-sediment storage from atmosphere
	KSER	complex	Coefficient of detached-sediment washoff equation
	JSER	complex	Exponent of detached-sediment washoff equation
	KGER	complex	Coefficient of matrix soil scour equation
	JGER	complex	Exponent of matrix soil scour equation

Appendix 1. Hydrological Simulation Program—FORTRAN parameters used to simulate the hydrologic and sediment processes in the lower Nueces River watershed, South Texas—Continued.

Impervious Land (IMPLND)

Secondary module	Parameter	Unit	Description					
	Water balance							
IWATER	LSUR	foot	Average length of overland-flow plane					
	SLSUR		Average slope of overland-flow plane					
	NSUR		Average roughness coefficient of overland-flow plane					
	RETSC	inch	Retention storage capacity of impervious areas					
	RETS	inch	Initial retention storage					
	SURS	inch	Initial overland-flow storage					
	Sediment washoff							
SOLIDS	KEIM	complex	Coefficient of solids washoff equation					
	JEIM	complex	Exponent of solids washoff equation					
	REMSDP	1/day	Fraction of solids removed on each day without runoff					
	ACCSDP	ton/acre- day	Solids accumulation rate					
	SLDS	ton/acre	Initial storage of solids					

Stream Reaches (RCHRES)

Secondary module	Parameter	Unit	Description
			Water balance
HYDR	FTABNO		Number of F-table that contains RCHRES geometric and hydraulic properties
	LEN	mile	Length of stream reach
	DELTH	foot	Drop in water elevation within stream reach
	STCOR	foot	Correction in reach depth to calculate stage
	KS		Weighting factor for flow routing
	DB50	millimeter	Median diameter of bed sediment
ADCALC	CRRAT		Ratio of maximum velocity to mean velocity in reach cross section under typical flow condition

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Appendix 1. Hydrological Simulation Program—FORTRAN parameters used to simulate the hydrologic and sediment processes in the lower Nueces River watershed, South Texas—Continued.

Stream Reaches (RCHRES)—Continued

Secondary module	Parameter	Unit	Description
			Sediment transport
SEDTRN	BEDWID	foot	Width of streambed
	BEDWRN	foot	Depth of streambed
	POR		Porosity of streambed
	D	inch	Effective diameter of sediment particle
	W	inch/second	Settling velocity of sediment particle in still water
	RHO gram/cubic centi- meter		Density of sediment particle
	KSAND	complex	Coefficient of HSPF sand-load equation
	EXPSND	complex	Exponent of HSPF sand-load equation
	TAUCD	pound/square foot	Critical bed shear stress for sediment deposition
	TAUCS	pound/square foot	Critical bed shear stress for sediment scour
	M	pound/square foot-day	Erodibility coefficient of sediment
	BEDDEP	foot	Initial thickness of bed material
	SSAND	milligram/liter	Initial concentration of sand in suspension
	SSILT	milligram/liter	Initial concentration of silt in suspension
	SCLAY	milligram/liter	Initial concentration of clay in suspension
	FRACSAND		Initial fraction by weight of sand in bed material
	FRACSILT		Initial fraction by weight of silt in bed material
	FRACCLAY		Initial fraction by weight of clay in bed material

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